



Project Number: U2716-0231-201

May 10, 2023

Sunmodo  
14800 NE 65<sup>th</sup> Street  
Vancouver, WA 98682

**REFERENCE: Sunmodo Sunturf Ground Mount A5a  
Ground Mount PV Array Installation**

To Whom It May Concern:

Per request of Sunmodo, we have been asked to prepare the structural design of a ground-mounted PV solar array system with several foundation options as shown in the attached calculations. The adopted building code in this jurisdiction is the International Building Code, 2015 Edition. Vector Structural Engineering requires that we review each site specific install, and we are not liable for installs at site specific locations we have not reviewed. The following design parameters are used in our analysis:

- Minimum Design Loads for Buildings and Other Structures (ASCE 7-16)
- Design wind speed for risk category I structures: 140 mph
- Wind exposure: B
- Ground snow load: 70 psf
- The ground screws and helical piers must be tested to 1.5 times uplift and 2.0 times lateral reactions found in the table below. A minimum of one ground screw or helical pier must be tested.

Load (ASD)	Value (lbs)	Factor of Safety	Test Value (lbs)
UPLIFT	2320	1.5	3480
LATERAL	1690	2	3380

Foundation concrete shall have a minimum compressive strength of 2500 psi at 28 days. Cement for all concrete shall be Type I or II with a minimum of 6% entrained air with a water/cement ratio of 0.50. Maximum aggregate size shall be 3/4". No special inspection of concrete strength is required.

Footings are designed based on an allowable soil bearing pressure of 1500 psf an allowable skin friction of 250 psf, an allowable lateral bearing pressure of 150 pcf, and a coefficient of friction of 0.3. Vector Structural Engineering strongly recommends independent soils testing be performed by a licensed geotechnical engineer to verify the assumed soil parameters.

All ground mounts are to be installed per manufacturer’s recommendations. The use of solar panel support span tables provided by the manufacturer is allowed only where the site conditions and solar panel configuration match the description of the span tables. Electrical engineering is beyond our scope. All work performed must be in accordance with accepted industry-wide methods and applicable safety standards. Vector Structural Engineering assumes no responsibility for improper installation of the solar panels.

Very truly yours,

VECTOR STRUCTURAL ENGINEERING, LLC

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Wells Holmes, S.E.  
License: 54240 - Expires: 06/30/2024  
Project Engineer

Enclosures

RNE/stb

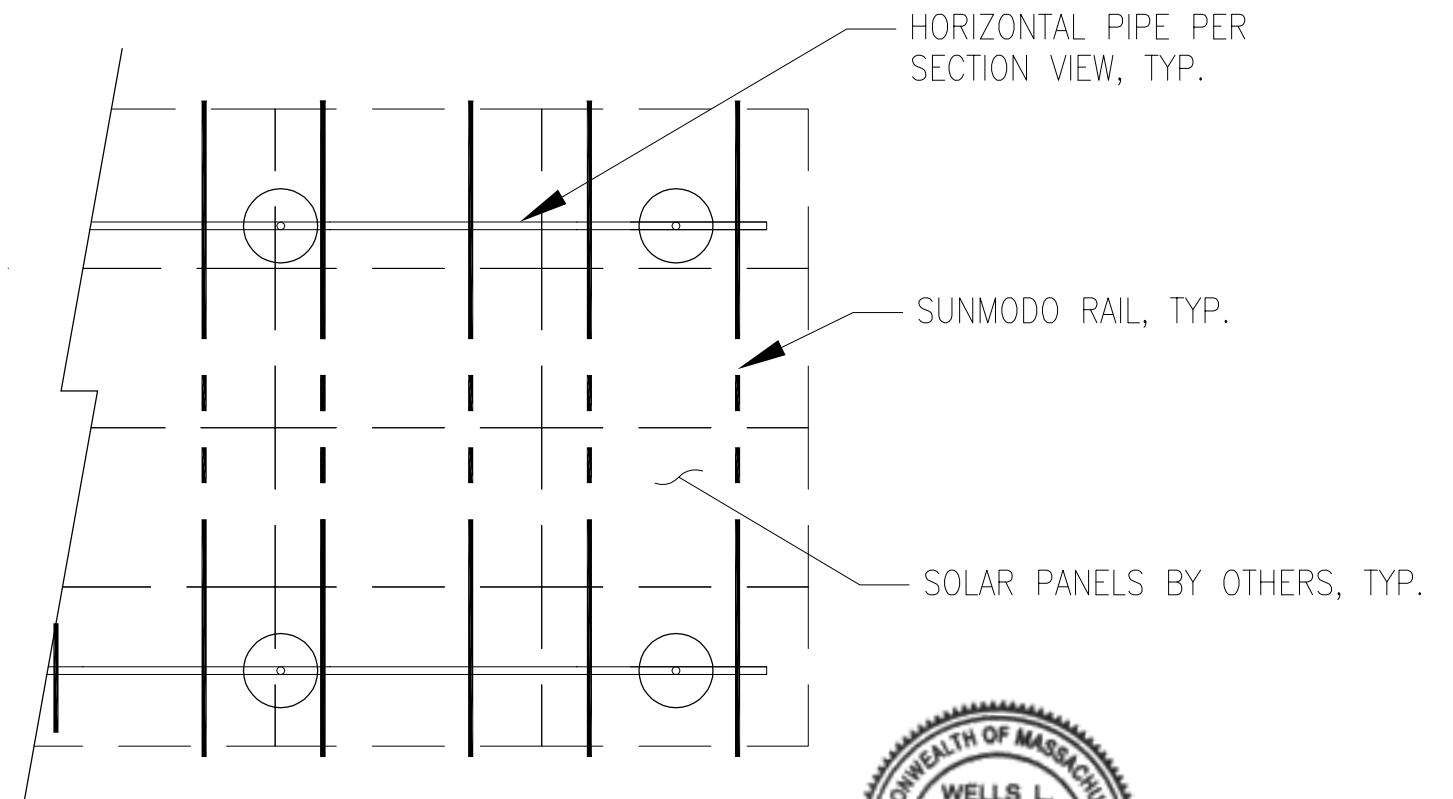
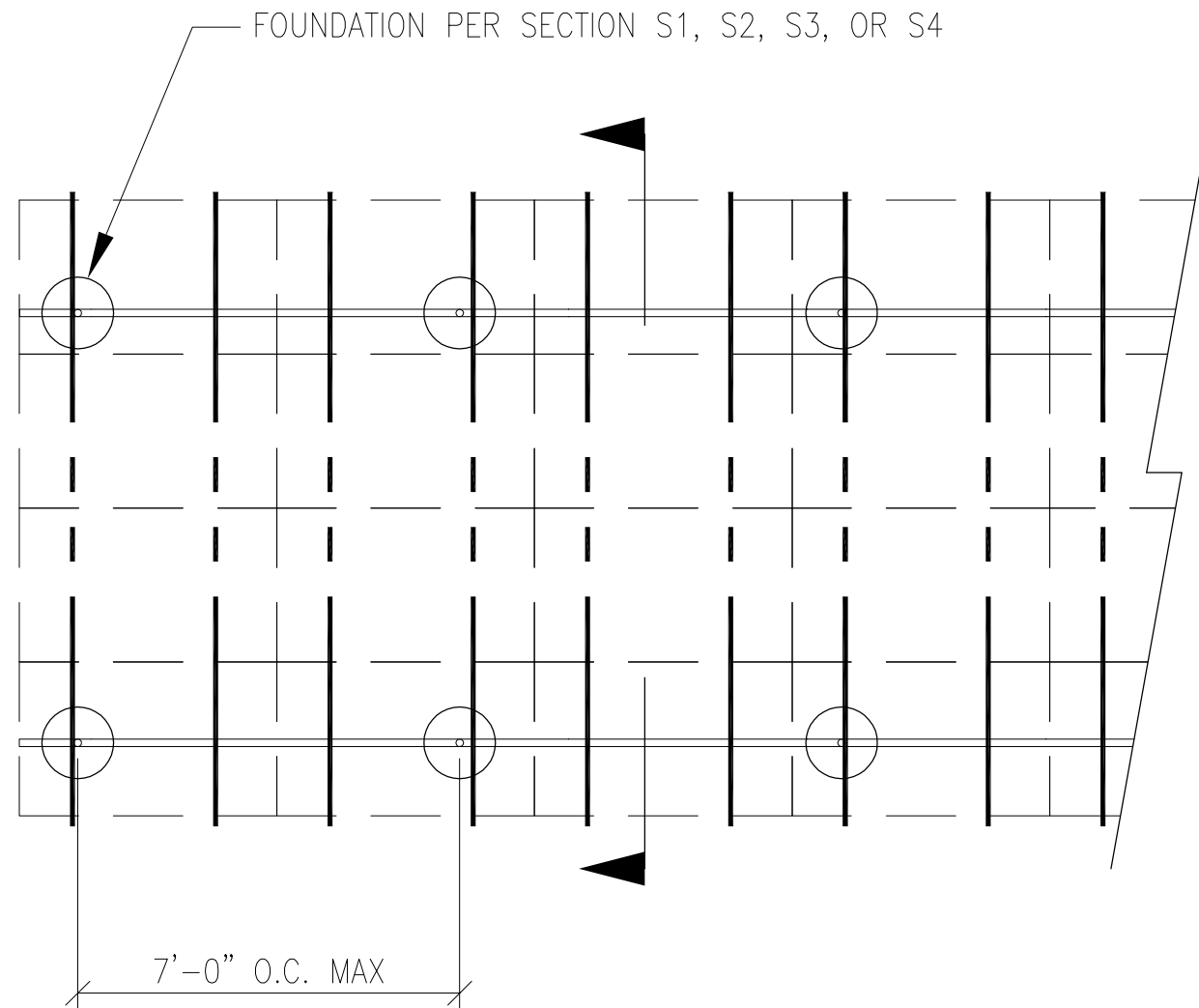


05/10/2023



JOB NO. U2716-0231-201  
 PROJECT SUNMODO SUNTURF GROUND MOUNTS A5a  
 SUBJECT ALL OPTIONS

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**PV ARRAY PLAN**

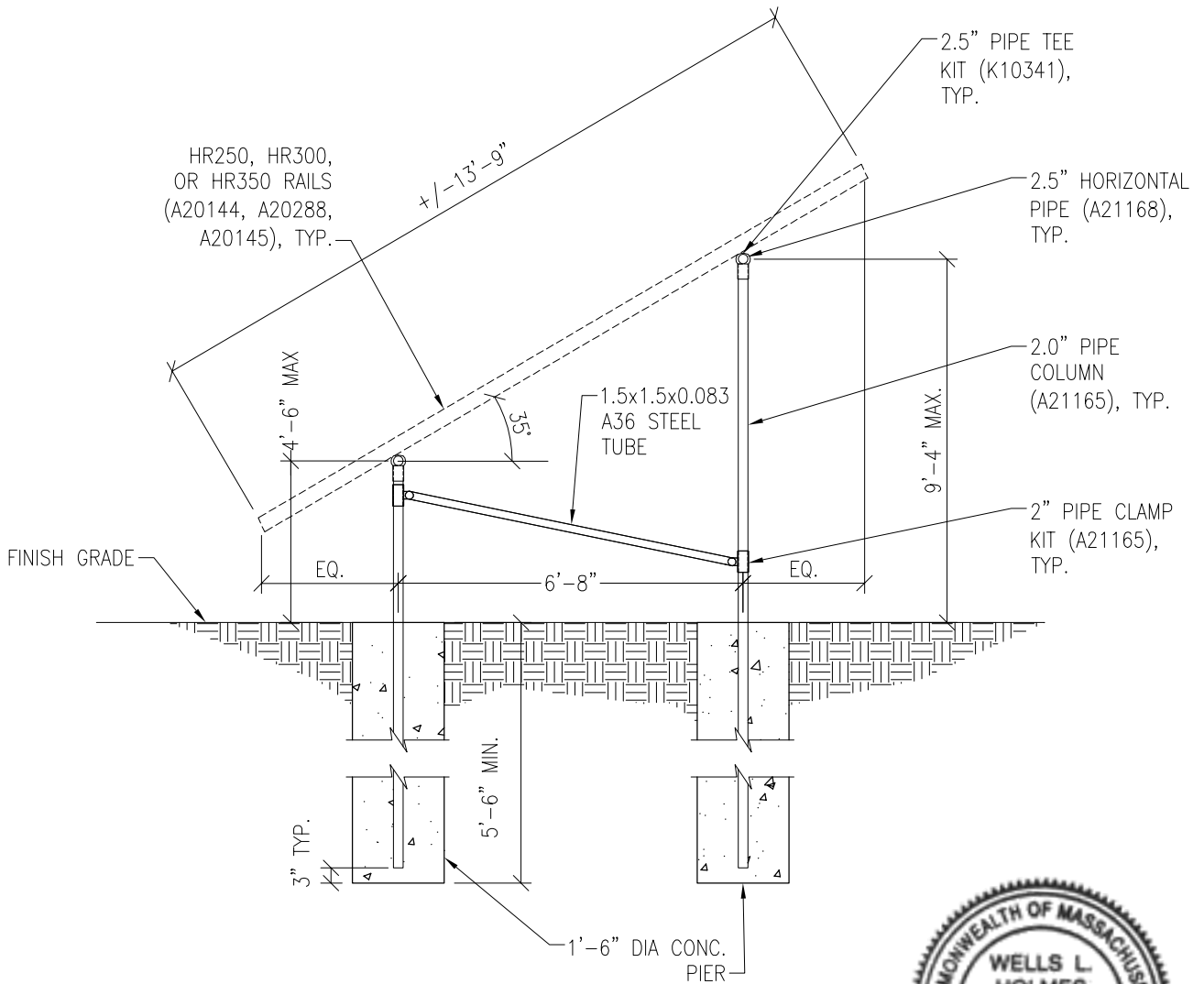
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**P1**

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PROJECT SUNMODO SUNTURF GROUND MOUNTS A5a

SUBJECT DRILLED PIER OPTION



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**PV ARRAY SECTION**

N.T.S.

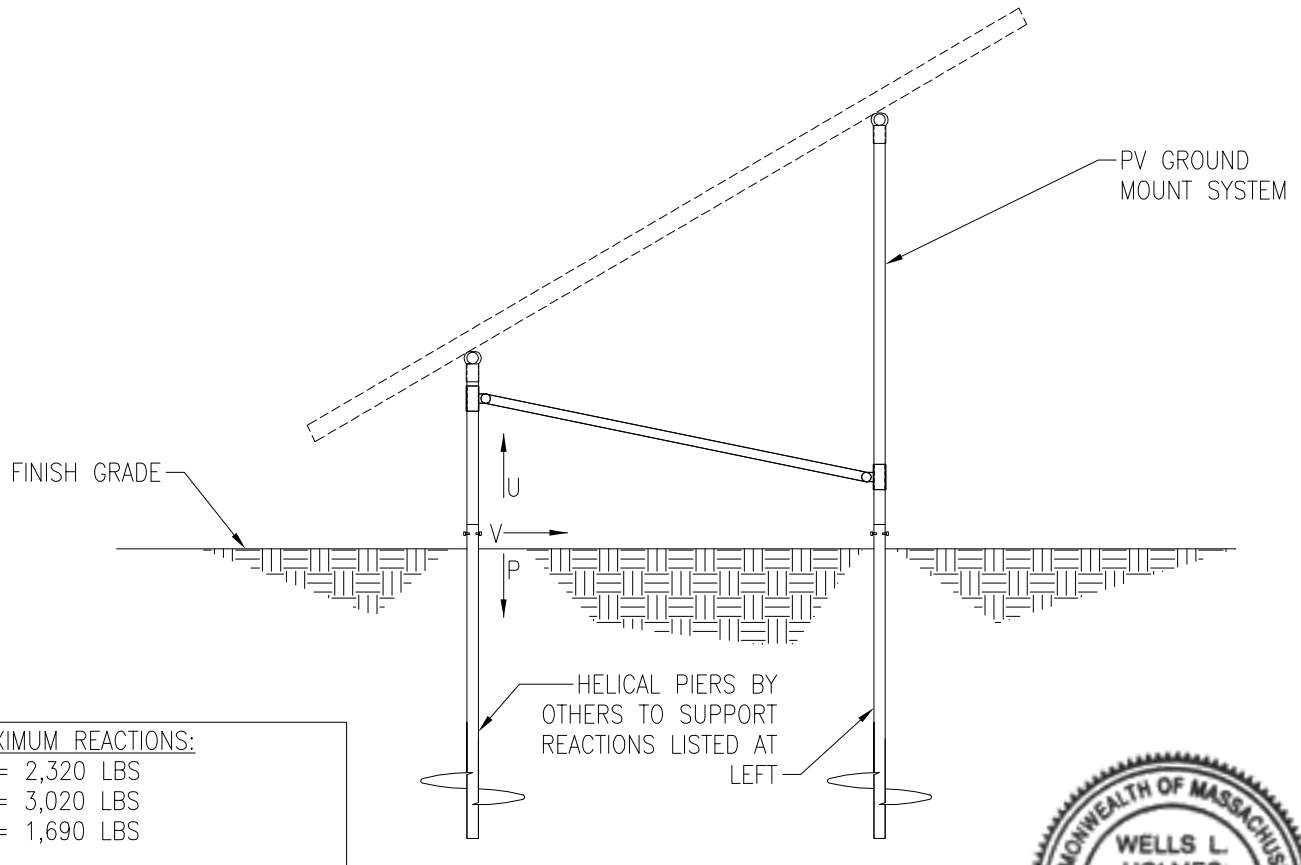
**S1**

PROJECT SUNMODO SUNTURF GROUND MOUNTS A5a

SUBJECT HELICAL PIER OPTION

**NOTES:**

1. For ground mount components see Section S1.
2. A minimum of (1) helical pier must be load-tested as follows:
  - 2.1. Safety factor for uplift = 1.5,
  - 2.2. Safety factor for lateral loads = 2.0
  - 2.3. Upward deflection limit = 1/2"
  - 2.4. Lateral deflection limit = 1"
  - 2.5. The load tests must be performed by an approved contractor.



**MAXIMUM REACTIONS:**  
 U = 2,320 LBS  
 P = 3,020 LBS  
 V = 1,690 LBS

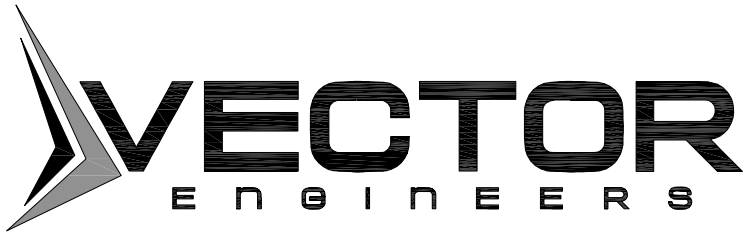


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**PV ARRAY SECTION**

N.T.S.



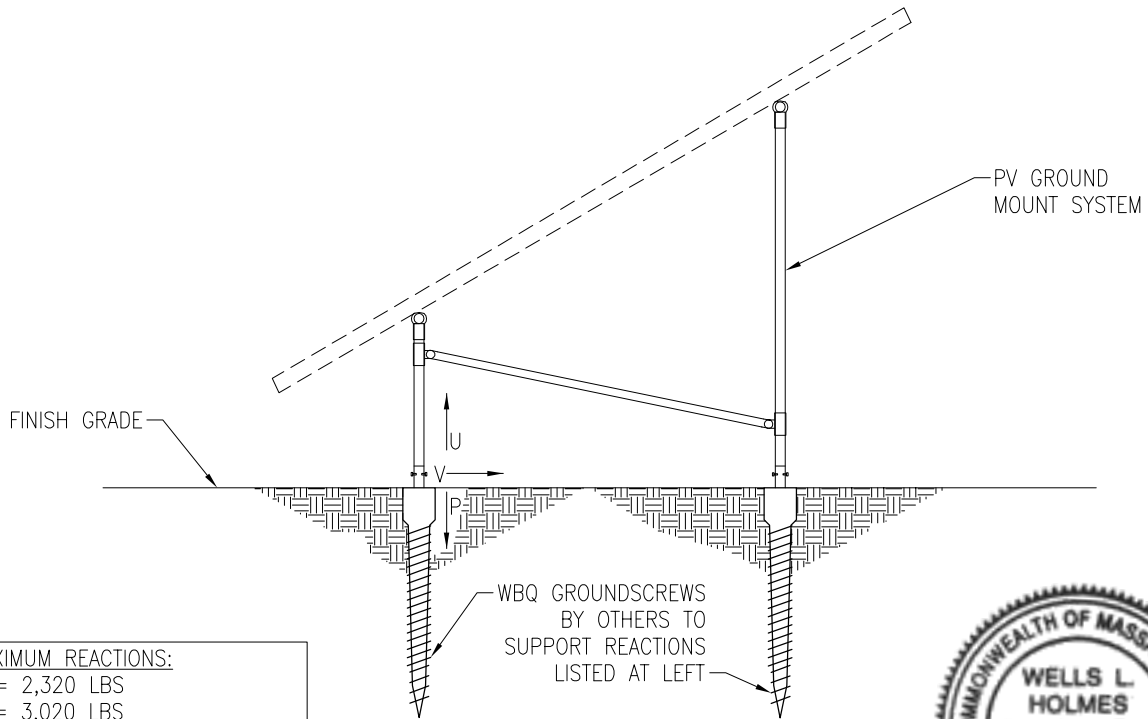
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PROJECT SUNMODO SUNTURF GROUND MOUNTS A5a

SUBJECT GROUND SCREW OPTION

NOTES:

- 1. For ground mount components see Section S1.
- 2. A minimum of (1) ground screw must be load-tested as follows:
  - 2.1. Safety factor for uplift = 1.5,
  - 2.2. Safety factor for lateral loads = 2.0
  - 2.3. Upward deflection limit = 1/2"
  - 2.4. Lateral deflection limit = 1"
  - 2.5. The load tests must be performed by an approved contractor.



