



Project Number: U2716-103-191

April 6, 2021

Sunmodo
14800 NE 65th Street
Vancouver, WA 98682

**REFERENCE: Sunmodo Sunturf Ground Mount D1 (85x45)
Ground Mount PV Array Installation**

To Whom It May Concern:

Per request of Sunmodo, we have been asked to prepare the structural design of a ground-mounted PV solar array system with several foundation options as shown in the attached calculations. The adopted building code in this jurisdiction is the Florida Building Code, 2017 Edition (2015 IBC). Vector Structural Engineering requires that we review each site specific install, and we are not liable for installs at site specific locations we have not reviewed. The following design parameters are used in our analysis:

- Minimum Design Loads for Buildings and Other Structures (ASCE 7-10)
- Design wind speed for risk category I structures: 170 mph
- Wind exposure: C
- Ground snow load: 0 psf
- The ground screws and helical piers must be tested to 1.5 times uplift and 2.0 times lateral reactions found in the table below. A minimum of one ground screw or helical pier must be tested.

Load (ASD)	Value (lbs)	Factor of Safety	Test Value (lbs)
UPLIFT	2059	1.5	3089
LATERAL	954	2	1908

Foundation concrete shall have a minimum compressive strength of 2500 psi at 28 days. Cement for all concrete shall be Type I or II with a water/cement ratio of 0.50. Maximum aggregate size shall be 3/4". No special inspection of concrete strength is required.

Footings are designed based on an allowable soil bearing pressure of 1500 psf an allowable skin friction of 250 psf, an allowable lateral bearing pressure of 150 pcf, and a coefficient of friction of 0.3. Vector Structural Engineering strongly recommends independent soils testing be performed by a licensed geotechnical engineer to verify the assumed soil parameters.

All ground mounts are to be installed per manufacturer’s recommendations. The use of solar panel support span tables provided by the manufacturer is allowed only where the site conditions and solar panel configuration match the description of the span tables. Electrical engineering is beyond our scope. All work performed must be in accordance with accepted industry-wide methods and applicable safety standards. Vector Structural Engineering assumes no responsibility for improper installation of the solar panels.