MAXIMUM REACTIONS:  
 U = 2,320 LBS  
 P = 3,020 LBS  
 V = 1,690 LBS



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**PV ARRAY SECTION**

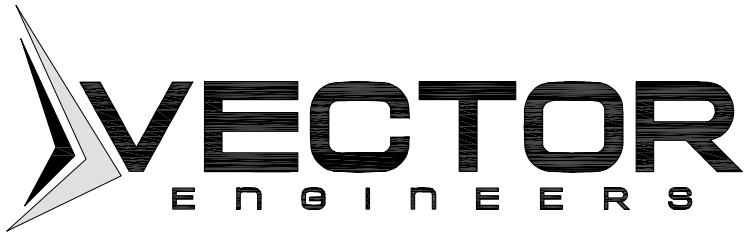
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651 W GALENA PARK BLVD. #101  
DRAPER, UTAH 84020

(801) 990-1775  
(801) 990-1776 FAX

WWW.VECTORSE.COM



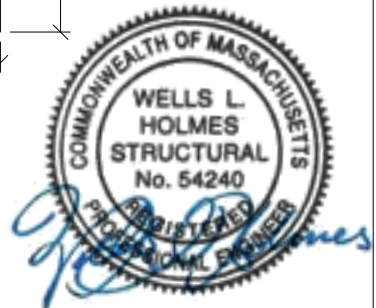
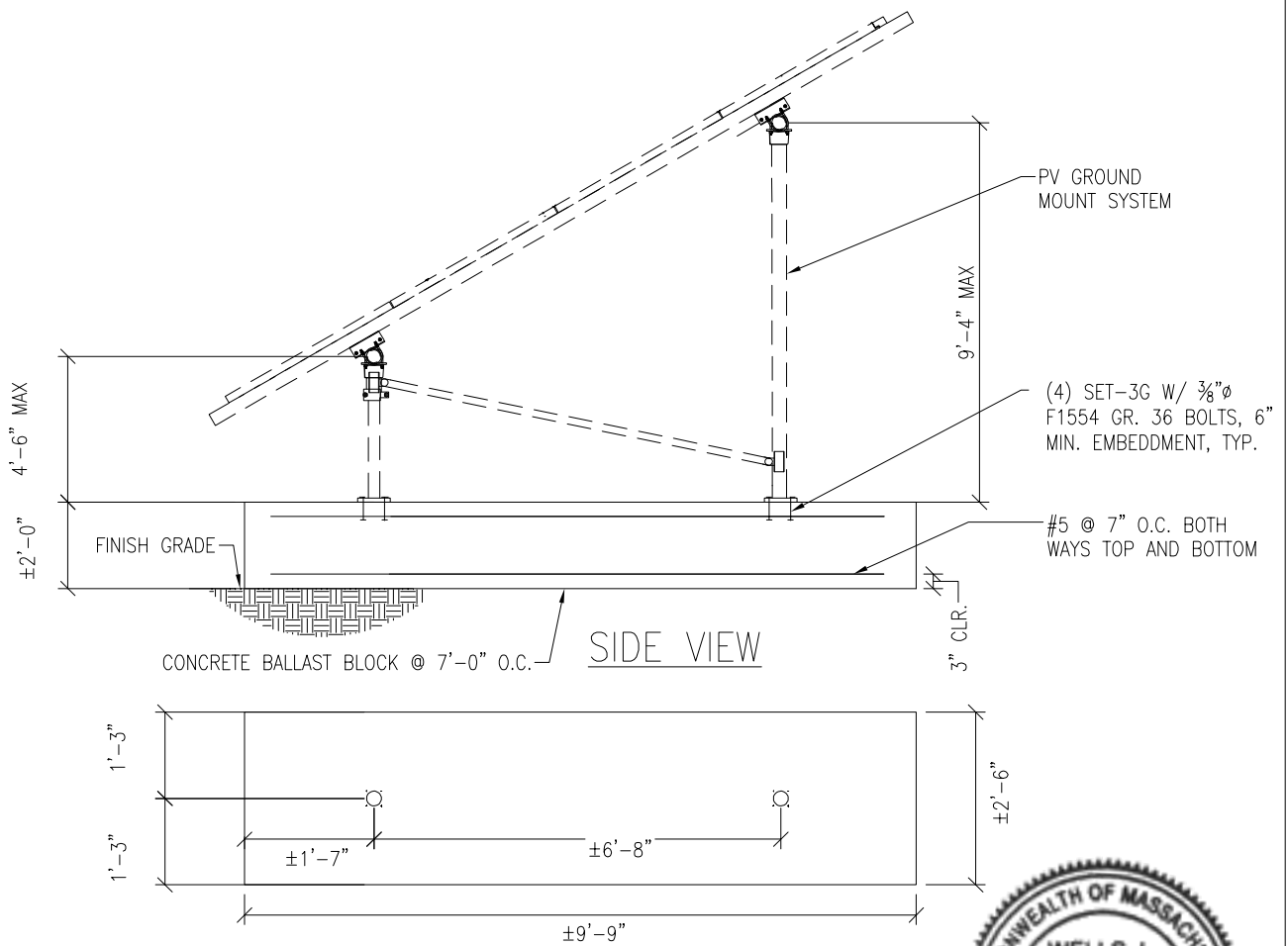
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PROJECT SUNMOD0 SUNTURF GROUND MOUNTS A5a

SUBJECT BALLASTED BLOCK OPTION

NOTES:

1. For ground mount components see Section S1.



PV ARRAY SECTION  
N.T.S.

05/10/2023

S4



**PROJECT:** Sunturf Package A5A Ground Mount

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**SNOW LOADS**

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Calculations Per:	ASCE 7-16	
Snow Ground Load, $p_g$ [psf]:	70.0	(Section 7.2)
Risk Category:	I	(Table 1.5-1)
Importance Factor, $I_s$ :	0.8	(Table 1.5-2)
Terrain Category:	B	(Section 26.7)
Exposure of Roof:	Fully Exposed	(Table 7-3.1)
Exposure Factor, $C_e$ :	0.9	(Table 7-3.1)
Thermal Factor, $C_t$ :	1.2	(Table 7-3.2)
Flat Roof Snow Load, $p_f$ [psf]:	42	(Equation 7.3-1)
Min. Roof Snow Load, $p_m$ [psf]:	0	(Section 7.3.4)
Panel Slope from Horizontal [°]:	35.0	
Unobstructed Slippery Surface?	Yes	(Section 7.4)
Slope Factor Figure:	Figure 7-2c	(of Figure 7.4-1 - See Section 7.4)
Roof Slope Factor, $C_s$ :	0.64	
Sloped Roof Snow Load, $p_s$ [psf]:	27	(Equation 7.4-1)
Design Snow Load, $S$ [psf]:	27	(1.0 Snow)



**PROJECT:** Sunturf Package A5A Ground Mount

**WIND PRESSURES**

Calculations per:	ASCE 7-16	
Design Wind Speed, V [mph]:	140	
Risk Category:	I	(Table 1.5-1)
Exposure Category:	B	(Section 26.7)
Ground Elevation Factor, $K_e$ :	1.0	(Table 26.9-1)
$\alpha$ :	7.0	(Table 26.11-1)
$z_g$ [ft]:	1200	(Table 26.11-1)
Velocity Pressure Exposure Coefficient, $K_h$ :	0.57	(Table 26.10-1)
Topographic Factor, $K_{ht}$ :	1.00	(Section 26.8)
Wind Directionality Factor, $K_d$ :	0.85	(Table 26.6-1)
Internal Pressure Coefficient, $GC_{pi}$ :	0.0	(Figure 26.13-1)
Velocity Pressure, $q_h$ [psf]:	24.31	(Equation 26.10-1)
Gust Effect Factor, G:	0.85	(Section 26.11.4)
Panel Slope [degrees]:	35.0	
Wind Flow:	Clear	
Roof Configuration:	Monoslope	

Wind Pressures in Transverse (N-S) Direction

Net Pressure Coefficients per Figure 27.3-4

Clear Wind Flow	$C_{NW}$	$C_{NL}$
Case 1 ( $\gamma = 0^\circ$ , Load Case A)	-1.8	-1.8
Case 2 ( $\gamma = 0^\circ$ , Load Case B)	-2.4	-0.6
Case 3 ( $\gamma = 180^\circ$ , Load Case A)	2.1	2.1
Case 4 ( $\gamma = 180^\circ$ , Load Case B)	2.7	1.1

Design Wind Pressures per Equation 27.3-2 [psf]

Clear Wind Flow	$q_h GC_{NW}$	$q_h GC_{NL}$
Case 1 ( $\gamma = 0^\circ$ , Load Case A)	-37.2	-37.2
Case 2 ( $\gamma = 0^\circ$ , Load Case B)	-49.6	-12.4
Case 3 ( $\gamma = 180^\circ$ , Load Case A)	43.4	43.4
Case 4 ( $\gamma = 180^\circ$ , Load Case B)	55.8	22.7
Case 5 ( $\gamma = 0^\circ$ , 16 psf Min. Horiz.)	-16.0	-16.0
Case 6 ( $\gamma = 180^\circ$ , 16 psf Min. Horiz.)	16.0	16.0

Wind Pressures in Longitudinal (E-W) Direction





PROJECT: Sunturf Package A5A Ground Mount

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**WIND PRESSURES**

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Net Pressure Coefficients per Figure 27.3-7

Clear Wind Flow	$C_N$
Case 1 ( $\gamma = 90^\circ$ , $d < h$ , Load Case A)	-0.80
Case 2 ( $\gamma = 90^\circ$ , $h < d < 2h$ , Load Case A)	-0.60
Case 3 ( $\gamma = 90^\circ$ , $d > 2h$ , Load Case A)	-0.30
Case 4 ( $\gamma = 90^\circ$ , $d < h$ , Load Case B)	0.80
Case 5 ( $\gamma = 90^\circ$ , $h < d < 2h$ , Load Case B)	0.50
Case 6 ( $\gamma = 90^\circ$ , $d > 2h$ , Load Case B)	0.30

Design Wind Pressures per Equation 27.3-2 [psf]

Clear Wind Flow	$q_h GC_N$
Case 1 ( $\gamma = 90^\circ$ , $d < h$ , Load Case A)	-16.5
Case 2 ( $\gamma = 90^\circ$ , $h < d < 2h$ , Load Case A)	-12.4
Case 3 ( $\gamma = 90^\circ$ , $d > 2h$ , Load Case A)	-6.2
Case 4 ( $\gamma = 90^\circ$ , $d < h$ , Load Case B)	16.5
Case 5 ( $\gamma = 90^\circ$ , $h < d < 2h$ , Load Case B)	10.3
Case 6 ( $\gamma = 90^\circ$ , $d > 2h$ , Load Case B)	6.2

*Notation:*

$h$  = mean roof height, ft

$d$  = horizontal distance from windward edge



JOB NO.: U2716-0231-201

DESIGNED: STB

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# Foundation Option 1: Drilled Concrete Pier



PROJECT: Sunturf Package A5A Ground Mount

**DRILLED CONCRETE PIER DESIGN**

**Column Reactions:**

Max. Shear, V [k]:	1.7	Max. Down, P <sub>d</sub> [k]:	3.0
Max. Moment, M [k-ft]:	0.0	Max. Uplift, P <sub>u</sub> [k]:	2.3

**Pier Properties:**

Pier Shape:	Round	Volume of Concrete [ft <sup>3</sup> ]:	10
Pier Diameter, b [ft]:	1.5	Volume of Concrete [yd <sup>3</sup> ]:	0.4
Top of Pier Elevation [ft]:	0.00	Weight of Concrete [k]:	1.5
Pier Depth, d [ft]:	5.5		

**Soil Properties:**

Allow. Bearing Pressure [psf]:	1,500	<u>Optional Parameters for Uplift:</u>	
1/3 increase for short term loads?	No		
Lateral Bearing, S [pcf]:	150	Skin Friction* [psf]:	250
Max. Lateral Bearing (opt'l) [psf]:		Top Length to Ignore [ft]:	0
Top Depth to Ignore [ft]:	0	1/3 increase for short term loads?	No
1/3 increase for short term loads?	No	Combine w/ Bearing:	No
1/2" deflection at t/o pier allowed:	Yes		

\*per IBC Section 1810.3.3.1.4

**Check Bearing:**

Bearing Capacity [k]:	6.5
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**Bearing capacity OK.**

**Check Uplift:**

Uplift Capacity [k]:	7.8
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**Uplift capacity OK.**

**Check Lateral Bearing:**

Top of Pier Constrained?:	No	IBC Section 1807.3.2.1
Applied Lateral Force, P [lb]:	1,690	
Point of Application, h [ft]:	0.0	
S <sub>max</sub> [psf]:		
S [psf]:	550	
A = 2.34*P/(S <sub>b</sub> ):	4.79	
Required Pier Depth, d <sub>reqd</sub> [ft]:	4.80	IBC Eq. 18-1

Result: **Lateral bearing capacity OK.**

# Foundation Option 2: Helical Pier

The ground screws and helical piers must be tested to 1.5 times uplift and 2.0 times lateral reactions found in the table below.

Load (ASD)	Value (lbs)	Factor of Safety	Test Value (lbs)
UPLIFT	2320	1.5	3480
LATERAL	1690	2	3380

# Foundation Option 3: Ground Screw

The ground screws and helical piers must be tested to 1.5 times uplift and 2.0 times lateral reactions found in the table below.

Load (ASD)	Value (lbs)	Factor of Safety	Test Value (lbs)
UPLIFT	2320	1.5	3480
LATERAL	1690	2	3380

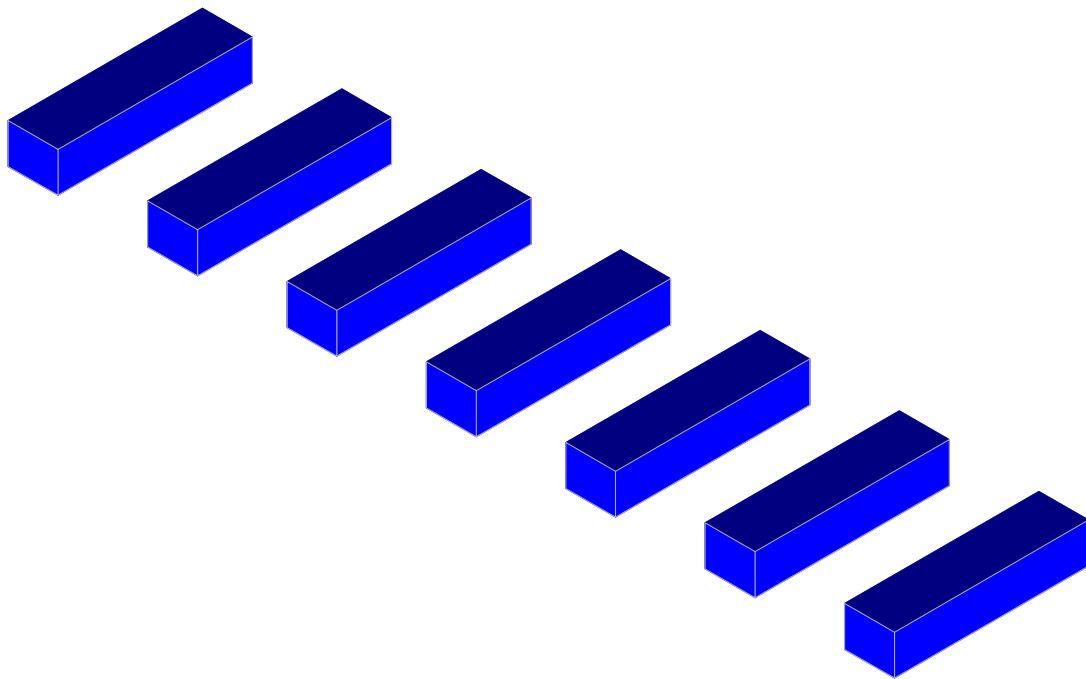


JOB NO.: U2716-0231-201

DESIGNED: STB

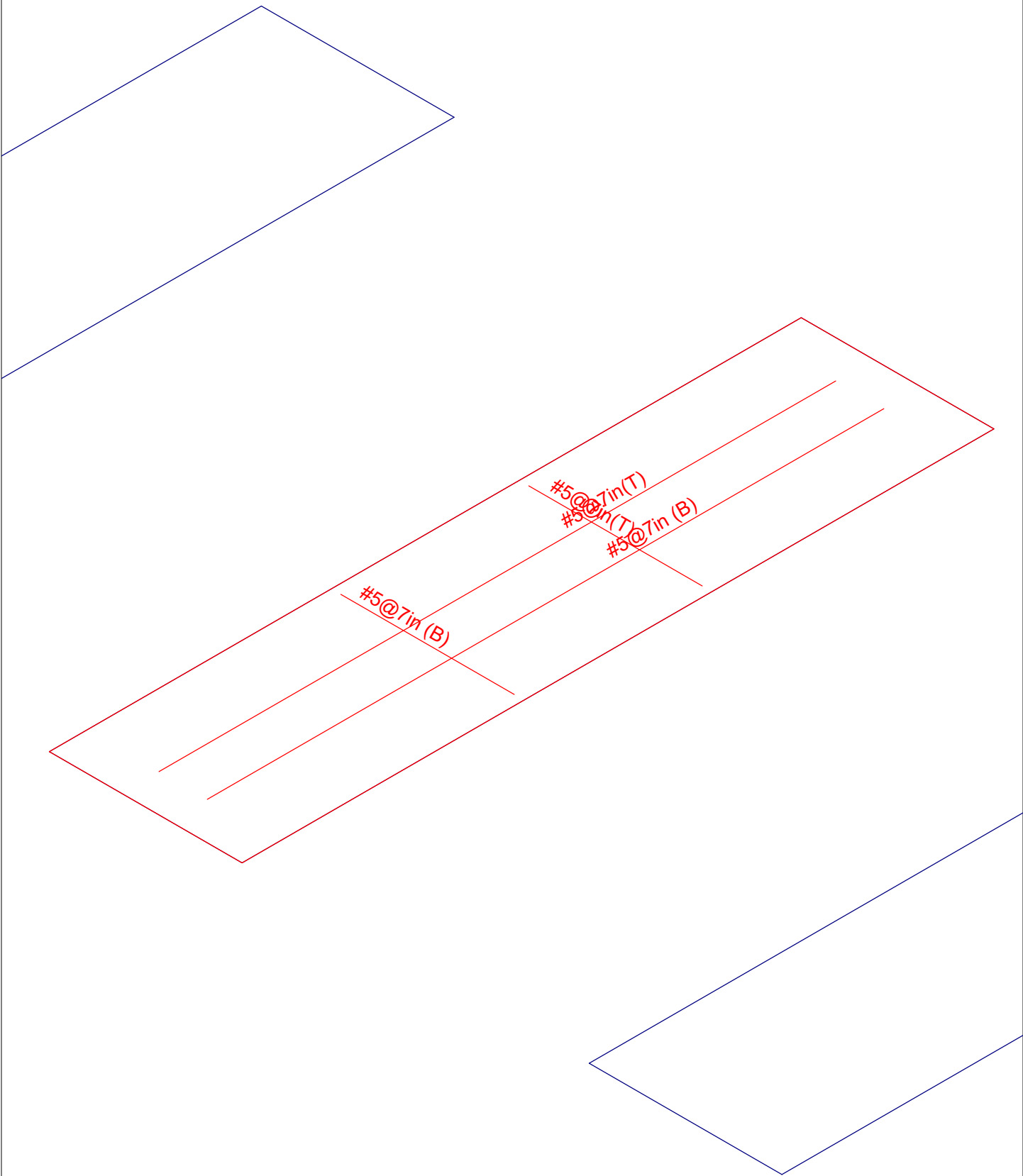
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# Foundation Option 4: Ballasted Block