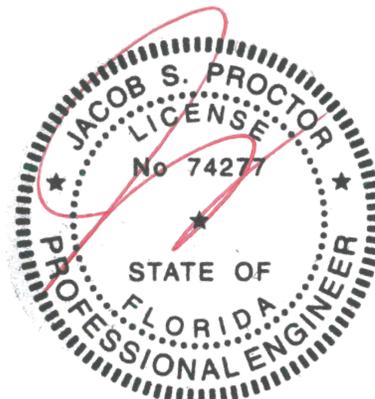
Very truly yours,

VECTOR STRUCTURAL ENGINEERING, LLC
Firm License: COA 26626

Jacob Proctor, P.E.
License: 74277 - Expires: 02/28/2023
Project Engineer

Enclosures

JSP/stb

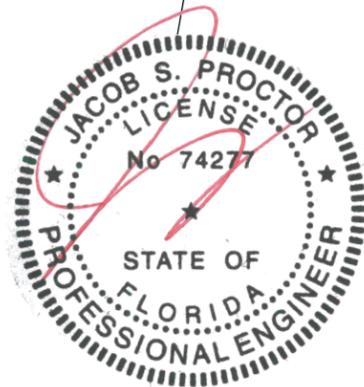
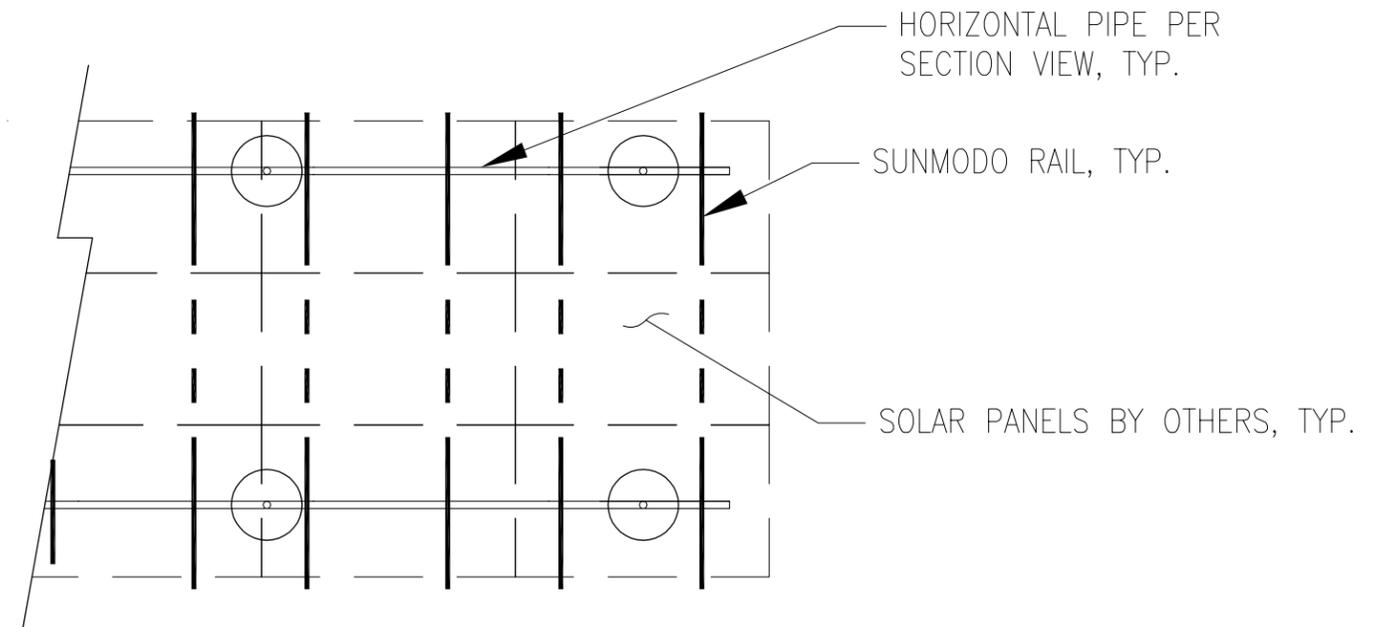
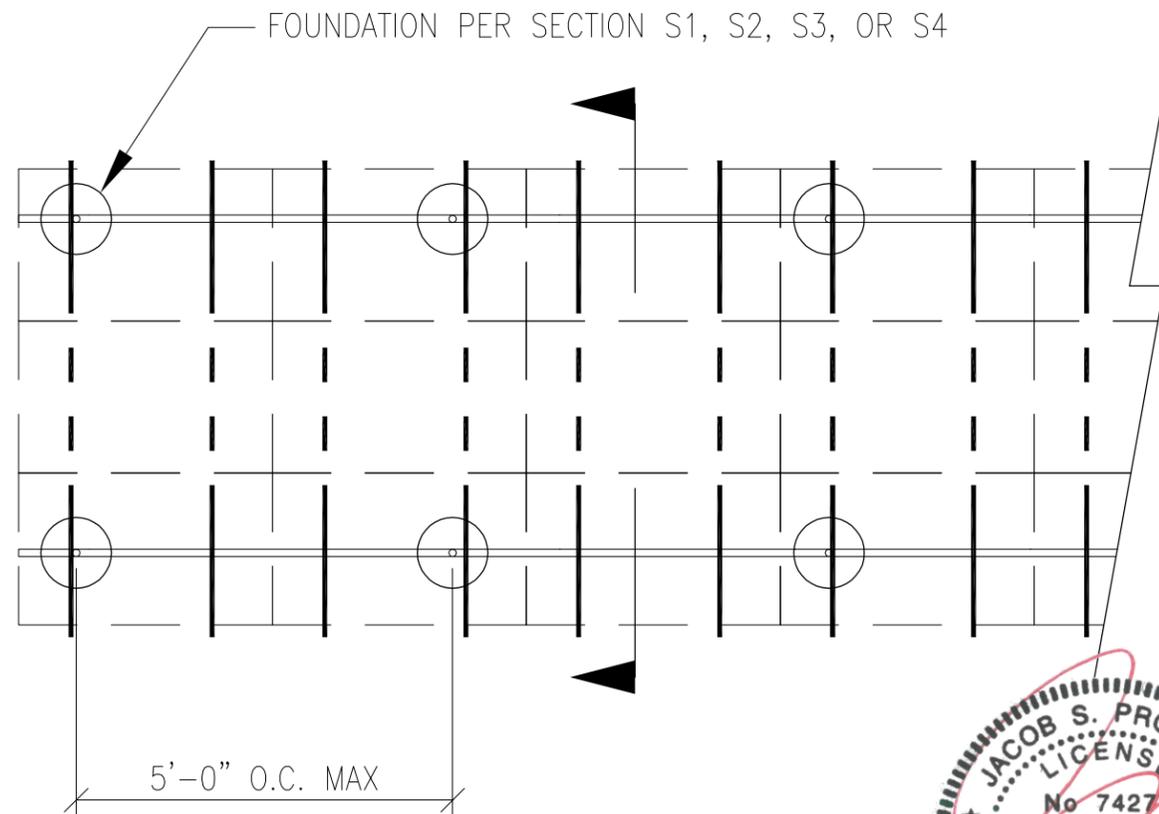