Results for LC 1, 1.0 D

Vector Structural Engineeri..	Ground Mount	SK - 2
STB		July 22, 2020 at 10:33 AM
U2716.0231.201		Sunmodo Sunturf A5a.r3d



Results for LC 1, 1.0 D

Vector Structural Engineeri...	Ground Mount	SK - 1
STB		July 22, 2020 at 10:33 AM
U2716.0231.201		Sunmodo Sunturf A5a.r3d





**(Global) Model Settings**

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	100
Mesh Size (in)	12
Max Iterations	10
Merge Tolerance (in)	.12
Solver	Sparse Accelerated
Coefficient of Friction	.3
No. of Shear Regions	4
Shear Region Spacing Increment (in)	4
Min 1 Bar Dia Spacing for Beams?	No
Optimize footings for OTM / Sliding?	Yes
Parme Beta Factor	.65
Pile Safety Factor	3
Concrete Stress Block	0
Concrete Rebar Set	Rectangular
Concrete Code	ASTM A615
HR Steel Pile Code	ACI 318-14
Wood Pile Code	AISC 14th (360-10): ASD AWC NDS-15: ASD

**Concrete Properties**

	Label	E [ksi]	G [ksi]	Nu	Therm (/...	Density[lb/ft^3]	fc[psi]	Lambda	Flex Stee...	Shear St...
1	Conc3000NW	3156	1372	.15	.6	145	3000	1	60000	60000
2	Conc3500NW	3409	1482	.15	.6	145	3500	1	60000	60000
3	Conc4000NW	3644	1584	.15	.6	145	4000	1	60000	60000
4	Conc3000LW	2085	907	.15	.6	109.999	3000	.75	60000	60000
5	Conc3500LW	2252	979	.15	.6	109.999	3500	.75	60000	60000
6	Conc4000LW	2408	1047	.15	.6	109.999	4000	.75	60000	60000
7	Conc2500NW	3156	1372	.15	.6	145	2500	1	60000	60000

**General Design Parameters**

	Label	Max Bending Chk	Max Shear Chk	Top Cover[in]	Bottom Cover[in]
1	Typical	1	1	3	3

**Slab Rebar Parameters**

	Label	Top Bar	Bottom Bar	Max Top Bar Sp...	Min Top Bar Sp...	Max Bot Bar Sp...	Min Bot Bar Sp...	Spacing Incr...	Rebar Options
1	Typical	#5	#5	18	3	18	3	1	Optimize

**Soil Definitions**

	Label	Subgrade Modulus[lb/ft^3]	Allowable Bearing[psf]	Depth Properties	Default?
1	Default	1e+5	1500	None	Yes

**Point Loads and Moments (Cat 1 : DL)**

	Label	Direction	Magnitude[lb,lb-ft]
1	R3D_N1_1	X	-11.981
2	R3D_N1_1	Y	216.315
3	R3D_N2	Y	183.998
4	R3D_N132_2	Y	210.689
5	R3D_N133_1	X	-1.035
6	R3D_N133_1	Y	201.84
7	R3D_N109_1	Y	194.866
8	R3D_N110A_1	Y	206.883

**Point Loads and Moments (Cat 1 : DL) (Continued)**

	Label	Direction	Magnitude[lb,lb-ft]
9	R3D_N121_1	Y	213.292
10	R3D_N122_1	Y	204.065
11	R3D_N133B	Y	210.369
12	R3D_N134B_1	X	-1.088
13	R3D_N134B_1	Y	204.722
14	R3D_N151_1	Y	208.076
15	R3D_N152_1	X	-2.806
16	R3D_N152_1	Y	216.842
17	R3D_N143A	X	13.773
18	R3D_N143A	Y	172.004
19	R3D_N144A	X	6.043
20	R3D_N144A	Y	138.199

**Point Loads and Moments (Cat 6 : RLL)**

	Label	Direction	Magnitude[lb,lb-ft]
1	R3D_N1_1	X	-67.331
2	R3D_N1_1	Y	1031.223
3	R3D_N2	X	-2.839
4	R3D_N2	Y	965.955
5	R3D_N132_2	X	-2.393
6	R3D_N132_2	Y	1044.539
7	R3D_N133_1	X	-6.442
8	R3D_N133_1	Y	1072.806
9	R3D_N109_1	X	-2.92
10	R3D_N109_1	Y	942.536
11	R3D_N109_1	Z	-1.733
12	R3D_N110A_1	Y	1106.213
13	R3D_N121_1	Y	1059.791
14	R3D_N121_1	Z	2.094
15	R3D_N122_1	X	-3.297
16	R3D_N122_1	Y	1086.093
17	R3D_N133B	X	-2.058
18	R3D_N133B	Y	1040.863
19	R3D_N133B	Z	-1.423
20	R3D_N134B_1	X	-6.539
21	R3D_N134B_1	Y	1090.394
22	R3D_N151_1	X	-2.829
23	R3D_N151_1	Y	1020.851
24	R3D_N152_1	X	-17.111
25	R3D_N152_1	Y	1164.394
26	R3D_N143A	X	78.066
27	R3D_N143A	Y	772.29
28	R3D_N144A	X	35.641
29	R3D_N144A	Y	694.667

**Point Loads and Moments (Cat 16 : OL1)**

	Label	Direction	Magnitude[lb,lb-ft]
1	R3D_N1_1	X	121.18
2	R3D_N1_1	Y	-3361.819
3	R3D_N1_1	Z	1910.216
4	R3D_N2	X	12.423
5	R3D_N2	Y	568.067
6	R3D_N2	Z	-95.762
7	R3D_N132_2	X	3.763
8	R3D_N132_2	Y	-3551.734
9	R3D_N132_2	Z	2117.481



**Point Loads and Moments (Cat 16 : OL1) (Continued)**

	Label	Direction	Magnitude[lb.lb-ft]
10	R3D_N133_1	X	1.431
11	R3D_N133_1	Y	627.579
12	R3D_N133_1	Z	-102.734
13	R3D_N109_1	X	2.365
14	R3D_N109_1	Y	-3453.856
15	R3D_N109_1	Z	2210.996
16	R3D_N110A_1	X	6.64
17	R3D_N110A_1	Y	680.656
18	R3D_N110A_1	Z	-102.659
19	R3D_N121_1	Y	-3599.346
20	R3D_N121_1	Z	2143.35
21	R3D_N122_1	X	3.848
22	R3D_N122_1	Y	636.796
23	R3D_N122_1	Z	-103.219
24	R3D_N133B	X	2.595
25	R3D_N133B	Y	-3579.127
26	R3D_N133B	Z	2166.706
27	R3D_N134B_1	X	3.625
28	R3D_N134B_1	Y	647.146
29	R3D_N134B_1	Z	-103.532
30	R3D_N151_1	X	2.639
31	R3D_N151_1	Y	-3729.308
32	R3D_N151_1	Z	2375.203
33	R3D_N152_1	X	17.318
34	R3D_N152_1	Y	755.333
35	R3D_N152_1	Z	-101.362
36	R3D_N143A	X	-131.673
37	R3D_N143A	Y	-2443.918
38	R3D_N143A	Z	1359.813
39	R3D_N144A	X	-45.309
40	R3D_N144A	Y	386.94
41	R3D_N144A	Z	-78.339

**Point Loads and Moments (Cat 17 : OL2)**

	Label	Direction	Magnitude[lb.lb-ft]
1	R3D_N1_1	X	155.556
2	R3D_N1_1	Y	-3751.678
3	R3D_N1_1	Z	1616.16
4	R3D_N2	X	4.573
5	R3D_N2	Y	1305.88
6	R3D_N2	Z	-80.939
7	R3D_N132_2	X	4.785
8	R3D_N132_2	Y	-3954.679
9	R3D_N132_2	Z	1788.55
10	R3D_N133_1	X	-4.396
11	R3D_N133_1	Y	1473.183
12	R3D_N133_1	Z	-86.691
13	R3D_N109_1	X	3.168
14	R3D_N109_1	Y	-3798.157
15	R3D_N109_1	Z	1860.522
16	R3D_N110A_1	X	12.662
17	R3D_N110A_1	Y	1561.76
18	R3D_N110A_1	Z	-86.49
19	R3D_N121_1	Y	-4007.832
20	R3D_N121_1	Z	1808.577
21	R3D_N122_1	Y	1491.677

















**Slab Overturning Safety Factors (Continued)**

	LC	Slab	Angle[deg]	Mo-xx[lb-ft]	Ms-xx[lb-ft]	Mo-zz[lb-ft]	Ms-zz[lb-ft]	Ms-xx/Mo-xx	Ms-zz/Mo-zz
69	10	S6	0	15645.102	44047.529	1359.516	11380.246	2.815	8.371
70	10	S7	0	10416.656	41584.076	1182.282	10809.17	3.992	9.143
71	11	S1	0	1684.748	43522.171	0	13347.424	9.999+	9.999+
72	11	S2	0	2438.482	44436.939	0	13120.844	9.999+	9.999+
73	11	S3	0	2043.134	44463.357	0	13354.88	9.999+	9.999+
74	11	S4	0	2131.961	44463.011	0	13330.055	9.999+	9.999+
75	11	S5	0	2014.639	44337.418	0	13308.201	9.999+	9.999+
76	11	S6	0	2716.257	44995.62	0	13454.102	9.999+	9.999+
77	11	S7	0	1069.306	41088.097	0	11572.636	9.999+	9.999+
78	12	S1	0	0	44214.243	0	12996.619	9.999+	9.999+
79	12	S2	0	0	45019.388	0	12917.023	9.999+	9.999+
80	12	S3	0	0	45228.166	0	13083.978	9.999+	9.999+
81	12	S4	0	0	45189.721	0	13075.029	9.999+	9.999+
82	12	S5	0	0	45101.136	0	13042.8	9.999+	9.999+
83	12	S6	0	0	45462.58	0	13242.059	9.999+	9.999+
84	12	S7	0	0	41706.753	0	11398.806	9.999+	9.999+
85	13	S1	0	603.177	43522.171	0	12049.114	9.999+	9.999+
86	13	S2	0	694.784	44436.939	0	11844.522	9.999+	9.999+
87	13	S3	0	379.059	44463.357	0	11749.157	9.999+	9.999+
88	13	S4	0	273.163	44463.011	0	11636.055	9.999+	9.999+
89	13	S5	0	261.884	44337.418	0	11630.754	9.999+	9.999+
90	13	S6	0	378.83	44995.62	0	11721.375	9.999+	9.999+
91	13	S7	0	132.023	41088.097	0	10556.292	9.999+	9.999+
92	14	S1	0	5802.916	43905.827	706.387	11337.915	7.566	9.999+
93	14	S2	0	4545.314	43382.164	508.49	11254.46	9.544	9.999+
94	14	S3	0	2992.834	44218.23	343.48	11364.469	9.999+	9.999+
95	14	S4	0	2394.109	44089.527	272.989	11367.942	9.999+	9.999+
96	14	S5	0	2420.997	44088.413	277.596	11351.949	9.999+	9.999+
97	14	S6	0	2475.7	44047.529	270.957	11451.292	9.999+	9.999+
98	14	S7	0	1706.786	41584.076	236.218	10809.17	9.999+	9.999+
99	15	S1	0	18139.031	21901.712	2255.637	5616.183	1.207	2.49
100	15	S2	0	18841.345	21817.207	2090.707	5602.883	1.158	2.68
101	15	S3	0	19511.873	21904.96	2226.53	5614.589	1.123	2.522
102	15	S4	0	19431.134	21891.229	2206.449	5614.196	1.127	2.544
103	15	S5	0	19256.408	21890.202	2199.349	5612.211	1.137	2.552
104	15	S6	0	20322.454	21890.902	2254.43	5623.628	1.077	2.494
105	15	S7	0	13164.748	21643.367	1755.112	5558.003	1.644	3.167
106	16	S1	0	19050.227	21901.712	2026.504	5616.183	1.15	2.771
107	16	S2	0	19334.253	21817.207	1696.293	5602.883	1.128	3.303
108	16	S3	0	20361.776	21904.96	1887.116	5614.589	1.076	2.975
109	16	S4	0	20212.348	21891.229	1850.843	5614.196	1.083	3.033
110	16	S5	0	20094.323	21890.202	1861.589	5612.211	1.089	3.015
111	16	S6	0	20860.137	21890.902	1818.697	5623.628	1.049	3.092
112	16	S7	0	13888.874	21643.367	1576.377	5558.003	1.558	3.526
113	17	S1	0	2246.331	21792.451	0	8295.529	9.701	9.999+
114	17	S2	0	3251.309	21885.355	0	8079.715	6.731	9.999+
115	17	S3	0	2724.178	21888.912	0	8255.282	8.035	9.999+
116	17	S4	0	2842.615	21889.39	0	8230.346	7.7	9.999+
117	17	S5	0	2686.185	21875.428	0	8220.548	8.144	9.999+
118	17	S6	0	3621.676	21947.205	0	8294.042	6.06	9.999+
119	17	S7	0	1425.741	21523.653	0	7088.911	9.999+	9.999+
120	18	S1	0	0	22715.213	0	7827.789	9.999+	9.999+
121	18	S2	0	0	22661.953	0	7807.953	9.999+	9.999+
122	18	S3	0	0	22908.658	0	7894.079	9.999+	9.999+
123	18	S4	0	0	22858.337	0	7890.312	9.999+	9.999+
124	18	S5	0	0	22893.718	0	7866.68	9.999+	9.999+
125	18	S6	0	0	22569.819	0	8011.317	9.999+	9.999+







**Slab Sliding Safety Factors (Continued)**

	LC	Slab	Angle[deg]	Va-xx[lb]	Vr-xx[lb]	Va-zz[lb]	Vr-zz[lb]	SR-xx	SR-zz
96	14	S5	0	7.149	2654.413	151.082	2654.413	9.999+	9.999+
97	14	S6	0	16.777	2675.227	170.462	2675.227	9.999+	9.999+
98	14	S7	0	87.894	2495.319	96.19	2495.319	9.999+	9.999+
99	15	S1	0	72.973	841.558	1088.672	841.558	9.999+	.773
100	15	S2	0	5.403	845.516	1265.002	845.516	9.999+	.668
101	15	S3	0	2.309	814.243	1224.078	814.243	9.999+	.665
102	15	S4	0	3.079	819.337	1237.904	819.337	9.999+	.662
103	15	S5	0	2.495	820.285	1208.848	820.285	9.999+	.679
104	15	S6	0	10.291	813.547	1364.305	813.547	9.999+	.596
105	15	S7	0	94.3	957.958	768.884	957.958	9.999+	1.246
106	16	S1	0	88.889	904.19	921.133	904.19	9.999+	.982
107	16	S2	0	9.498	942.14	1064.419	942.14	9.999+	.885
108	16	S3	0	0	894.594	1032.938	894.594	9.999+	.866
109	16	S4	0	.17	903.123	1044.09	903.123	9.999+	.865
110	16	S5	0	.388	899.963	1021.115	899.963	9.999+	.881
111	16	S6	0	.181	913.096	1147.845	913.096	9.999+	.795
112	16	S7	0	95.735	1001.543	651.209	1001.543	9.999+	1.538
113	17	S1	0	103.28	1941.352	1291.108	1941.352	9.999+	1.504
114	17	S2	0	6.587	1935.97	1500.011	1935.97	9.999+	1.291
115	17	S3	0	2.681	1979.981	1451.592	1979.981	9.999+	1.364
116	17	S4	0	4.982	1972.892	1467.969	1972.892	9.999+	1.344
117	17	S5	0	4.23	1970.901	1433.569	1970.901	9.999+	1.375
118	17	S6	0	15.571	1983.096	1617.758	1983.096	9.999+	1.226
119	17	S7	0	138.469	1767.804	911.88	1767.804	9.999+	1.939
120	18	S1	0	65.154	1847.395	1116.256	1847.395	9.999+	1.655
121	18	S2	0	.855	1873.499	1302.119	1873.499	9.999+	1.439
122	18	S3	0	3.702	1892.802	1257.462	1892.802	9.999+	1.505
123	18	S4	0	6.773	1890.424	1272.106	1890.424	9.999+	1.486
124	18	S5	0	5.877	1885.182	1240.934	1885.182	9.999+	1.519
125	18	S6	0	21.361	1912.463	1404.448	1912.463	9.999+	1.362
126	18	S7	0	105.525	1696.365	787.807	1696.365	9.999+	2.153
127	19	S1	0	34.021	1559.138	475.203	1559.138	9.999+	3.281
128	19	S2	0	1.177	1531.273	466.144	1531.273	9.999+	3.285
129	19	S3	0	4.575	1469.632	281.054	1469.632	9.999+	5.229
130	19	S4	0	2.306	1432.096	199.3	1432.096	9.999+	7.186
131	19	S5	0	1.273	1435.537	201.903	1435.537	9.999+	7.11
132	19	S6	0	2.993	1434.661	227.226	1434.661	9.999+	6.314
133	19	S7	0	35.172	1392.991	128.257	1392.991	9.999+	9.999+
134	20	S1	0	18.68	1130.806	473.083	1130.806	9.999+	2.39
135	20	S2	0	3.064	1183.446	407.252	1183.446	9.999+	2.906
136	20	S3	0	1.541	1238.328	249.795	1238.328	9.999+	4.957
137	20	S4	0	.517	1260.299	203.517	1260.299	9.999+	6.193
138	20	S5	0	.062	1258.13	201.442	1258.13	9.999+	6.246
139	20	S6	0	.371	1262.786	227.283	1262.786	9.999+	5.556
140	20	S7	0	11.047	1263.633	128.254	1263.633	9.999+	9.853

**Envelope Slab Soil Pressures**

	Label	UC	LC	Soil Pressure[psf]	Allowable Bearing[psf]	Point
1	S1	.415	5	622.568	1500	N198
2	S2	.42	5	630.45	1500	N205
3	S3	.43	5	644.419	1500	N212
4	S4	.428	5	642.547	1500	N219
5	S5	.426	5	639.715	1500	N226
6	S6	.466	16	698.629	1500	N38
7	S7	.355	5	532.632	1500	N240



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**1. Project information**

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

**2. Input Data & Anchor Parameters**

**General**

Design method: ACI 318-14  
Units: Imperial units

**Anchor Information:**

Anchor type: Bonded anchor  
Material: F1554 Grade 36  
Diameter (inch): 0.375  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: ICC-ES ESR-4057  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 7.25  
 $c_{ac}$  (inch): 10.99  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

**Base Material**

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 30.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Ignore concrete breakout in tension: No  
Ignore concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 150/110°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

**Base Plate**

Length x Width x Thickness (inch): 4.75 x 4.75 x 0.31

**Recommended Anchor**

Anchor Name: SET-3G - SET-3G w/ 3/8"Ø F1554 Gr. 36  
Code Report: ICC-ES ESR-4057





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**Load and Geometry**

Load factor source: ACI 318 Section 5.3

Load combination: not set

Seismic design: No

Anchors subjected to sustained tension: No

Apply entire shear load at front row: No

Anchors only resisting wind and/or seismic loads: No

Strength level loads:

$N_{ua}$  [lb]: 3870

$V_{uax}$  [lb]: 175

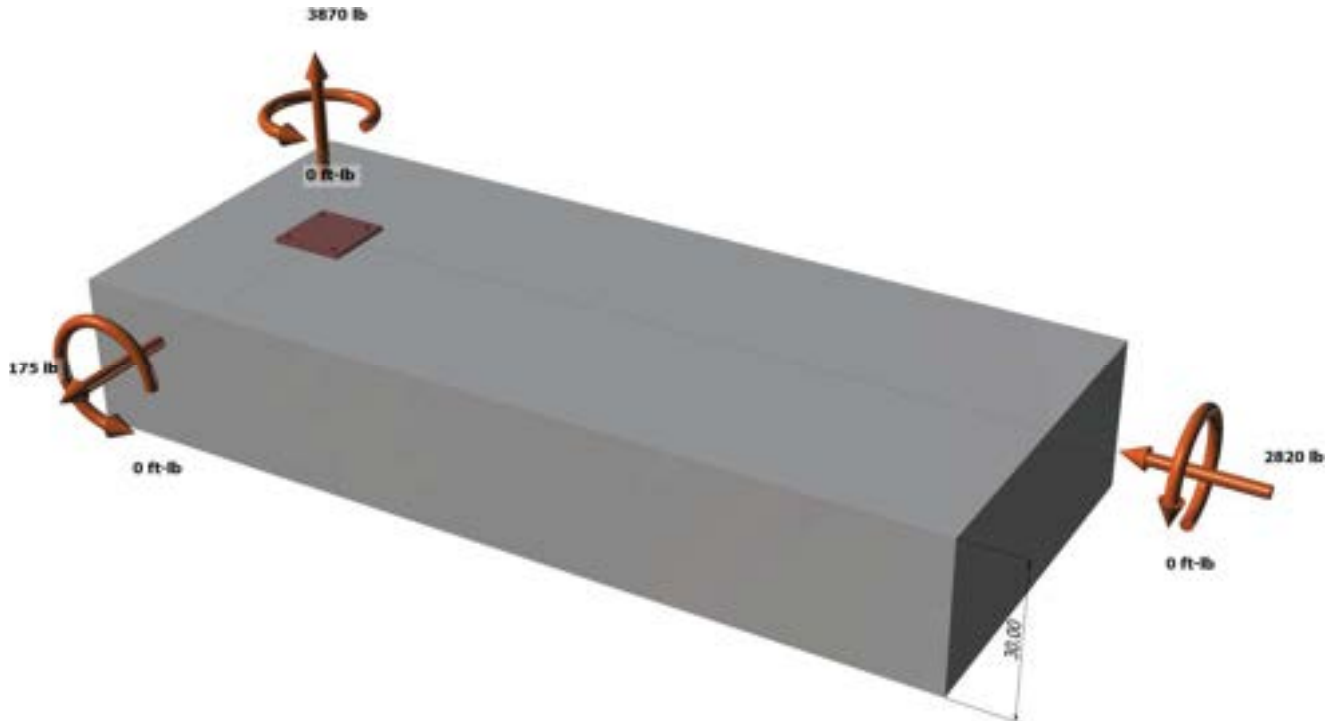
$V_{uay}$  [lb]: -2820

$M_{ux}$  [ft-lb]: 0

$M_{uy}$  [ft-lb]: 0

$M_{uz}$  [ft-lb]: 0

<Figure 1>





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<Figure 2>







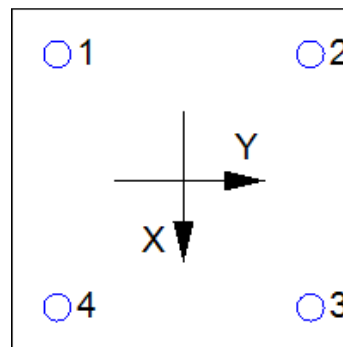
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### 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	967.5	43.8	-705.0	706.4
2	967.5	43.8	-705.0	706.4
3	967.5	43.8	-705.0	706.4
4	967.5	43.8	-705.0	706.4
Sum	3870.0	175.0	-2820.0	2825.4

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 3870  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis, e'<sub>Nx</sub> (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis, e'<sub>Ny</sub> (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis, e'<sub>Vx</sub> (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis, e'<sub>Vy</sub> (inch): 0.00

<Figure 3>



### 4. Steel Strength of Anchor in Tension (Sec. 17.4.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
4525	0.75	3394

### 5. Concrete Breakout Strength of Anchor in Tension (Sec. 17.4.2)

$$N_b = K_c \lambda_a \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. 17.4.2.2a)}$$

K <sub>c</sub>	λ <sub>a</sub>	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. 17.3.1 \& Eq. 17.4.2.1b)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	c <sub>a,min</sub> (in)	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
424.63	324.00	7.25	1.000	0.942	1.00	1.000	12492	0.65	10021

### 6. Adhesive Strength of Anchor in Tension (Sec. 17.4.5)

$$\tau_{k,cr} = \tau_{k,cr,short-term} K_{sat} (f_c / 2,500)^n$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	f <sub>c</sub> (psi)	n	τ <sub>k,cr</sub> (psi)
1346	1.00	1.00	2500	0.24	1346

$$N_{ba} = \lambda_a \tau_{cr} \pi d_a h_{ef} \text{ (Eq. 17.4.5.2)}$$

λ <sub>a</sub>	τ <sub>cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>ba</sub> (lb)
1.00	1346	0.38	6.000	9514

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ec,Na} \psi_{ed,Na} \psi_{cp,Na} N_{ba} \text{ (Sec. 17.3.1 \& Eq. 17.4.5.1b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	c <sub>Na</sub> (in)	c <sub>a,min</sub> (in)	ψ <sub>ec,Na</sub>	ψ <sub>ed,Na</sub>	ψ <sub>cp,Na</sub>	N <sub>ba</sub> (lb)	φ	φN <sub>ag</sub> (lb)
198.45	112.09	5.29	7.25	1.000	1.000	1.000	9514	0.55	9265



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**8. Steel Strength of Anchor in Shear (Sec. 17.5.1)**

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
2715	1.0	0.65	1765

**9. Concrete Breakout Strength of Anchor in Shear (Sec. 17.5.2)**

**Shear perpendicular to edge in x-direction:**

$V_{bx} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$  (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

$l_e$ (in)	$d_a$ (in)	$\lambda_a$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
3.00	0.375	1.00	2500	13.75	16564

$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx}$  (Sec. 17.3.1 & Eq. 17.5.2.1b)

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
647.11	850.78	1.000	0.805	1.000	1.000	16564	0.70	7103

**Shear perpendicular to edge in y-direction:**

$V_{by} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$  (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

$l_e$ (in)	$d_a$ (in)	$\lambda_a$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
3.00	0.375	1.00	2500	10.75	11450

$\phi V_{cbgy} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by}$  (Sec. 17.3.1 & Eq. 17.5.2.1b)

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgy}$ (lb)
387.00	520.03	1.000	0.891	1.000	1.000	11450	0.70	5313

**Shear parallel to edge in x-direction:**

$V_{by} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$  (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

$l_e$ (in)	$d_a$ (in)	$\lambda_a$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
3.00	0.375	1.00	2500	7.25	6342

$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by}$  (Sec. 17.3.1, 17.5.2.1(c) & Eq. 17.5.2.1b)

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
261.00	236.53	1.000	1.000	1.000	1.000	6342	0.70	9797

**Shear parallel to edge in y-direction:**

$V_{bx} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$  (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

$l_e$ (in)	$d_a$ (in)	$\lambda_a$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
3.00	0.375	1.00	2500	10.25	10661

$\phi V_{cbgy} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx}$  (Sec. 17.3.1, 17.5.2.1(c) & Eq. 17.5.2.1b)

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgy}$ (lb)
401.67	472.78	1.000	1.000	1.000	1.000	10661	0.70	12680

**10. Concrete Pryout Strength of Anchor in Shear (Sec. 17.5.3)**

$\phi V_{cp} = \phi \min|k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min|k_{cp} (A_{Na} / A_{Na0}) \psi_{ec,Na} \psi_{ed,Na} \psi_{cp,Na} N_{ba}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b|$  (Sec. 17.3.1 & Eq. 17.5.3.1b)

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{ec,Na}$	$\psi_{cp,Na}$	$N_{ba}$ (lb)	$N_a$ (lb)
2.0	198.45	112.09	1.000	1.000	1.000	9514	16845

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
424.63	324.00	1.000	0.942	1.000	1.000	12492	15417	0.70

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Company:		Date:	5/14/2018
Engineer:		Page:	6/6
Project:			
Address:			
Phone:			
E-mail:			

$\phi V_{cpq}$  (lb)  
21584

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. R17.6)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	968	3394	0.29	Pass	
Concrete breakout	3870	10021	0.39	Pass	
<b>Adhesive</b>	<b>3870</b>	<b>9265</b>	<b>0.42</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	706	1765	0.40	Pass	
T Concrete breakout x+	175	7103	0.02	Pass	
T Concrete breakout y-	2820	5313	0.53	Pass	
Concrete breakout y-	88	9797	0.01	Pass	
Concrete breakout x-	1410	12680	0.11	Pass	
<b>Concrete breakout, combined</b>	-	-	<b>0.53</b>	<b>Pass (Governs)</b>	
Pryout	2825	21584	0.13	Pass	
Interaction check	$(N_{ua}/\phi N_{ua})^{5/3}$	$(V_{ua}/\phi V_{ua})^{5/3}$	Combined Ratio	Permissible	Status
Sec. R17.6	0.23	0.35	58.2%	1.0	Pass

**SET-3G w/ 3/8"Ø F1554 Gr. 36 with hef = 6.000 inch meets the selected design criteria.**

### 12. Warnings

- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.

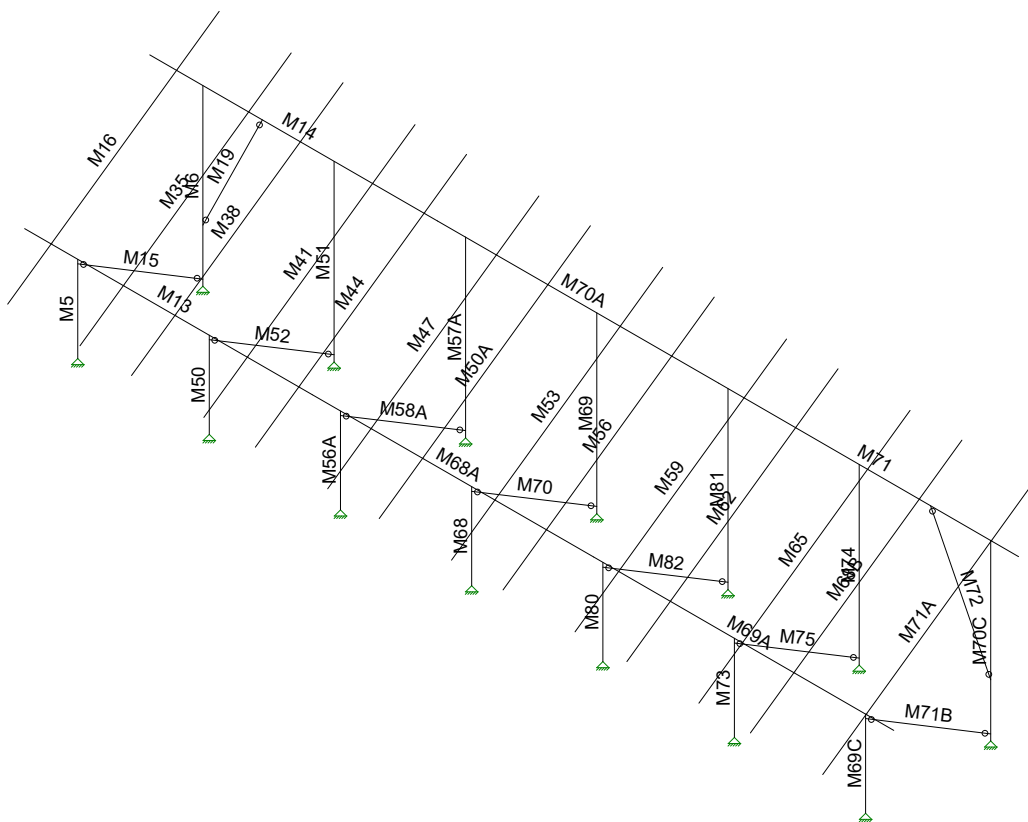


JOB NO.: U2716-0231-201

DESIGNED: STB

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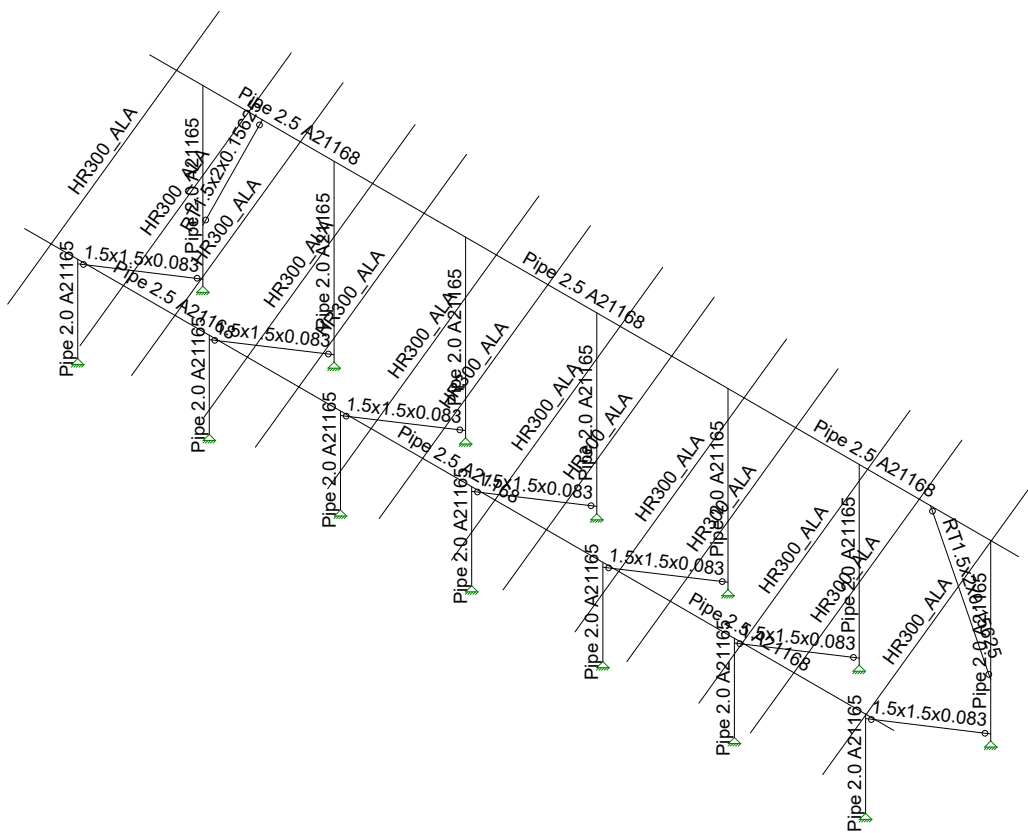
# Framing Analysis



Vector Structural Engineeri...  
STB  
U2716.0231.201

Ground Mount

SK - 3  
July 22, 2020 at 10:29 AM  
Sunmodo Sunturf A5a.r3d



Vector Structural Engineeri...

STB

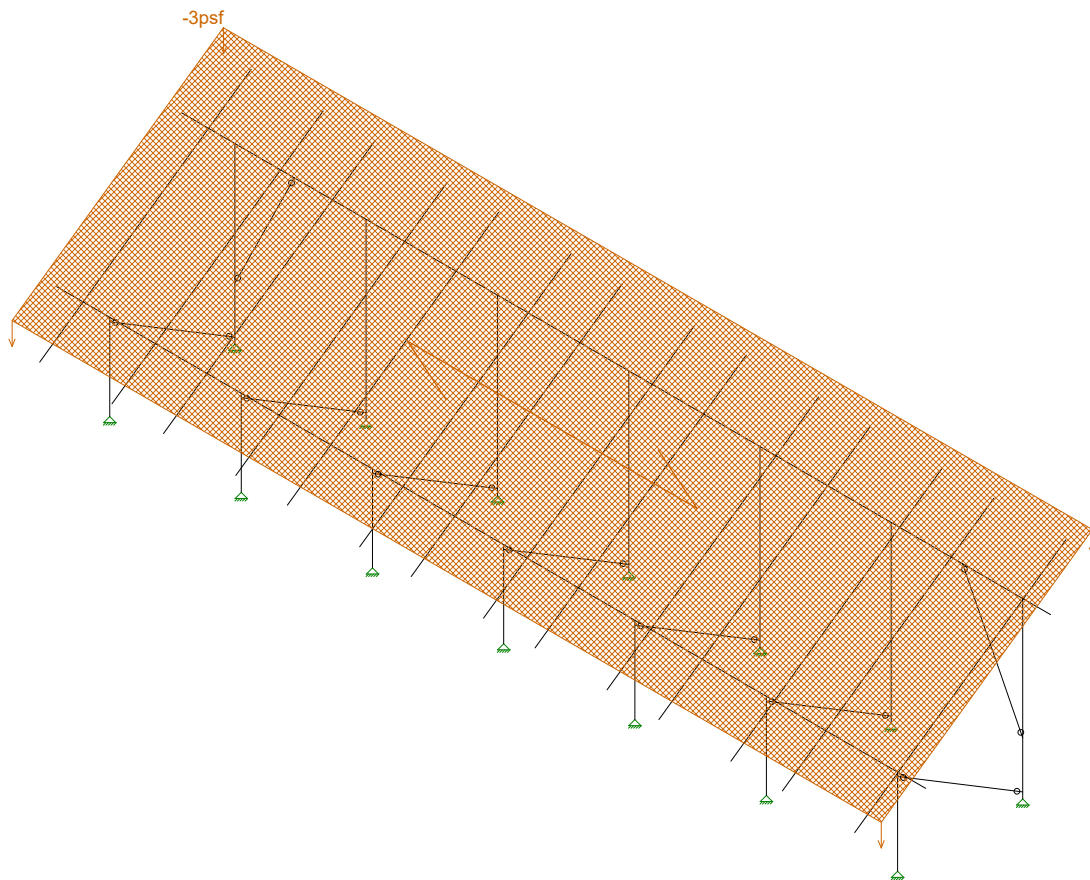
U2716.0231.201

Ground Mount

SK - 4

July 22, 2020 at 10:29 AM

Sunmodo Sunturf A5a.r3d



Loads: BLC 2, Solar Panel Weight

Vector Structural Engineeri..

STB

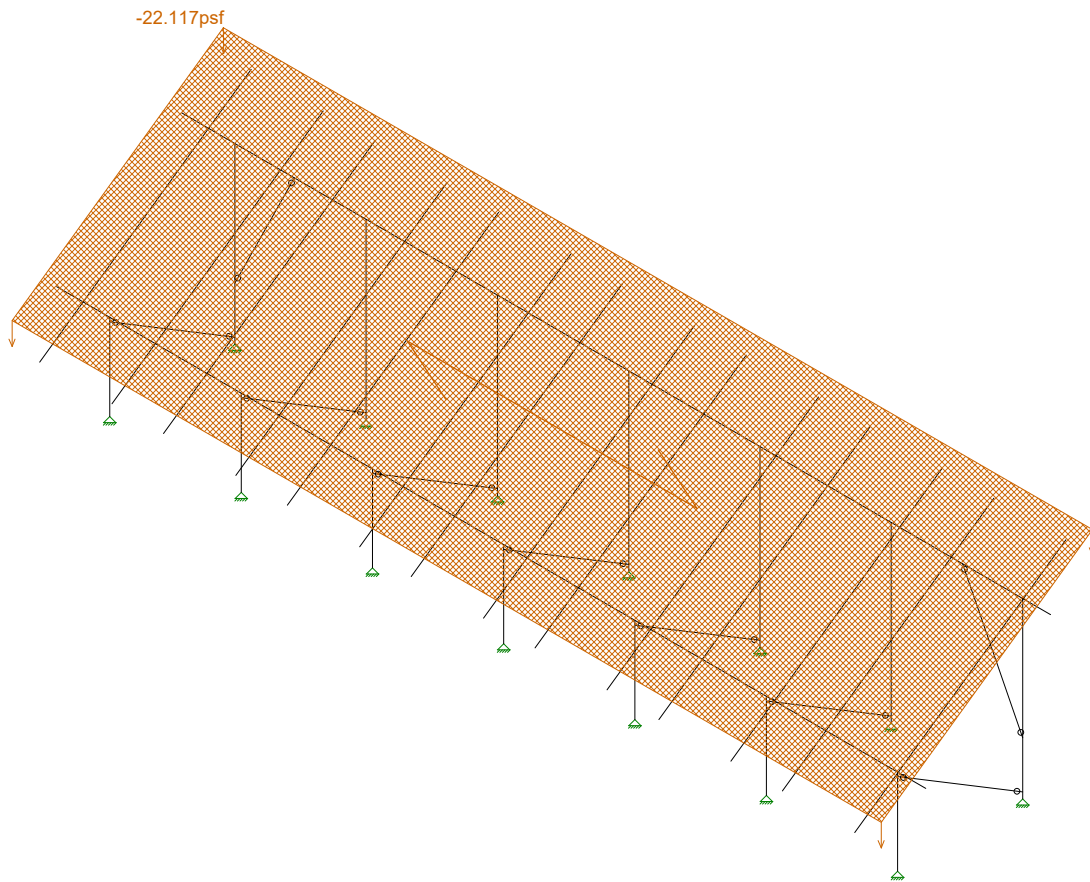
U2716.0231.201

Ground Mount

SK - 5

July 22, 2020 at 10:29 AM

Sunmodo Sunturf A5a.r3d



Loads: BLC 3, Roof Live/Snow

Vector Structural Engineeri..