04/06/2021



JOB NO. U2716-103-191
 PROJECT SUNMODO SUNTURF GROUND MOUNTS D1
 SUBJECT ALL OPTIONS

651 W GALENA PARK BLVD. #101 (801) 990-1775
 DRAPER, UTAH 84020 (801) 990-1776 FAX



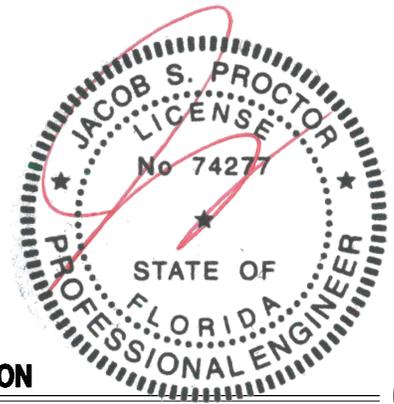
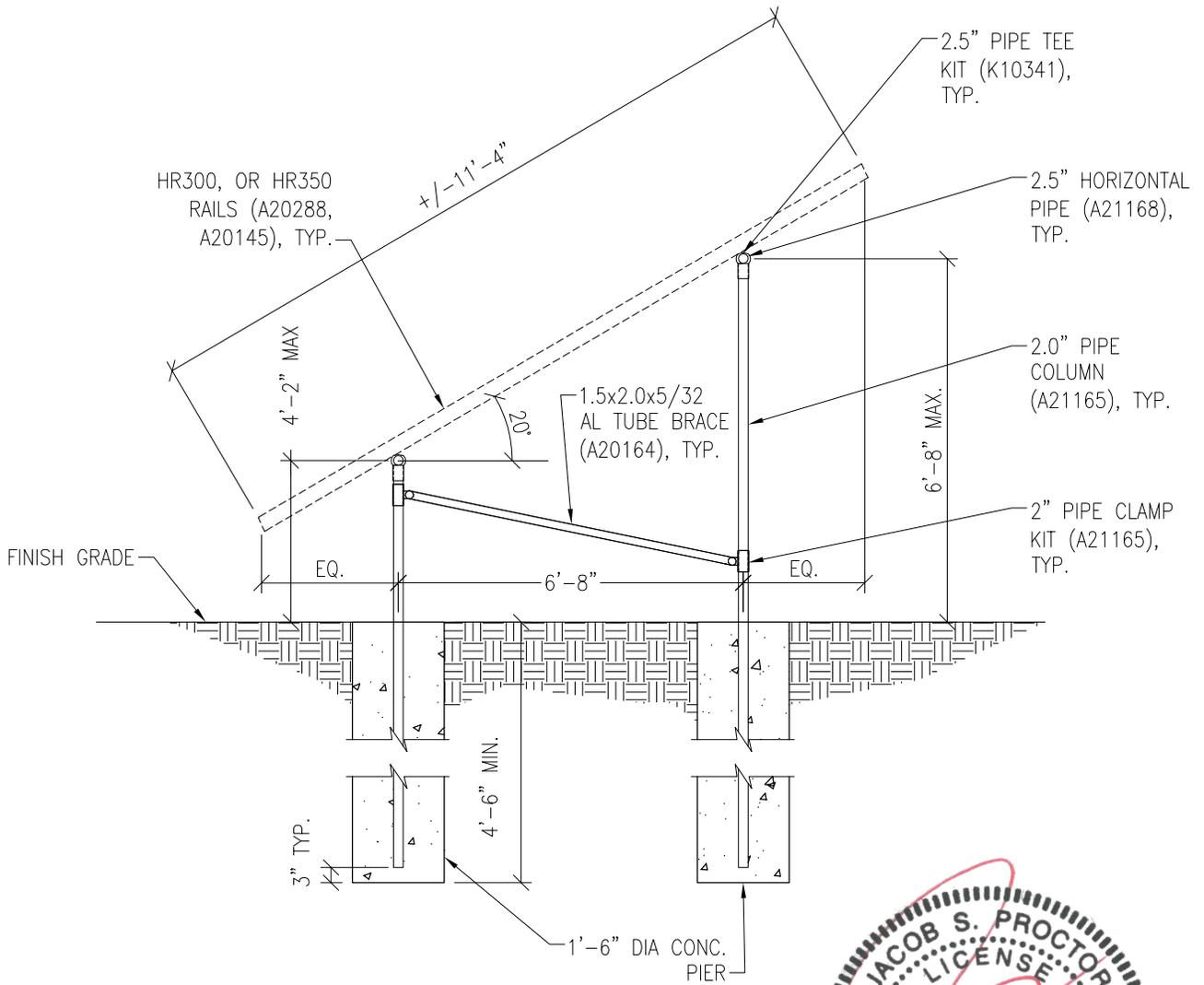
PV ARRAY PLAN

N.T.S.

P1

PROJECT SUNMODO SUNTURF GROUND MOUNTS D1

SUBJECT DRILLED PIER OPTION



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PV ARRAY SECTION

N.T.S.

S1

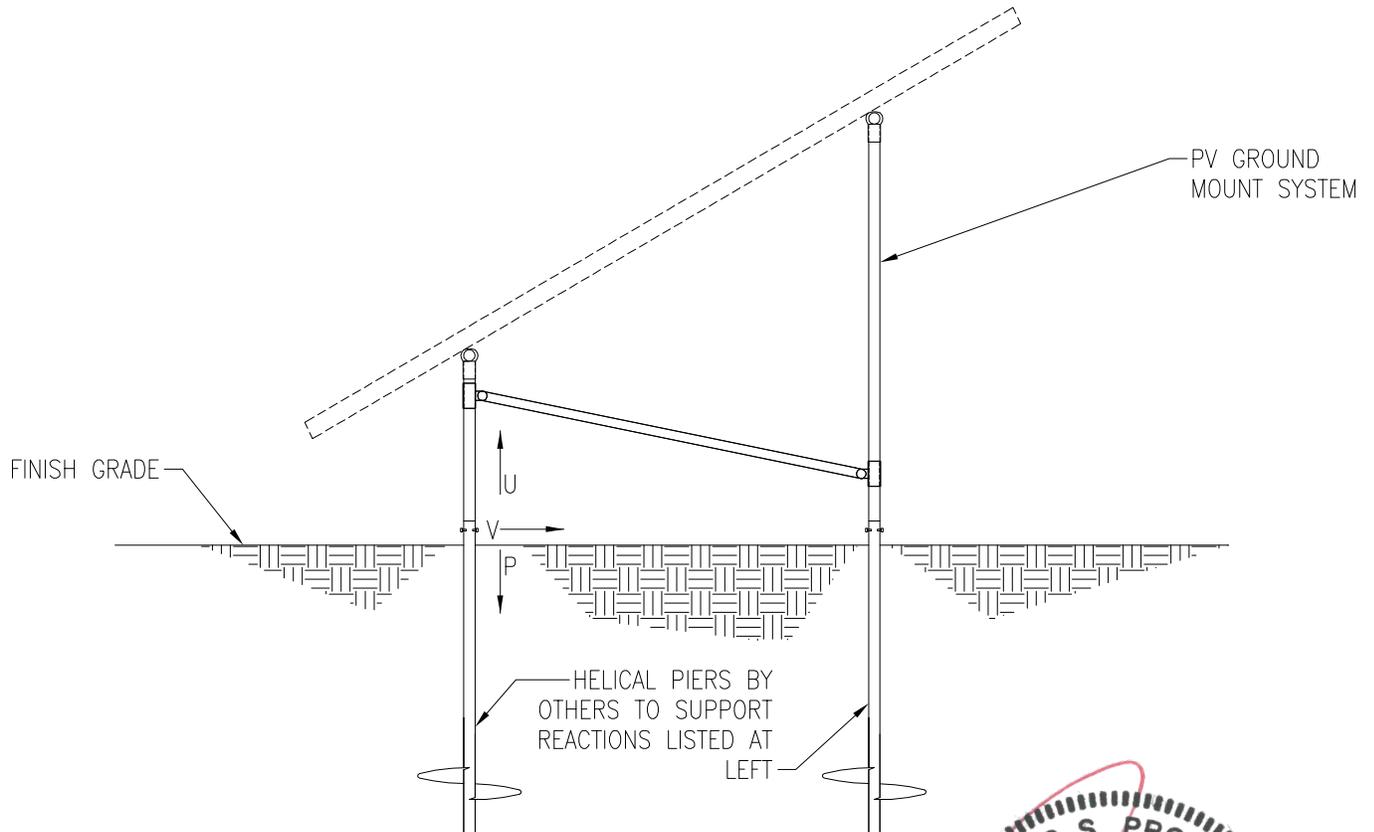
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PROJECT SUNMODO SUNTURF GROUND MOUNTS D1

SUBJECT HELICAL PIER OPTION

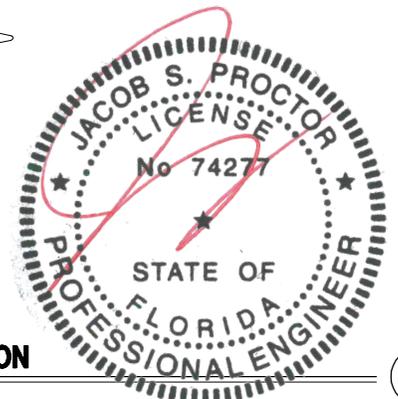
NOTES:

1. For ground mount components see Section S1.
2. A minimum of (1) helical pier must be load-tested as follows:
 - 2.1. Safety factor for uplift = 1.5,
 - 2.2. Safety factor for lateral loads = 2.0
 - 2.3. Upward deflection limit = 1/2"
 - 2.4. Lateral deflection limit = 1"
 - 2.5. The load tests must be performed by an approved contractor.