STB

U2716.0231.201

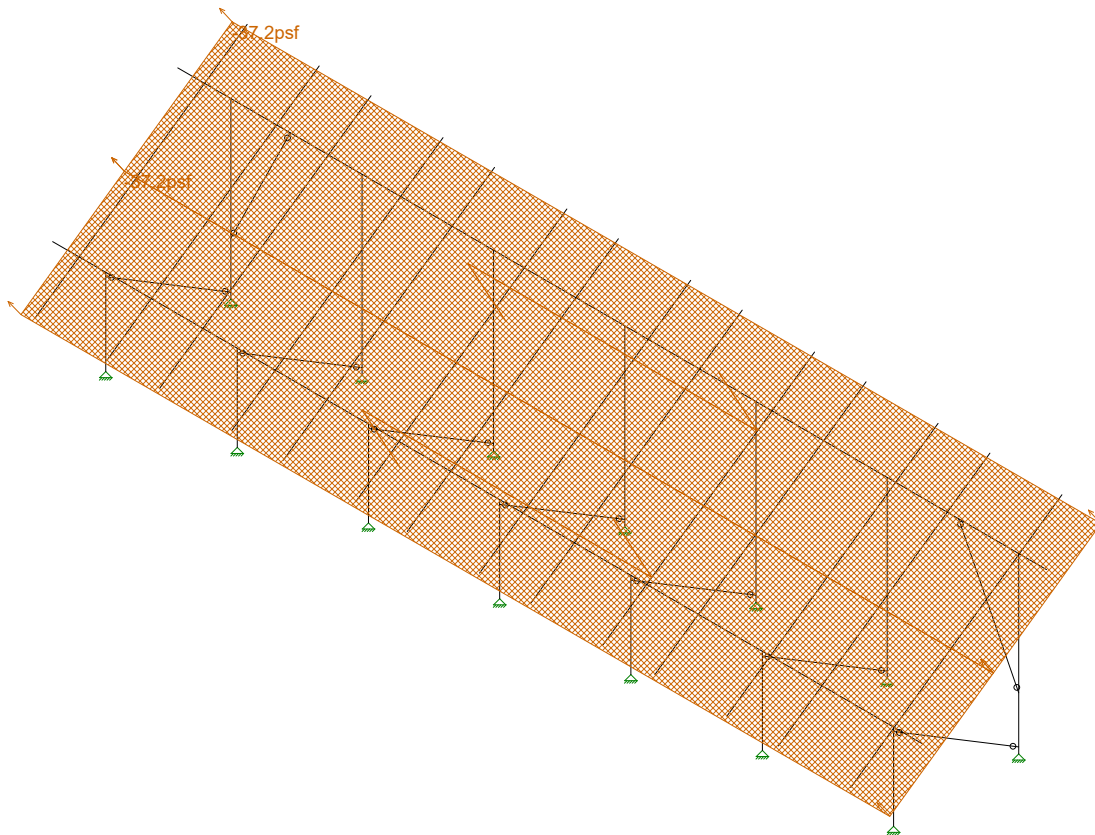
Ground Mount

SK - 6

July 22, 2020 at 10:29 AM

Sunmodo Sunturf A5a.r3d





Loads: BLC 4, Wind A 0 deg

Vector Structural Engineeri..

STB

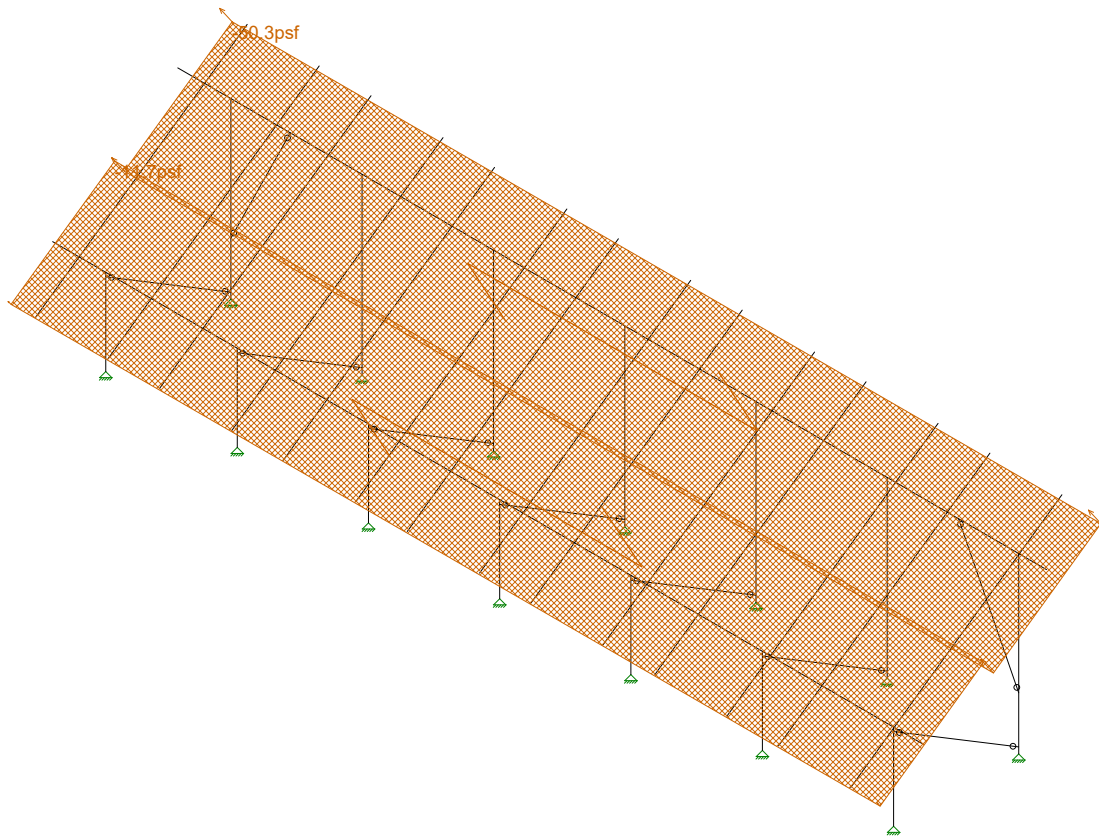
U2716.0231.201

Ground Mount

SK - 7

July 22, 2020 at 10:29 AM

Sunmodo Sunturf A5a.r3d



Loads: BLC 5, Wind B 0 deg

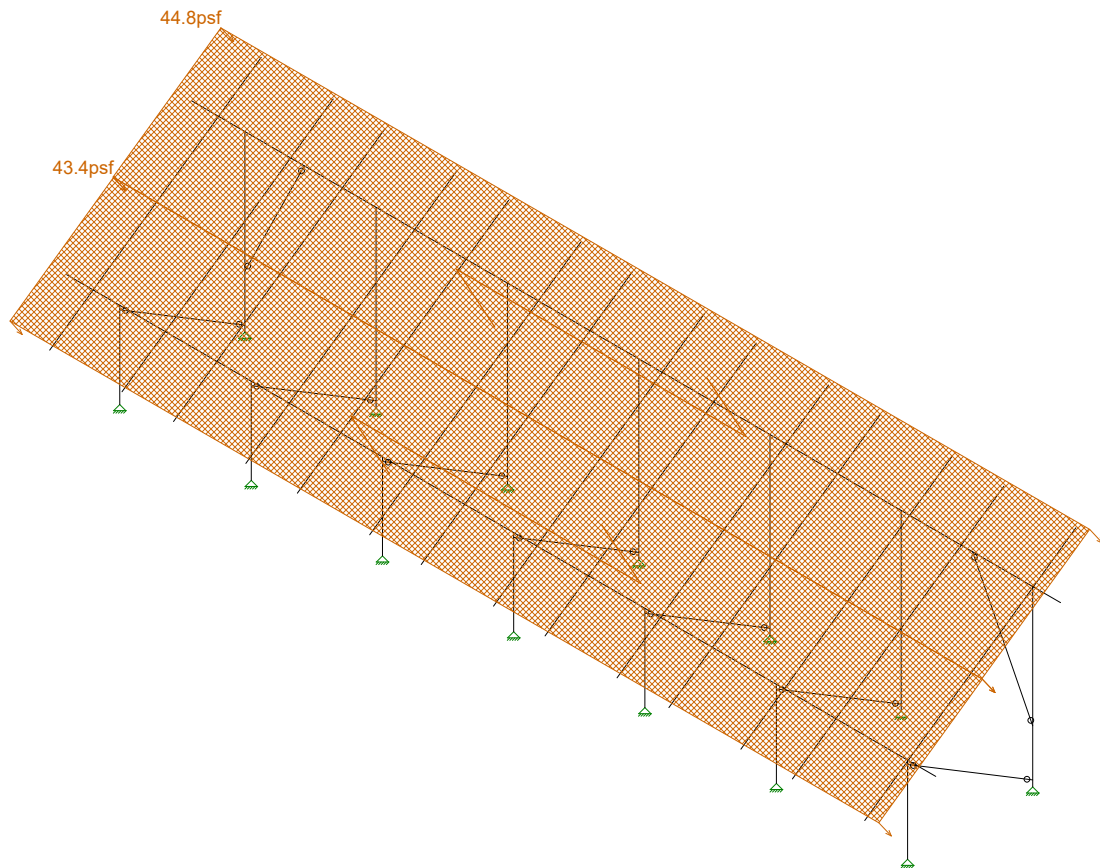
Vector Structural Engineeri...  
STB  
U2716.0231.201

Ground Mount

SK - 8

July 22, 2020 at 10:29 AM

Sunmodo Sunturf A5a.r3d



Loads: BLC 6, Wind A 180 deg

Vector Structural Engineeri...

STB

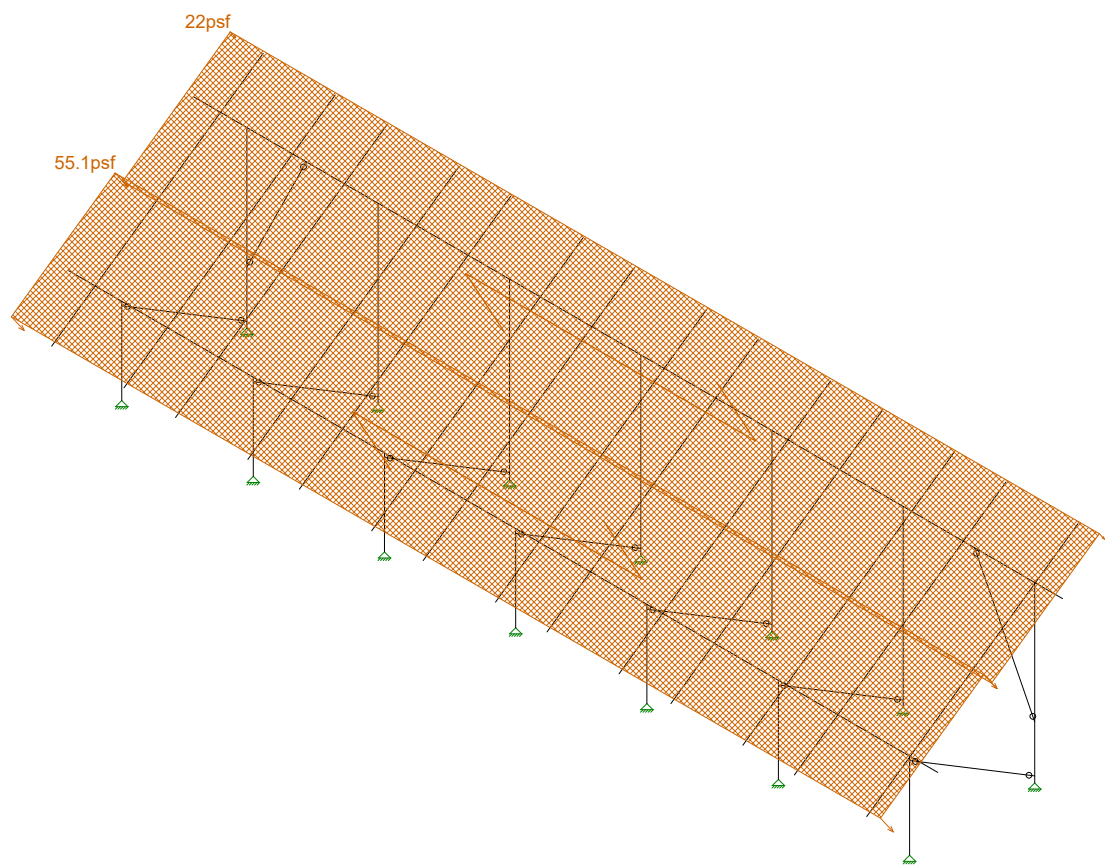
U2716.0231.201

Ground Mount

SK - 9

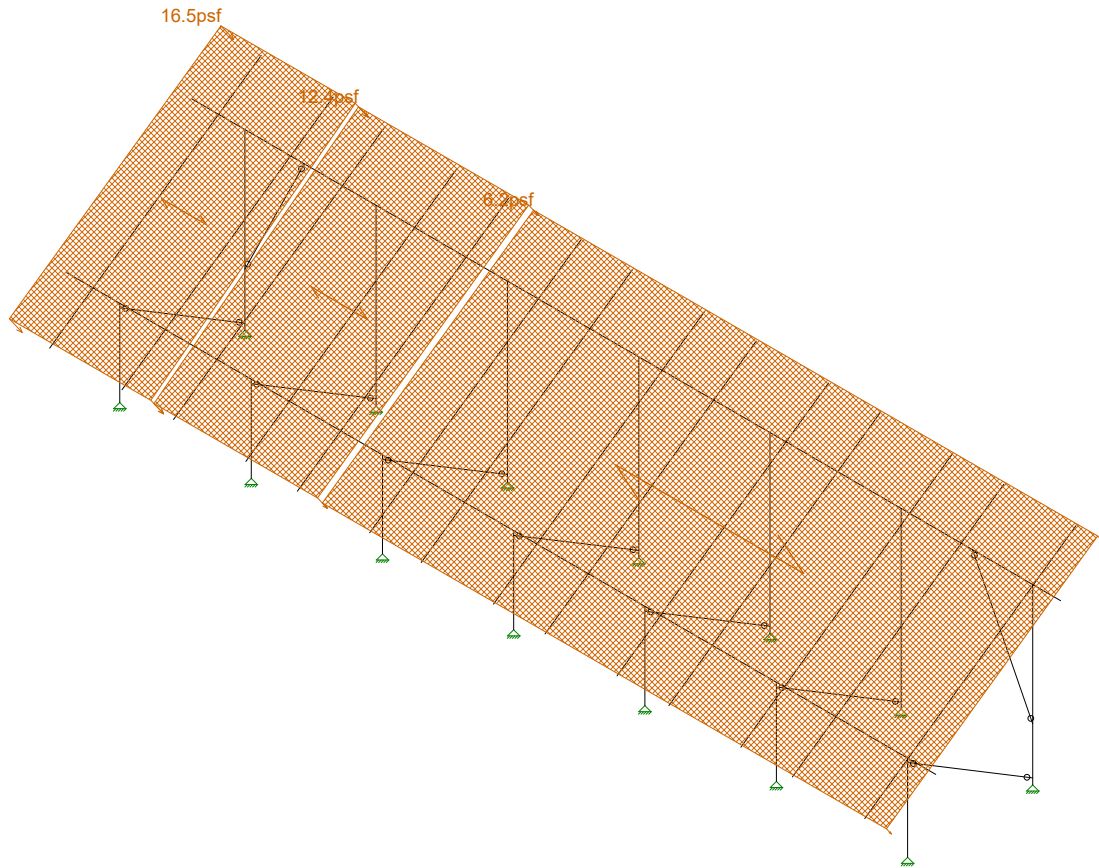
July 22, 2020 at 10:29 AM

Sunmodo Sunturf A5a.r3d



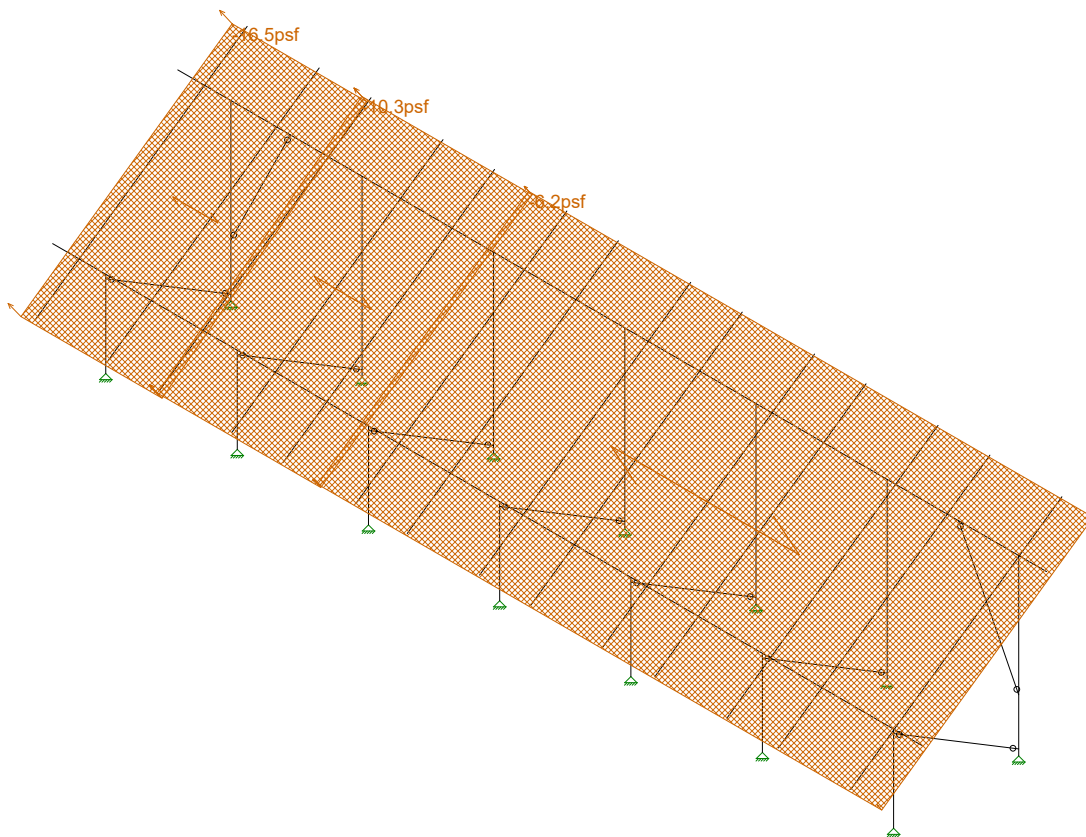
Loads: BLC 7, Wind B 180 deg

Vector Structural Engineeri..	Ground Mount	SK - 10
STB		July 22, 2020 at 10:30 AM
U2716.0231.201		Sunmodo Sunturf A5a.r3d



Loads: BLC 8, Wind A 90

Vector Structural Engineeri..	Ground Mount	SK - 11
STB		July 22, 2020 at 10:30 AM
U2716.0231.201		Sunmodo Sunturf A5a.r3d



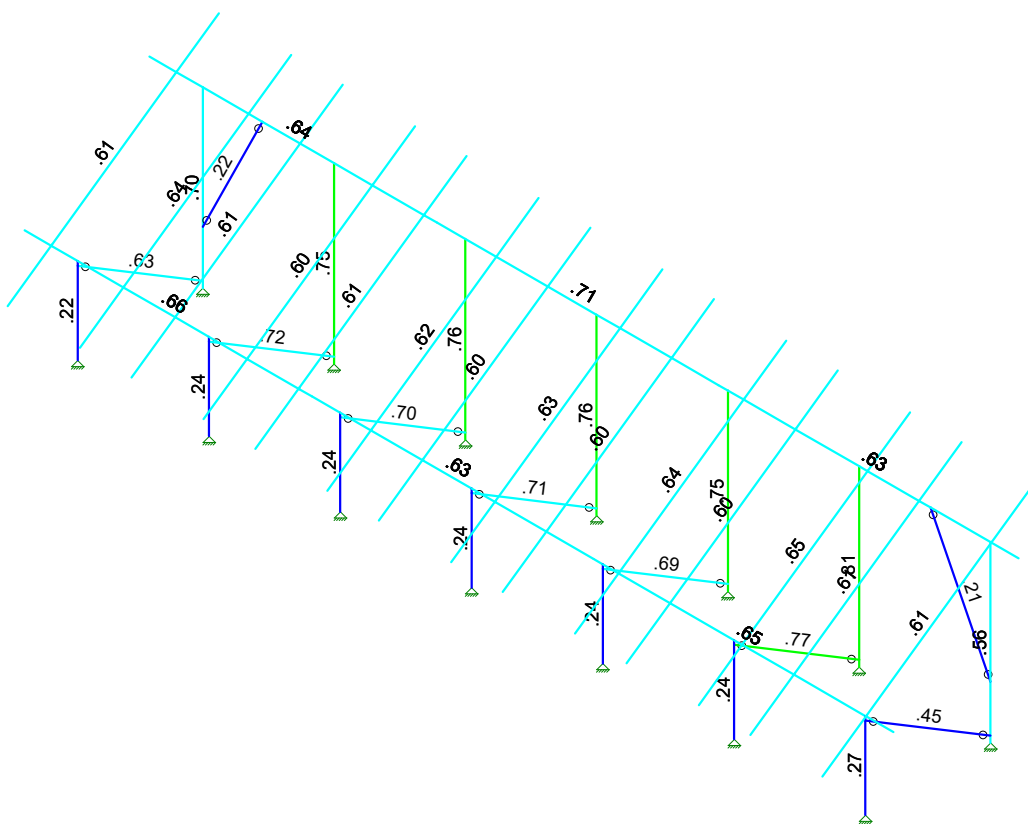
Loads: BLC 9, Wind B 90

Vector Structural Engineeri..	Ground Mount	SK - 12
STB		July 22, 2020 at 10:30 AM
U2716.0231.201		Sunmodo Sunturf A5a.r3d



Code Check  
(Elem.)