MAXIMUM REACTIONS:	
U	= 2,059 LBS
P	= 2,206 LBS
V	= 954 LBS

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PV ARRAY SECTION

N.T.S.

S2



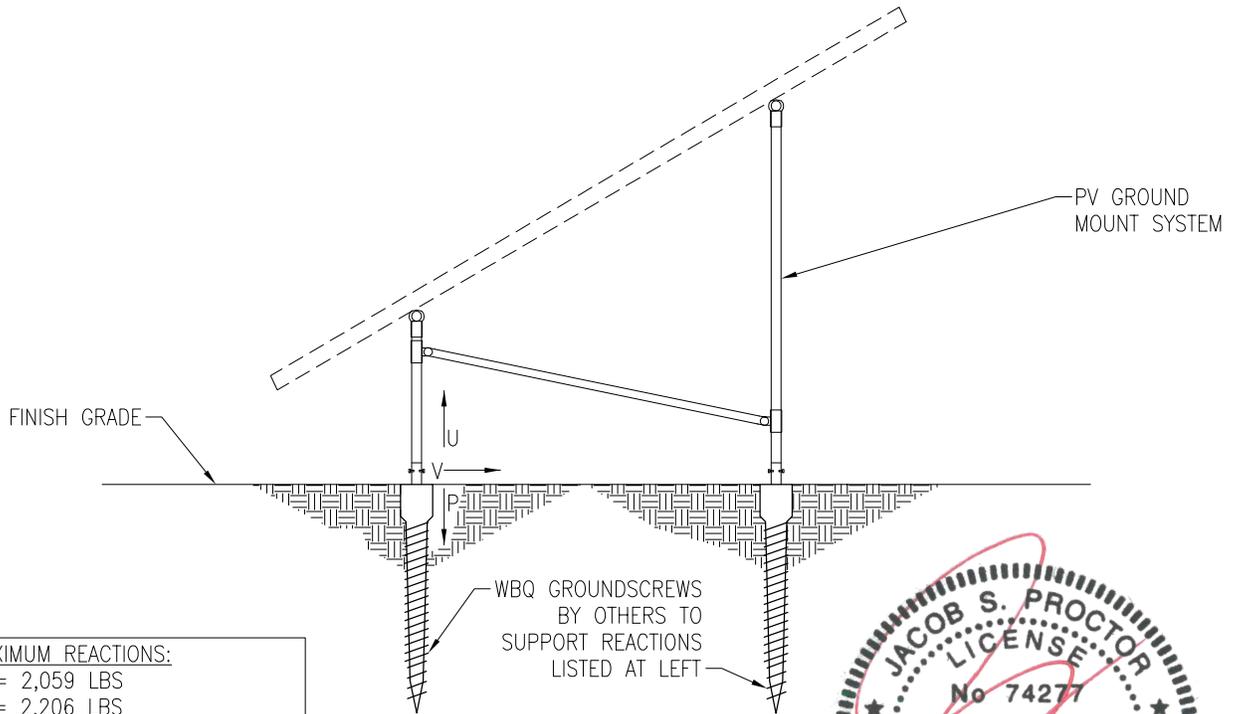
JOB NO. U2716-103-191

PROJECT SUNMODO SUNTURF GROUND MOUNTS D1

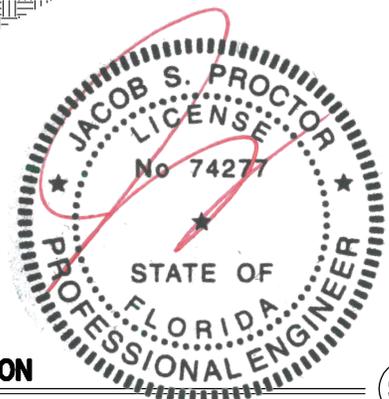
SUBJECT GROUND SCREW OPTION

NOTES:

- 1. For ground mount components see Section S1.
- 2. A minimum of (1) ground screw must be load-tested as follows:
 - 2.1. Safety factor for uplift = 1.5,
 - 2.2. Safety factor for lateral loads = 2.0
 - 2.3. Upward deflection limit = 1/2"
 - 2.4. Lateral deflection limit = 1"
 - 2.5. The load tests must be performed by an approved contractor.



MAXIMUM REACTIONS:
 U = 2,059 LBS
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PV ARRAY SECTION

N.T.S.

S3



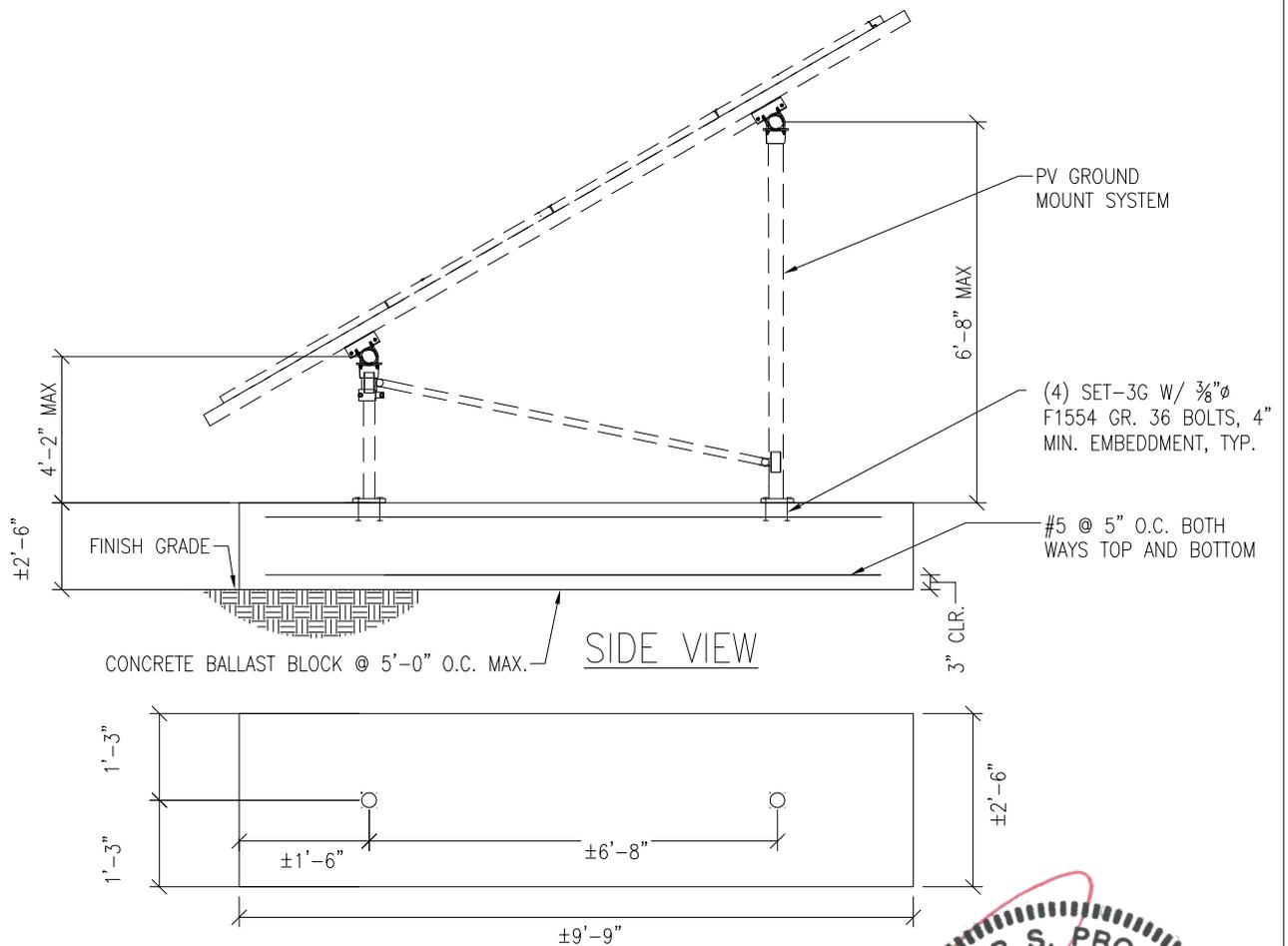
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PROJECT SUNMOD0 SUNTURF GROUND MOUNTS D1