Black	No Calc
Red	> 1.0
Yellow	40-1.0
Green	75-90
Cyan	50-75
Blue	0-.50



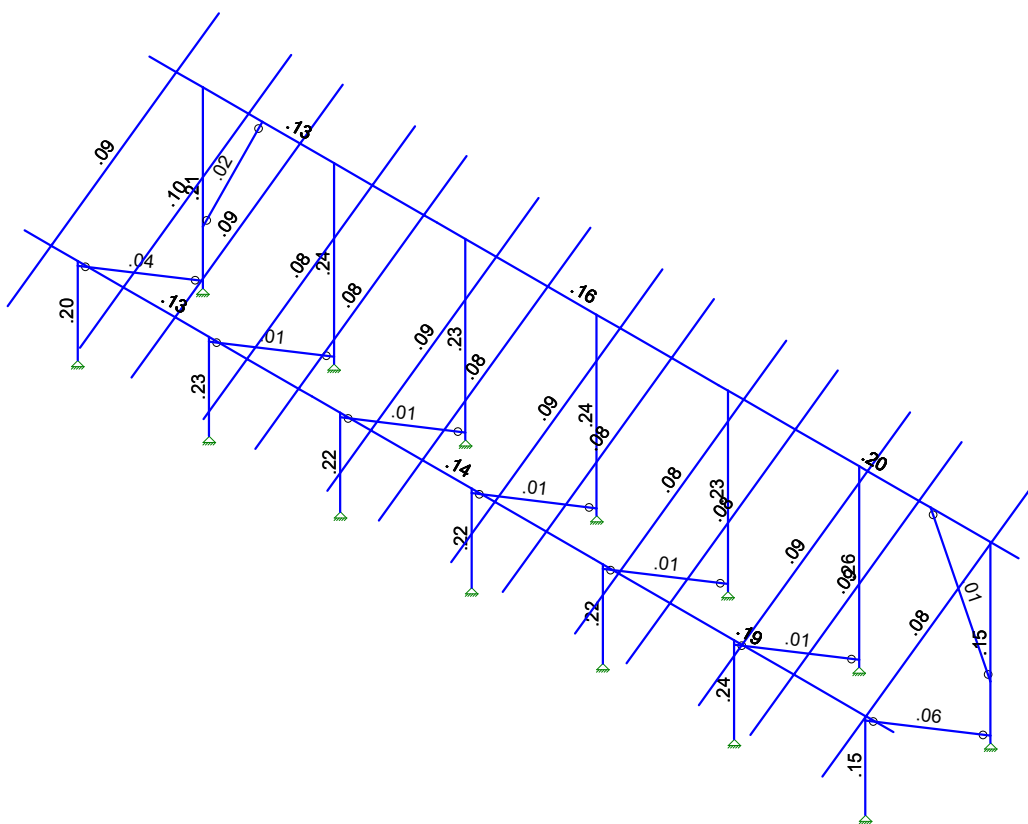
Member Code Checks Displayed (Enveloped)  
Envelope Only Solution

Vector Structural Engineeri...	Ground Mount	SK - 1
STB		July 22, 2020 at 10:28 AM
U2716.0231.201		Sunmodo Sunturf A5a.r3d



Shear Check  
(Elev.)

Black	No Calc
Red	> 1.0
Yellow	40-1.0
Green	75-90
Blue	50-75
Light Blue	0-.50



Member Shear Checks Displayed (Enveloped)  
Envelope Only Solution

Vector Structural Engineeri...	Ground Mount	SK - 2
STB		July 22, 2020 at 10:28 AM
U2716.0231.201		Sunmodo Sunturf A5a.r3d







**(Global) Model Settings, Continued**

Seismic Code	ASCE 7-16
Seismic Base Elevation (in)	15600
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
T X (sec)	Not Entered
T Z (sec)	Not Entered
R X	3
R Z	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	I or II
Drift Cat	Other
Om Z	1
Om X	1
Cd Z	4
Cd X	4
Rho Z	1
Rho X	1

**Hot Rolled Steel Properties**

	Label	E [ksi]	G [ksi]	Nu	Therm (/1E5 F)	Density[lb/ft^3]	Yield[psi]	Ry	Fu[psi]	Rt
1	A992	29000	11154	.3	.65	490	50000	1.1	65000	1.1
2	A36 Gr.36	29000	11154	.3	.65	490	36000	1.5	58000	1.2
3	A572 Gr.50	29000	11154	.3	.65	490	50000	1.1	65000	1.1
4	A500 Gr.B R...	29000	11154	.3	.65	527	42000	1.4	58000	1.3
5	A500 Gr.B Re...	29000	11154	.3	.65	527	46000	1.4	58000	1.3
6	A53 Gr.B	29000	11154	.3	.65	490	35000	1.6	60000	1.2
7	A1085	29000	11154	.3	.65	490	50000	1.4	65000	1.3

**Aluminum Properties**

	Label	E [ksi]	G [ksi]	Nu	Therm (...Density[... Table B.4	kt	Ftu[psi]	Fty[psi]	Fcy[psi]	Fsu[psi]	Ct
1	3003-H14	10100	3787.5	.33	1.3 172.8 Table B.4-1	1	19000	16000	13000	12000	141
2	6061-T6	10100	3787.5	.33	1.3 172.8 Table B.4-2	1	38000	35000	35000	24000	141
3	6063-T5	10100	3787.5	.33	1.3 172.8 Table B.4-2	1	22000	16000	16000	13000	141
4	6063-T6	10100	3787.5	.33	1.3 172.8 Table B.4-2	1	30000	25000	25000	19000	141
5	5052-H34	10200	3787.5	.33	1.3 172.8 Table B.4-1	1	34000	26000	24000	20000	141
6	6061-T6 W	10100	3787.5	.33	1.3 172.8 Table B.4-1	1	24000	15000	15000	15000	141
7	6005-T5	10100	3787.5	.33	1.3 172.8 Table B.4-1	1	38000	35000	35000	24000	141

**Hot Rolled Steel Section Sets**

	Label	Shape	Type	Design List	Material	Design R...	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	Post	Pipe 2.0 A21165	Column	Pipe	A572 Gr.50	Typical	.776	.499	.499	.998
2	Cross Beam	Pipe 2.5 A21168	Beam	Wide Flange	A572 Gr.50	Typical	.947	.907	.907	1.814
3	Diagonal Brace	1.5x1.5x0.083	HBrace	SquareTube	A572 Gr.50	Typical	.47	.158	.158	.236



**Aluminum Section Sets**

	Label	Shape	Type	Design List	Material	Design R...	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	AL Posts	2.375ODX0.188	Column	Pipe	6005-T5	Typical	1.29	.778	.778	1.54
2	AL Brace	RT1.5x2x0.15625	VBrace	Rectangular Tubes	6005-T5	Typical	.996	.327	.524	.602
3	AL Rails	HR300 ALA	Beam	Rectangular Tubes	6005-T5	Typical	.736	.214	.727	.614
4	AL Cross Beam	Cross Rail	Beam	Rectangular Tubes	6005-T5	Typical	1.909	1.97	4.366	4.017

**Member Area Loads (BLC 2 : Solar Panel Weight)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N199	N196	Y	A-B	-3

**Member Area Loads (BLC 3 : Roof Live/Snow)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N199	N196	PY	A-B	-27

**Member Area Loads (BLC 4 : Wind A 0 deg)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	-37.2
2	N198	N201	N199	N196	Perp	A-B	-37.2

**Member Area Loads (BLC 5 : Wind B 0 deg)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	-50.3
2	N198	N201	N199	N196	Perp	A-B	-11.7

**Member Area Loads (BLC 6 : Wind A 180 deg)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	44.8
2	N198	N201	N199	N196	Perp	A-B	43.4

**Member Area Loads (BLC 7 : Wind B 180 deg)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	22
2	N198	N201	N199	N196	Perp	A-B	55.1

**Member Area Loads (BLC 8 : Wind A 90)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N203	N202	N196	Perp	A-B	16.5
2	N203	N209	N208	N202	Perp	A-B	12.4
3	N209	N200	N199	N208	Perp	A-B	6.2

**Member Area Loads (BLC 9 : Wind B 90)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N203	N202	N196	Perp	A-B	-16.5
2	N203	N209	N208	N202	Perp	A-B	-10.3
3	N209	N200	N199	N208	Perp	A-B	-6.2

**Basic Load Cases**

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed	Area(M...Surface...
1	Self Weight	DL		-1.05					
2	Solar Panel Weight	DL							1





**Envelope Joint Reactions (Continued)**

Joint	X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [lb-ft]	LC	MY [lb-ft]	LC	MZ [lb-ft]	LC
14	min	-1.945	19	-2274.39	16	-1287....	3	0	1	0	1	0
15	N122	max	6.223	12	1691.661	10	63.102	3	0	20	0	20
16	min	-2.133	15	-360.184	17	-73.405	5	0	1	0	1	0
17	N133B	max	2.832	11	2928.258	11	1541.6...	5	0	20	0	20
18	min	-1.436	16	-2254.267	16	-1301....	3	0	1	0	1	0
19	N134B	max	10.194	12	1703.534	10	63.325	3	0	20	0	20
20	min	-1.721	15	-368.173	17	-73.541	5	0	1	0	1	0
21	N151	max	4.182	11	3013.052	11	1689	5	0	20	0	20
22	min	-1.189	16	-2319.256	16	-1426....	3	0	1	0	1	0
23	N152	max	30.296	12	1845.549	10	62.377	3	0	20	0	20
24	min	-8.746	15	-437.71	17	-71.617	5	0	1	0	1	0
25	N143A	max	98.796	16	2049.977	11	968.029	5	0	20	0	20
26	min	-130.72	11	-1545.649	16	-816.1...	3	0	1	0	1	0
27	N144A	max	23.183	15	1076.121	10	47.422	3	0	20	0	20
28	min	-64.677	12	-210.865	17	-55.787	5	0	1	0	1	0
29	Totals:	max	.021	19	23713.032	11	9673.8...	5				
30	min	-.042	14	-9980.659	15	-8157....	15					

**Envelope AISC 15th(360-16): ASD Steel Code Checks**

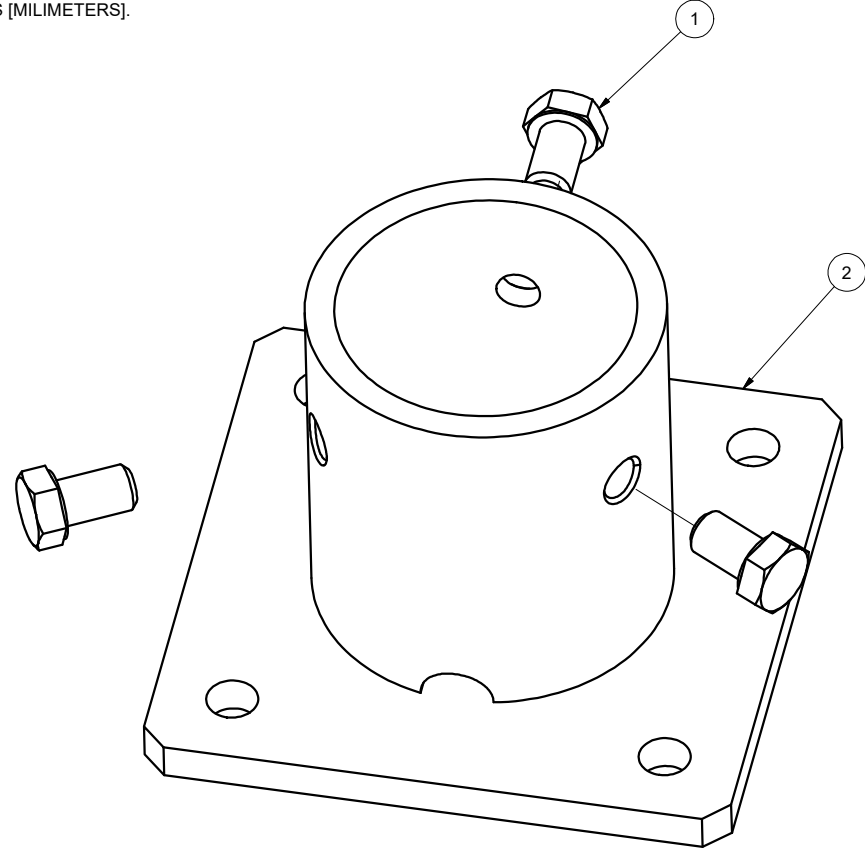
Member	Shape	Code Check	Loc[in]	LC	Shear	...	Loc[in]	Dir	LC	Pnc/om	[..Pnt/om	[lb]	Mnyy/om..	Mnzz/om..	Cb	Eqn
1	M5	Pipe 2.0 A2...	.222	52.073	17	.201	52.645	5	16486.797	23232.186	1397.505	1397.505	1...	H1-1b		
2	M6	Pipe 2.0 A2...	.703	3.477	5	.210	0	5	6062.107	23232.186	1397.505	1397.505	1...	H1-1a		
3	M13	Pipe 2.5 A2...	.656	118....	12	.128	116....	12	11641.036	28358.413	2081.747	2081.747	2...	H1-1b		
4	M14	Pipe 2.5 A2...	.639	118....	11	.127	116....	11	11641.036	28358.413	2081.747	2081.747	2...	H1-1b		
5	M15	1.5x1.5x0.083	.626	48.694	5	.039	93.493	y	11	2716.455	14085.15	624.421	624.421	1...	H1-1a	
6	M80	Pipe 2.0 A2...	.237	52.073	17	.217	52.645	5	16486.797	23232.186	1397.505	1397.505	1...	H1-1b		
7	M81	Pipe 2.0 A2...	.752	3.477	5	.230	0	5	6062.107	23232.186	1397.505	1397.505	1...	H1-1a		
8	M82	1.5x1.5x0.083	.691	48.694	5	.010	93.493	y	10	2716.455	14085.15	624.421	624.421	1...	H1-1a	
9	M50	Pipe 2.0 A2...	.239	52.073	17	.227	52.645	5	16486.797	23232.186	1397.505	1397.505	1...	H1-1b		
10	M51	Pipe 2.0 A2...	.754	3.477	5	.241	0	5	6062.107	23232.186	1397.505	1397.505	1...	H1-1a		
11	M52	1.5x1.5x0.083	.722	48.694	5	.007	93.493	y	14	2716.455	14085.15	624.421	624.421	1...	H1-1a	
12	M56A	Pipe 2.0 A2...	.239	52.073	17	.220	52.645	5	16486.797	23232.186	1397.505	1397.505	1...	H1-1b		
13	M57A	Pipe 2.0 A2...	.762	3.477	5	.234	0	5	6062.107	23232.186	1397.505	1397.505	2...	H1-1a		
14	M58A	1.5x1.5x0.083	.699	48.694	5	.007	93.493	y	4	2716.455	14085.15	624.421	624.421	1...	H1-1a	
15	M68	Pipe 2.0 A2...	.239	52.073	17	.222	52.645	5	16486.797	23232.186	1397.505	1397.505	1...	H1-1b		
16	M69	Pipe 2.0 A2...	.763	3.477	5	.236	0	5	6062.107	23232.186	1397.505	1397.505	1...	H1-1a		
17	M70	1.5x1.5x0.083	.707	48.694	5	.009	0	y	10	2716.455	14085.15	624.421	624.421	1...	H1-1a	
18	M68A	Pipe 2.5 A2...	.635	18.75	12	.138	103....	12	11641.036	28358.413	2081.747	2081.747	2...	H1-1b		
19	M69A	Pipe 2.5 A2...	.652	90	12	.193	90	12	11641.036	28358.413	2081.747	2081.747	1...	H1-1b		
20	M70A	Pipe 2.5 A2...	.707	18.75	11	.161	103....	11	11641.036	28358.413	2081.747	2081.747	2...	H1-1b		
21	M71	Pipe 2.5 A2...	.632	6	11	.196	90	11	11641.036	28358.413	2081.747	2081.747	2...	H1-1b		
22	M73	Pipe 2.0 A2...	.240	52.073	17	.245	52.645	5	16486.797	23232.186	1397.505	1397.505	1...	H1-1b		
23	M74	Pipe 2.0 A2...	.811	3.477	5	.258	0	5	6062.107	23232.186	1397.505	1397.505	1...	H1-1a		
24	M75	1.5x1.5x0.083	.774	48.694	5	.011	0	y	10	2716.455	14085.15	624.421	624.421	1...	H1-1a	
25	M69C	Pipe 2.0 A2...	.274	52.645	12	.146	52.645	5	16486.797	23232.186	1397.505	1397.505	1...	H1-1b		
26	M70C	Pipe 2.0 A2...	.561	33.608	11	.147	0	5	6062.107	23232.186	1397.505	1397.505	1...	H1-1a		
27	M71B	1.5x1.5x0.083	.448	48.694	5	.063	0	y	11	2716.455	14085.15	624.421	624.421	1...	H1-1a	

**Envelope AA ADM1-15: ASD - Building Aluminum Code Checks**

Member	Shape	Code C...	Loc[in]	LC	Shear	...	Loc[in]	Dir	LC	Pnc/O...	Pnt/Om...	Mny/O...	Mnz/O...	Vny/O...	Vnz/O...	Cb	Eqn
1	M19	RT1.5x2x...	.222	52.977	11	.023	0	z	5	2260.001	19411....	770.742	927.083	5889.423	3966.346	1...	H.1-1
2	M16	HR300_A...	.615	82.515	11	.085	36.1	y	11	3887.213	14342....	494.953	934.619	6030.769	2749.538	1	H.1-1
3	M35	HR300_A...	.635	80.796	11	.095	36.1	y	12	3887.213	14342....	494.953	934.619	6030.769	2749.538	1	H.1-1
4	M38	HR300_A...	.615	84.234	11	.089	36.1	y	12	3887.213	14342....	494.953	934.619	6030.769	2749.538	1	H.1-1



NOTES: UNLESS OTHERWISE SPECIFIED  
 1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].