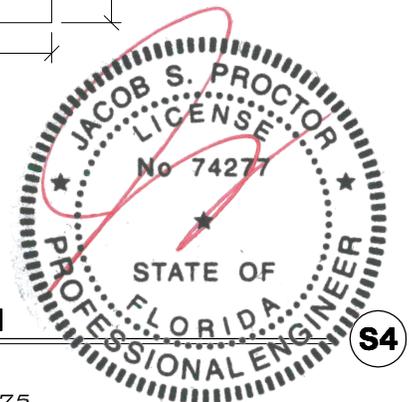
SUBJECT BALLASTED BLOCK OPTION

NOTES:

1. For ground mount components see Section S1.



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PV ARRAY SECTION

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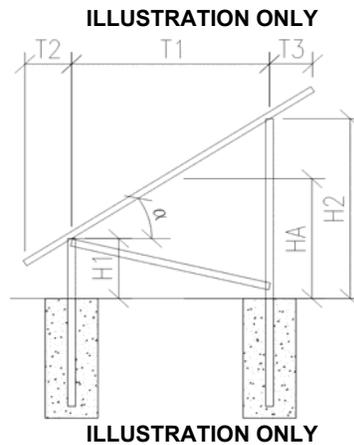
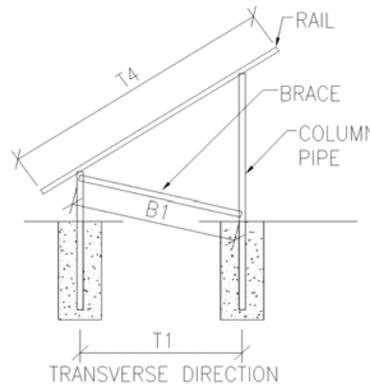
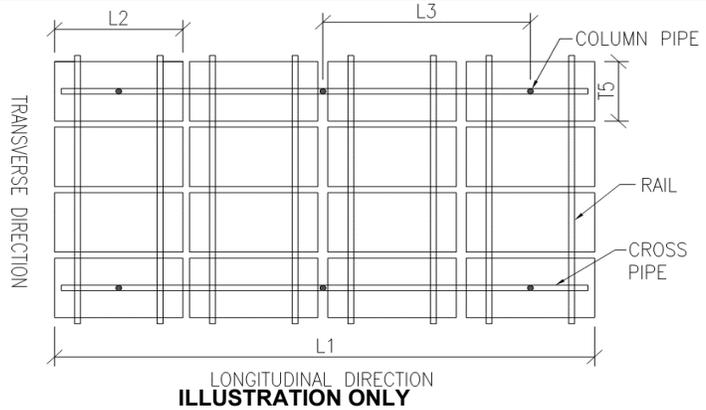
S4

PROJECT: D1 - Florida GM

SUBJECT: Dead Load

Design Weight:

Individual Panel Weight [lb]:	50.7
Panel Transverse Length (T5) [in]:	39.1
Panel Transverse Length (T5) [ft]:	3.3
Panel Longitudinal Length (L2) [in]:	77.0
Panel Longitudinal Length (L2) [ft]:	6.4
Individual Panel Area [ft ²]:	20.9
Individual Panel Weight [psf]:	2.4
# of Panels in Transverse Direction:	4
Approximate Transverse Length (T4) [ft]:	13.0
# of Panels in Longitudinal Direction:	12
Approximate Longitudinal Length (L1) [ft]:	77.0
Transverse Column Spacing (T1) [ft]:	7.8
Longitudinal Column Spacing (L3) [ft]:	5.0
# of Columns in Longitudinal Direction:	16
# of Columns in Transverse Direction:	2
Total Number of Columns:	32
Panel Slope from Horizontal (a) [°]:	20.0
Short Column Height (H1) [ft]:	4.0
Approximate Tall Column Height (H2) [ft]:	6.8
Transverse Brace between Columns :	Yes
Approximate Brace Length (B1) [ft]:	9.0
Weight of Columns [plf]:	3.7
Weight of Cross Pipe [plf]:	3.7
Weight of Brace [plf]:	3.7
Tributary Transverse Length per Column [ft]:	6.5
Tributary Longitudinal Length per Column [ft]:	5.0
Tributary Area per Column [ft ²]:	32.6
Rail Weight [plf]:	1.0
Transverse Rail Weight per Column [lb]:	13.0
Longitudinal Rail Weight per Column [lb]:	18.3
Tall Column Weight [lb]:	24.9
Panel Weight per Column [lb]:	79.0
Rail Weight per Column [lb]:	13.0
Cross Pipe Weight per Column [lb]:	18.3
Brace Weight per Column [lb]:	16.4
Total Weight per Column (1.0 D) [lb]:	151.7



Assumptions:

- T2 = T3



JOB NO.: U2716-103-191

DESIGNED: STB

DATE: 07/31/19

PROJECT: D1 - Florida GM

SUBJECT: Snow Load

SNOW LOAD (S):