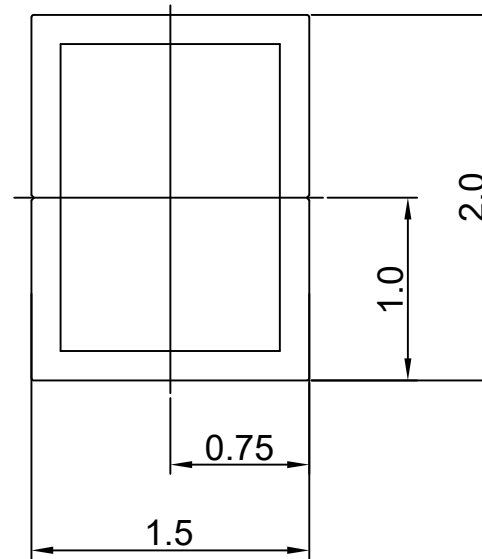
REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	10/20/2016

2	A21120-001	2" PIPE BASE	1
1	B15018-011	HEX CAP SCREW 3/8-16 X 5/8	3
ITEM	PART NUMBER	DESCRIPTION	QTY
MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		<b>Sunmodo Corp.</b> 1905 E 5TH STREET, STE A, VANCOUVER, WA 98661	
Tolerances		TITLE	
XXXX ±0.01 [0.25mm] XXX ±0.02 [0.50mm] XX ±0.039 [1.0mm] Unless otherwise spec'd		2" PIPE BASE KIT	
DRAWN BY		DATE	
LWF		10/20/2016	
CHECKED BY		DRAWING NUMBER	
		B K10268-001	
APPROVALS		SCALE: NONE SHEET 1 of 1	

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NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERAIL: ALUMINUM 6005-T5.  
FINISH: CLEAR ANODIZED 10  $\mu$ m THICK.
3. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.



## Section properties:

Weight: 1.156 lbs/ft

Area: 0.992 in<sup>2</sup>

Perimeter: 12.601 in

Bounding Box: X: -1.000,1.000

Y: -0.750, 0.750

Centroid:(0.000,0.000)

Moments of Inertia(in<sup>4</sup>): I<sub>x</sub>=0.506,I<sub>y</sub>=0.322

Section modulus in bending(in<sup>3</sup>): W<sub>x</sub>=0.675,W<sub>y</sub>=0.322

Radii of Gyration: X: 0.714, Y: 0.570

MATERIAL		SEE NOTES	
Third Angle Projection:		<h1>Sunmodo Corp.</h1> <p>1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661</p>	
<b>GENERAL SPECIFICATIONS</b> All Dimensions in inches [millimeters] <b>Tolerances</b> X.XXX ± 0.01 [0.25mm] X.XX ± 0.02 [0.50mm] X.X ± 0.039 [1.0mm] Unless otherwise specd			
DRAWN BY		DATE	
zcg		03/12/2014	
CHECKED BY		B	
APPROVALS		DRAWING NUMBER	
		A20164	
SCALE:		NONE	
SHEET		1 of 1	

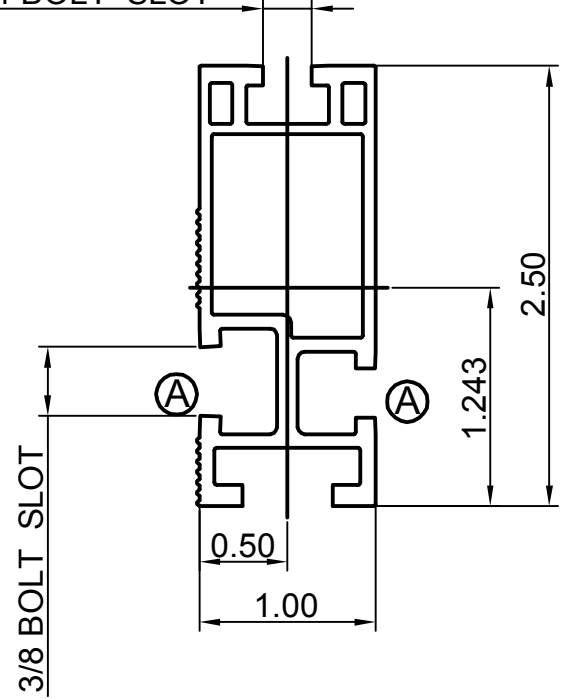


REVISIONS			
REV	DESCRIPTION	BY	DATE
A	ADDED BOTTOM CHANNEL & CHANGED ONE 3/8 CHANNEL TO 1/4	zcg	02/21/2013

NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERAIL: ALUMINUM 6005-T5.  
FINISH: CLEAR ANODIZED 15  $\mu$ m THICK.
3. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.

2X 1/4 BOLT SLOT



### Section properties:

Weight: 0.850 lbs/ft  
 Area: 0.723 in<sup>2</sup>  
 Perimeter: 17.325 in  
 Bounding Box: X: -0.500,0.500  
 Y: -1.243,1.257

Centroid:(0.000,0.000)  
 Moments of Incertia(in<sup>4</sup>): Ix=0.486,Iy=0.095  
 Section modulus in bending(in<sup>3</sup>): Wx=0.387,Wy=0.190  
 Radii of Gyration: X: 0.820, Y: 0.363

MATERIAL		SEE NOTES
Third Angle Projection:		
GENERAL SPECIFICATIONS		
All Dimensions in inches [millimeters]		
Tolerances		
X.XXX ± 0.01 [0.25mm]	Break all sharp edges	
X.XX ± 0.02 [0.50mm]	.010-.020 unless	
X.X ± 0.039 [1.0mm]	otherwise specified.	
DRAWN BY		DATE
zcg		02/21/2013
CHECKED BY		
APPROVALS		

<b>Sunmodo Corp.</b>	
1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661	
TITLE HELIO STANDARD RAIL	
DRAWING NUMBER	A20144
SCALE: NONE	SHEET 1 of 1

4

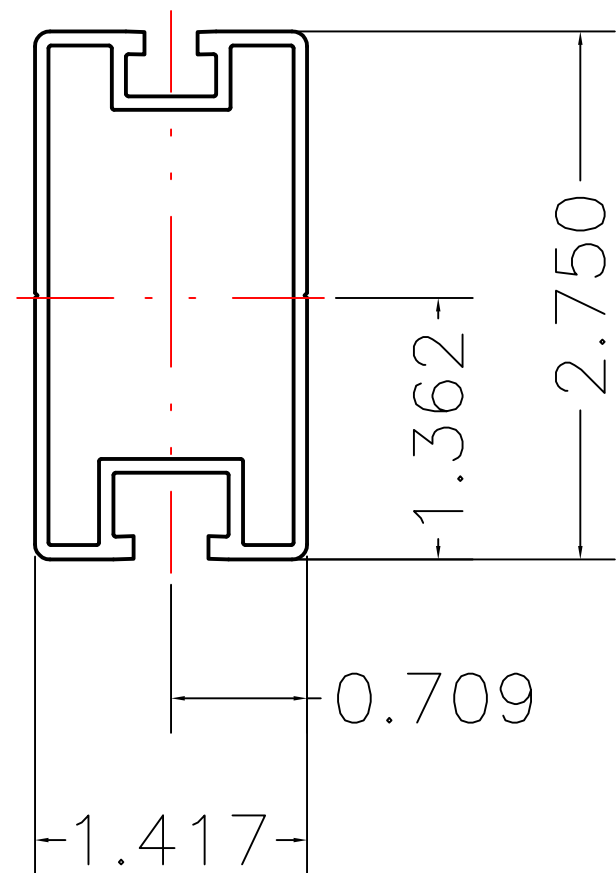
3

2

1

NOTES: UNLESS OTHERWISE SPECIFIED

- 1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
- 2. MATERIAL: 6005-T5.  
FINISH: CLEAR ANODIZED 10um THICK.
- 3. THE UNSPECIFIED RADII ARE .02" MAX.
- 4. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.



### Section properties:

Weight: 0.862 lbs/ft  
 Area: 0.736 in<sup>2</sup>  
 Perimeter: 19.824 in  
 Bounding Box: X: -0.709,0.709  
                   Y: -1.362,1.388  
 Centroid:(0.000,0.000)  
 Moments of Inertia(in<sup>4</sup>): I<sub>x</sub>=0.727,I<sub>y</sub>=0.214  
 Section modulus in bending(in<sup>3</sup>): W<sub>x</sub>=0.524,W<sub>y</sub>=0.302  
 Radii of Gyration: X: 994, Y: 0.539

MATERIAL SEE NOTES	
Third Angle Projection:	
GENERAL SPECIFICATIONS All Dimensions in inches [millimeters]	
Tolerances	
X.XXX ±0.01 [0.25mm]	Break all sharp edges
X.XX ±0.02 [0.50mm]	.010-.020 unless
X.X ±0.039 [1.0mm]	otherwise specified.
Unless otherwise spec'd	
DRAWN BY KYY	DATE 01/18/2018
CHECKED BY	
APPROVALS	

<b>SunModo Corp.</b>	
14800 NE 65TH STREET, VANCOUVER WA 98682	
TITLE RAIL, HR300 (SUNRAY), EXTRUSION	
B	DRAWING NUMBER A20288
SCALE: NONE	SHEET 1 of 1

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4

3

2

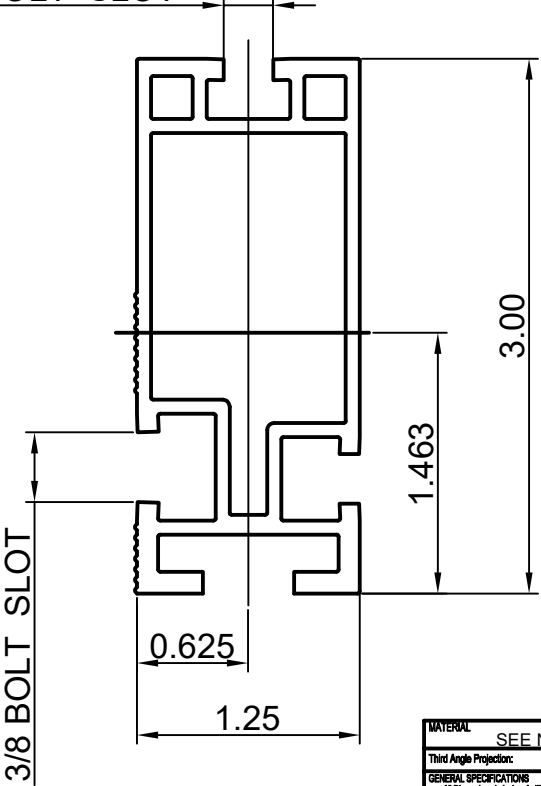
1

REV	DESCRIPTON	BY	DATE
A	0.44 WAS 0.41, 0.44 WAS 0.33	LWF	11/30/2015

NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERAIL: ALUMINUM 6005-T5.  
FINISH: CLEAR ANODIZED 15  $\mu$ m THICK.
3. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.

2X 1/4 BOLT SLOT



**Section properties:**

Weight: 1.151 lbs/ft  
 Area: 0.980 in<sup>2</sup>  
 Perimeter: 22.104 in  
 Bounding Box: X: -0.625,0.625  
                   Y: -1.463,1.537  
 Centroid:(0.000,0.000)  
 Moments of Inertia(in<sup>4</sup>): Ix=1.047,Iy=0.207  
 Section modulus in bending(in<sup>3</sup>): Wx=0.681,Wy=0.331  
 Radii of Gyration: X: 1.034, Y: 0.460

MATERIAL		SEE NOTES	
Third Angle Projection			
GENERAL SPECIFICATIONS			
All Dimensions in Inches (millimeters)			
Tolerances			
XXX ±0.01 (0.25mm)			
XX ±0.02 (0.50mm)			
X ±0.03 (1.0mm)			
Unless otherwise noted			
Break all sharp edges 0.10-0.25 unless otherwise specified.			
DRAWN BY		DATE	
ZCJ		02/21/2013	
CHECKED BY		B	
APPROVALS		DRAWING NUMBER	
		A20145	
SCALE:		SHEET	
NONE		1 of 1	

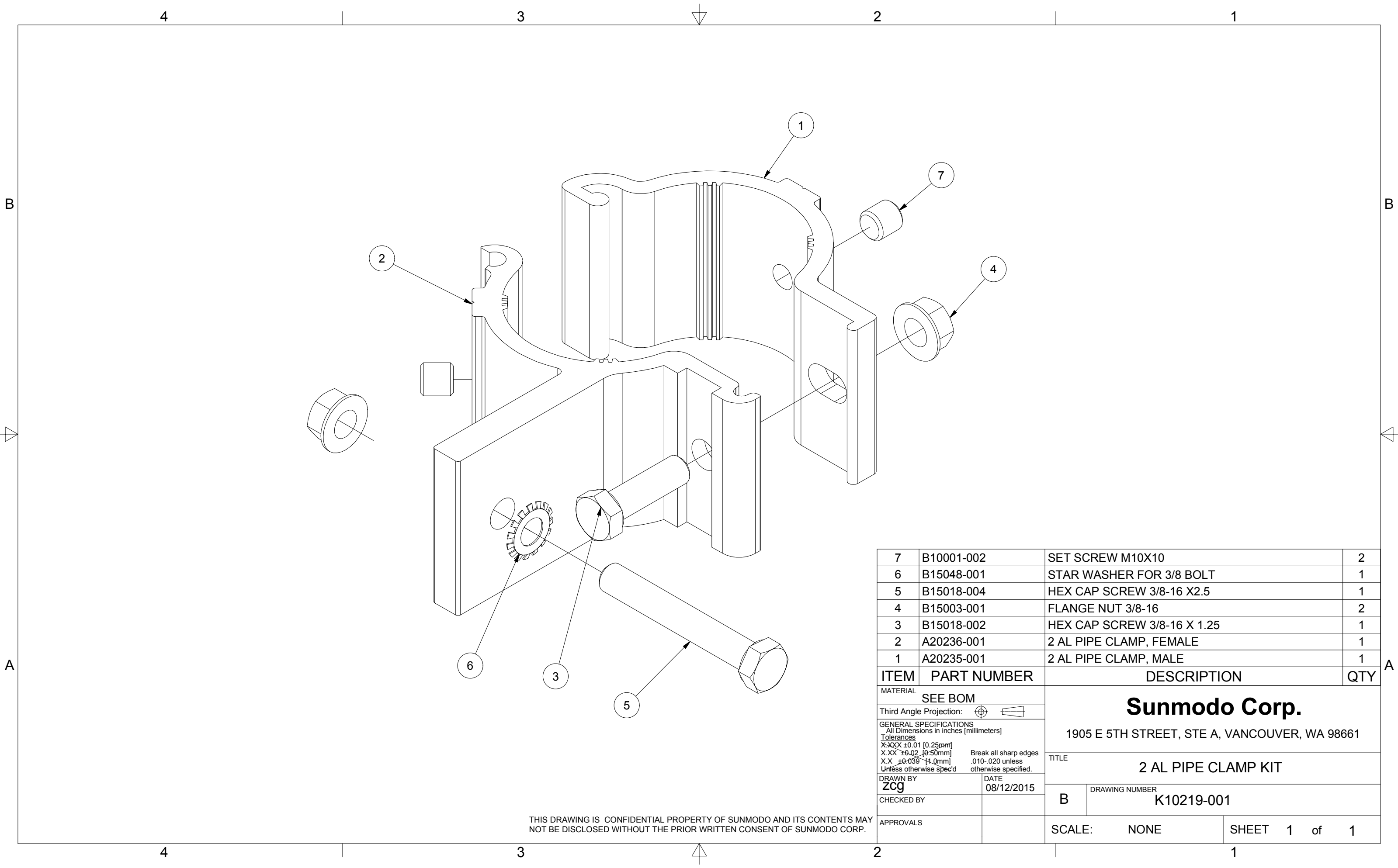
**Sunmodo Corp.**

1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661

TITLE  
HELIO HEAVY RAIL

B DRAWING NUMBER  
A20145

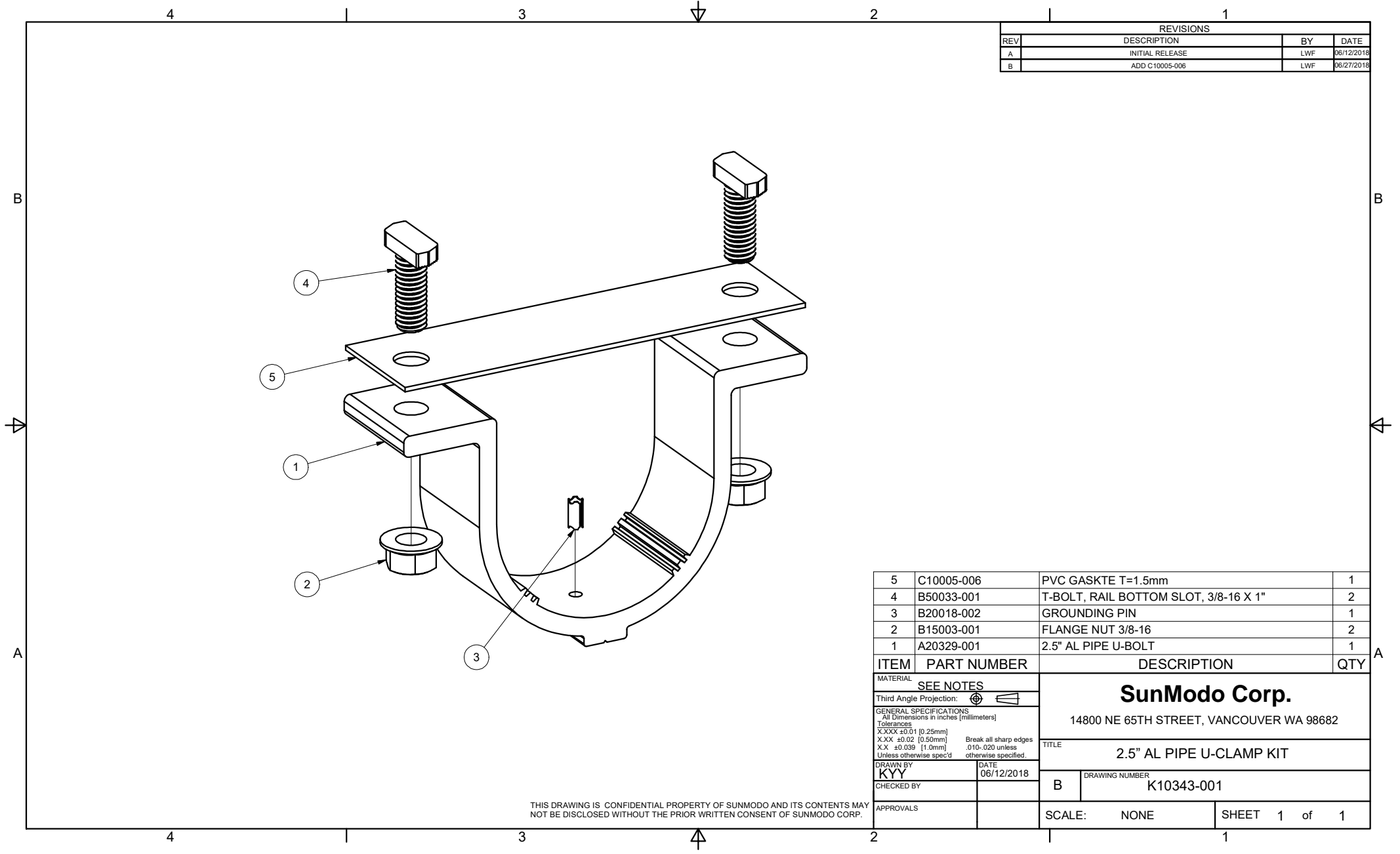
SCALE: NONE SHEET 1 of 1



7	B10001-002	SET SCREW M10X10	2
6	B15048-001	STAR WASHER FOR 3/8 BOLT	1
5	B15018-004	HEX CAP SCREW 3/8-16 X2.5	1
4	B15003-001	FLANGE NUT 3/8-16	2
3	B15018-002	HEX CAP SCREW 3/8-16 X 1.25	1
2	A20236-001	2 AL PIPE CLAMP, FEMALE	1
1	A20235-001	2 AL PIPE CLAMP, MALE	1

ITEM	PART NUMBER	DESCRIPTION	QTY
MATERIAL		SEE BOM	
Third Angle Projection:			
GENERAL SPECIFICATIONS All Dimensions in inches (millimeters)			
Tolerances			
X.XXX ±0.01 [0.25mm]		Break all sharp edges	
X.XX ±0.02 [0.50mm]		.010-.020 unless	
X.X ±0.039 [1.0mm]		otherwise specified.	
DRAWN BY		DATE	
zcg		08/12/2015	
CHECKED BY		B	
APPROVALS		DRAWING NUMBER	
		K10219-001	
SCALE:		NONE	SHEET 1 of 1

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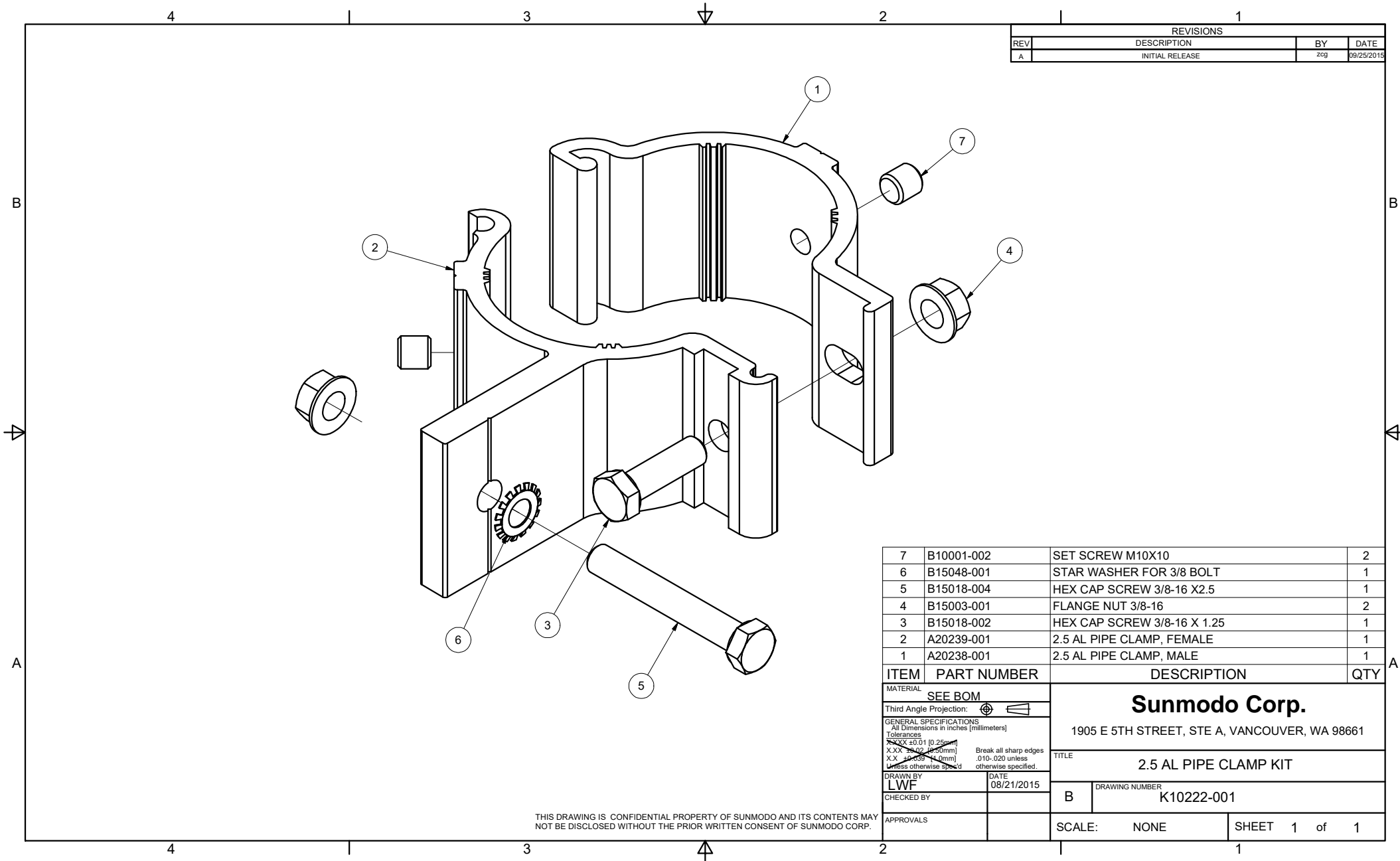


REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	06/12/2018
B	ADD C10005-006	LWF	08/27/2018

ITEM	PART NUMBER	DESCRIPTION	QTY
5	C10005-006	PVC GASKTE T=1.5mm	1
4	B50033-001	T-BOLT, RAIL BOTTOM SLOT, 3/8-16 X 1"	2
3	B20018-002	GROUNDING PIN	1
2	B15003-001	FLANGE NUT 3/8-16	2
1	A20329-001	2.5" AL PIPE U-BOLT	1

MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		<b>SunModo Corp.</b> 14800 NE 65TH STREET, VANCOUVER WA 98682	
Tolerances		TITLE	
X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] X.X ±0.039 [1.0mm] Unless otherwise spec'd		2.5" AL PIPE U-CLAMP KIT	
DRAWN BY	DATE	DRAWING NUMBER	
KYY	06/12/2018	B K10343-001	
CHECKED BY		SCALE: NONE SHEET 1 of 1	
APPROVALS			

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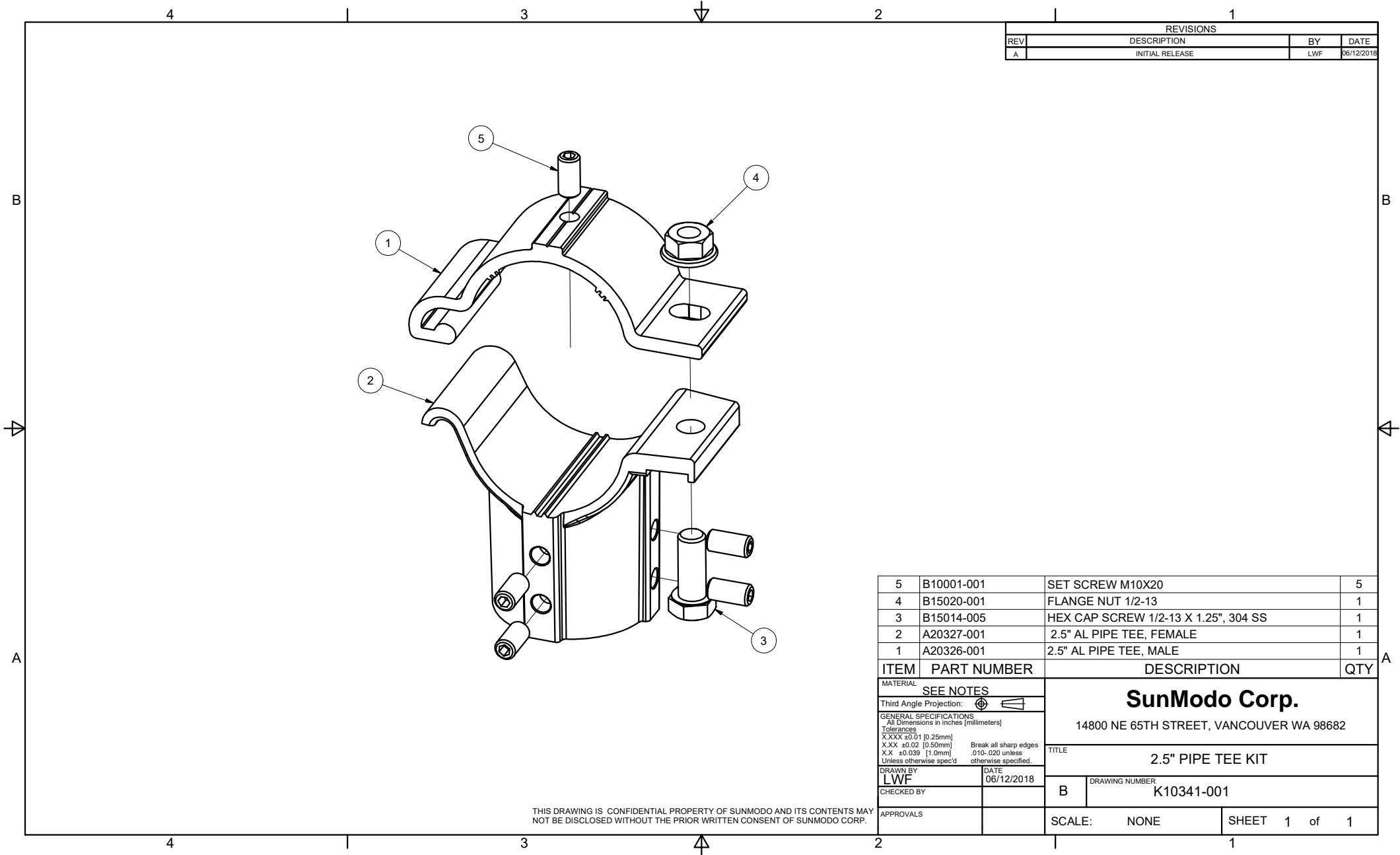


REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	zcg	09/25/2015

ITEM	PART NUMBER	DESCRIPTION	QTY
7	B10001-002	SET SCREW M10X10	2
6	B15048-001	STAR WASHER FOR 3/8 BOLT	1
5	B15018-004	HEX CAP SCREW 3/8-16 X2.5	1
4	B15003-001	FLANGE NUT 3/8-16	2
3	B15018-002	HEX CAP SCREW 3/8-16 X 1.25	1
2	A20239-001	2.5 AL PIPE CLAMP, FEMALE	1
1	A20238-001	2.5 AL PIPE CLAMP, MALE	1

MATERIAL		SEE BOM	
Third Angle Projection:			
GENERAL SPECIFICATIONS All Dimensions in inches [millimeters]			
Tolerances			
X.XX ±0.01 (0.25mm)		Break all sharp edges	
X.XX ±0.02 (0.51mm)		.010-.020 unless	
X.X ±0.03 (0.76mm)		otherwise specified.	
Unless otherwise specified			
DRAWN BY	DATE	TITLE	
LWF	08/21/2015	2.5 AL PIPE CLAMP KIT	
CHECKED BY		DRAWING NUMBER	
		B	K10222-001
APPROVALS		SCALE:	NONE
		SHEET	1 of 1

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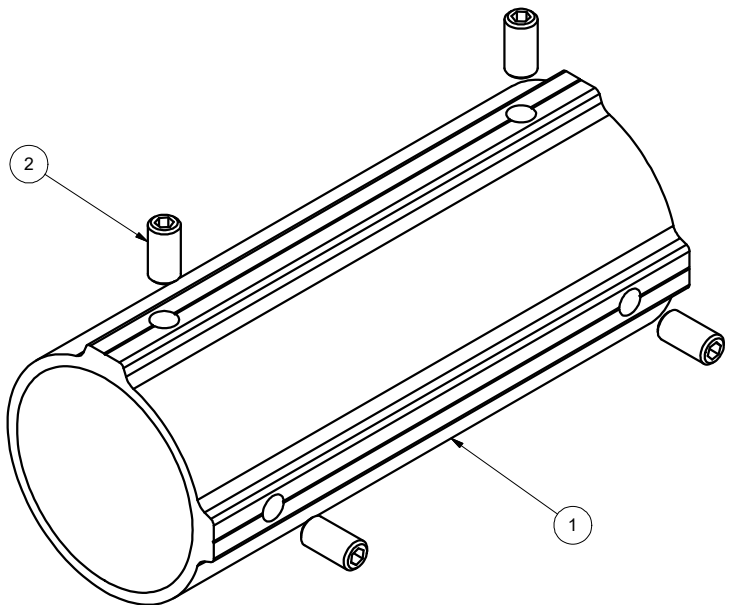
REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	06/12/2018

ITEM	PART NUMBER	DESCRIPTION	QTY
5	B10001-001	SET SCREW M10X20	5
4	B15020-001	FLANGE NUT 1/2-13	1
3	B15014-005	HEX CAP SCREW 1/2-13 X 1.25", 304 SS	1
2	A20327-001	2.5" AL PIPE TEE, FEMALE	1
1	A20326-001	2.5" AL PIPE TEE, MALE	1

MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		<b>SunModo Corp.</b> 14800 NE 65TH STREET, VANCOUVER WA 98682	
Tolerances		TITLE	
X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] X.X ±0.039 [1.0mm] Unless otherwise spec'd		2.5" PIPE TEE KIT	
DRAWN BY		DATE	
LWF		06/12/2018	
CHECKED BY		DRAWING NUMBER	
		B K10341-001	
APPROVALS		SCALE: NONE SHEET 1 of 1	

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REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	06/12/2018



2	B10001-001	SET SCREW M10X20	4
1	A20328-001	2.5" PIPE SPLICE	1
ITEM	PART NUMBER	DESCRIPTION	QTY
MATERIAL		<p style="text-align: center;"><b>SunModo Corp.</b></p> <p style="text-align: center;">14800 NE 65TH STREET, VANCOUVER WA 98682</p>	
SEE NOTES			
Third Angle Projection:		TITLE	
GENERAL SPECIFICATIONS All Dimensions in inches [millimeters] Tolerances X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] X.X ±0.039 [1.0mm] Unless otherwise spec'd		2.5" PIPE SPLICE KIT	
DRAWN BY		DRAWING NUMBER	
LWF		B K10342-001	
DATE		SCALE:	
06/12/2018		NONE	
CHECKED BY		SHEET 1 of 1	
APPROVALS			

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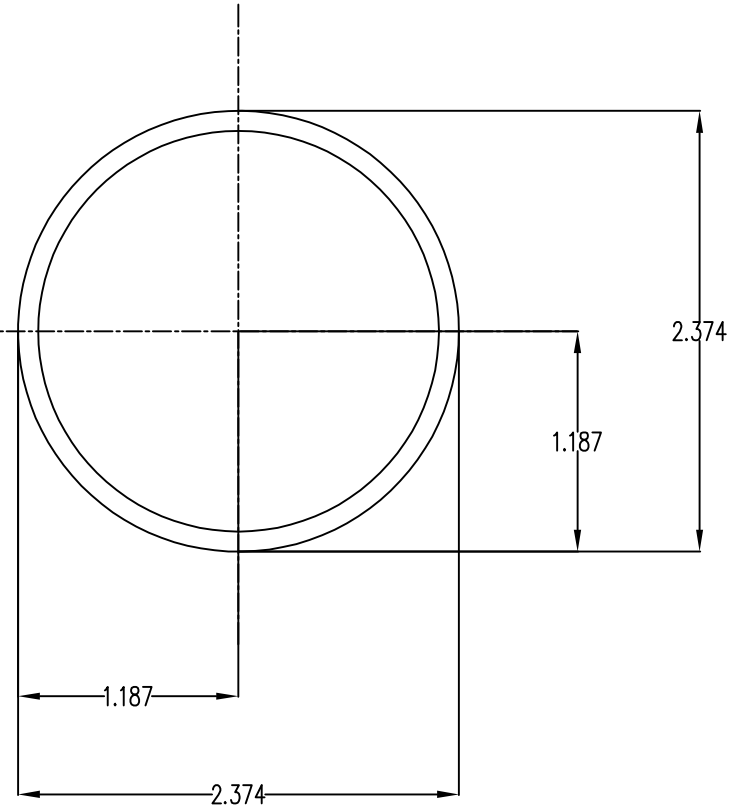
NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERIAL:HIGH STRENGTH STEEL PIPE OR TUBE.
3. FINISH: HOT DIP GALVANIZE PER ASTM A123 / A123M - 02.

MINIMUM 50 KSI YIELD STRESS.

4. BREAK ALL BURRS AND SHARP EDGES.

5. ALL WELDING MUST BE IN COMPLIANCE WITH AWS CODE D1;1.



## Section properties:

Weight: 2.641 lbs/ft

Area: 0.776 in<sup>2</sup>

Perimeter: 14.238 in

Bounding Box: X: -1.187,1.187

Y: -1.187,1.187

Centroid:(0.000,0.000)

Moments of Inertia(in<sup>4</sup>): Ix=0.499,Iy=0.499

Section modulus in bending(in<sup>3</sup>): Wx=0.420,Wy=0.420

Radii of Gyration: X: 0.802, Y: 0.802

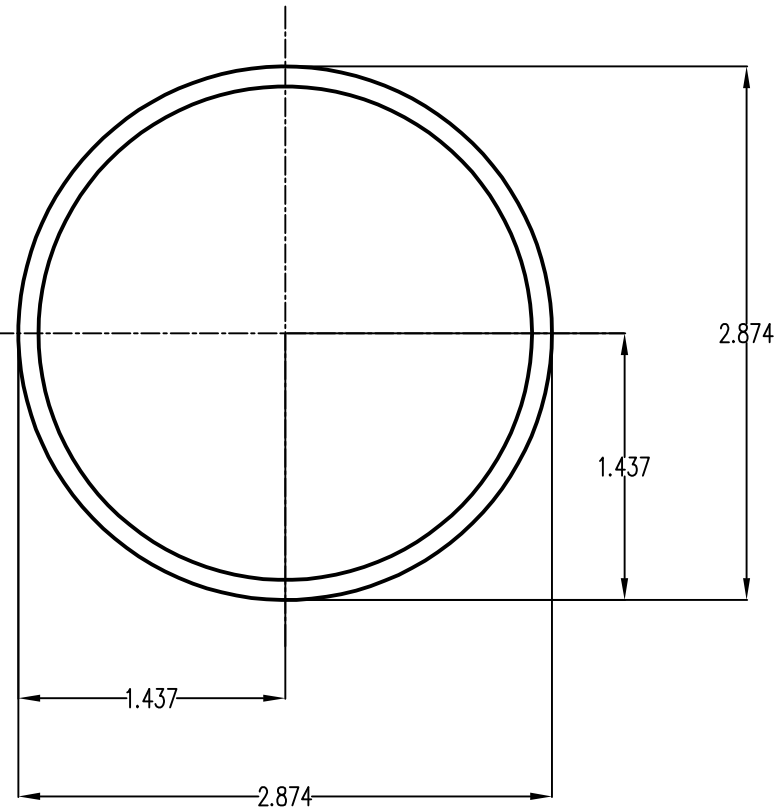
MATERIAL		SEE NOTES		<b>Sunmodo Corp.</b> 14800 NE 85TH STREET, VANCOUVER WA 98682	
Third Angle Projection:					
GENERAL SPECIFICATIONS				TITLE	
All Dimensions in Inches (millimeters)				PIPE, HSS, 2.375" OD X 12 GAUGE,L=XXX	
Tolerances				Break all sharp edges .010-.020 unless otherwise specified.	
XXX ±0.01 (0.25mm)					
XX ±0.02 (0.50mm)					
X ±0.050 (1.27mm)					
Unless otherwise specified					
DRAWN BY	DATE	B		DRAWING NUMBER	
LWF	04/03/2019			A21165	
CHECKED BY					
APPROVALS				SCALE: NONE SHEET 1 of 1	

NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERAIL:HIGH STRENGTH STEEL PIPE OR TUBE.
3. FINISH: HOT DIP GALVANIZE PER ASTM A123 / A123M - 02.

MINIMUM 50 KSI YIELD STRESS.

4. BREAK ALL BURRS AND SHARP EDGES.
5. ALL WELDING MUST BE IN COMPLIANCE WITH AWS CODE D1;1.



### Section properties:

Weight: 3.201 lbs/ft

Area: 0.941 in<sup>2</sup>

Perimeter: 17.378 in

Bounding Box: X: -1.437,1.437

Y: -1.437,1.437

Centroid:(0.000,0.000)

Moments of Inertia(in<sup>4</sup>): Ix=0.901,Iy=0.901

Section modulus in bending(in<sup>3</sup>): Wx=0.627,Wy=0.627

Radii of Gyration: X: 0.979, Y: 0.979

MATERIAL		SEE NOTES		<b>Sunmodo Corp.</b> 14800 NE 85TH STREET, VANCOUVER WA 98682	
Third Angle Projection:					
GENERAL SPECIFICATIONS				TITLE	
All Dimensions in Inches (millimeters)				PIPE, HSS, 2.875" OD X 12 GAUGE,L=XXX	
Tolerances				Break all sharp edges .010-.020 unless otherwise specified.	
XXX ±0.01 (0.25mm)					
XX ±0.02 (0.50mm)					
X ±0.050 (1.27mm)					
Unless otherwise specified					
DRAWN BY	DATE	B		DRAWING NUMBER	
LWF	04/03/2019			A21168	
CHECKED BY					
APPROVALS				SCALE: NONE SHEET 1 of 1	