ASCE 7 Standard:	10	
Panel Slope from Horizontal [°]:	20.0	
Snow Ground Load, p_g [psf]:	0.0	(Section 7.2)
Terrain Category:	C	(Table 7-2)
Exposure of Roof:	Fully Exposed	(Table 7-2)
Exposure Factor, C_e :	0.9	(Table 7-2)
Thermal Factor, C_t :	1.2	(Table 7-3)
Risk Category:	I	(Table 1.5-1)
Importance Factor, I_S :	0.8	(Table 1.5-2)
Flat Roof Snow Load, p_f [psf]:	0	(Equation 7.3-1)
Minimum Roof Snow Load, p_m [psf]:	0	(Section 7.3.4)
Unobstructed Slippery Surface?	Yes	(Section 7.4)
Slope Factor Figure:	Figure 7-2c	(Section 7.4)
Roof Slope Factor, C_s :	0.909	(Figure 7-2)
Sloped Roof Snow Load, p_s [psf]:	0	(Equation 7.4-1)
Design Snow Load, S [psf]:	0	
Tributary Transverse Length [ft]:	6.1	
Tributary Longitudinal Length [ft]:	5	
Tributary Area per Column [ft ²]:	30.6	
Snow Load per Column (1.0 S) [lb]:	0.0	



PROJECT: D1 - Florida GM

SUBJECT: Wind Pressure

Design Wind Load:

ASCE 7 Standard:	10	
Basic Wind Speed, V [mph]:	170	
Risk Category:	I	
Exposure Category	C	(Section 26.7.3)
Velocity Pressure Exposure Coefficient, K_h :	0.85	(Table 27.3-1)
Topographic Factor, K_{ht} :	1.0	(Section 26.8.2)
Wind Directionality Factor, K_d :	0.85	(Table 26.6-1)
Internal Pressure Coefficient, GC_{pi} :	0.00	(Table 26.11-1)
Velocity Pressure, q_h [psf]:	53.5	(Equation 27.3-1)
Gust Effect Factor, G:	0.85	(Section 26.9.1)
Panel Slope [degrees]:	20.0	

Net Pressure Coefficients (C_N) per: (Figure 27.4-4)

Clear Wind Flow	C_{NW}	C_{NL}
Case 1 ($\gamma = 0^\circ$, Load Case A)	-1.30	-1.50
Case 2 ($\gamma = 0^\circ$, Load Case B)	-2.23	-0.20
Case 3 ($\gamma = 180^\circ$, Load Case A)	1.57	1.73
Case 4 ($\gamma = 180^\circ$, Load Case B)	2.07	0.67

Design Wind Pressures (p) [psf] per: (Equation 27.4-3)

Clear Wind Flow	$q_h GC_{NW}$	$q_h GC_{NL}$
Case 1 ($\gamma = 0^\circ$, Load Case A)	-59.1	-68.2
Case 2 ($\gamma = 0^\circ$, Load Case B)	-101.5	-9.1
Case 3 ($\gamma = 180^\circ$, Load Case A)	71.2	78.8
Case 4 ($\gamma = 180^\circ$, Load Case B)	93.9	30.3

Wind Pressure on Each Side of Panels [psf]

Clear Wind Flow	Short Col. Pressure	Long Col. Pressure
Case 1 ($\gamma = 0^\circ$, Load Case A)	-68.2	-59.1
Case 2 ($\gamma = 0^\circ$, Load Case B)	-9.1	-101.5
Case 3 ($\gamma = 180^\circ$, Load Case A)	71.2	78.8
Case 4 ($\gamma = 180^\circ$, Load Case B)	93.9	30.3



JOB NO.: U2716-103-191

PROJECT: D1 - Florida GM

SUBJECT: Open Building Wind Loads

Design Wind Load Per ASCE 7-10

$$p = q_h G C_n$$

Velocity Pressure Exposure Coefficient, K_{zt} :	0.85	(Table 27.3-1)
Topographic Factor, K_{ht} :	1.0	(Section 26.8.2)
Wind Directionality Factor, K_d :	0.85	(Table 26.6-1)
Ultimate Wind Speed, V [mph]:	170	

Velocity Pressure, q_h [psf]:	53.5	(Equation 27.3-1)
Gust Effect Factor, G :	0.85	(Section 26.9.1)

$\gamma = 90^\circ$ or 270°

Force Coefficient, C_N :

Horizontal Distance from Winward Edge	Roof angle		
	Load Case	Obstructed Wind Flow	
		CN	
$\leq h$	A	-0.8	
	B	0.8	
$> h, \leq 2h$	A	-0.6	
	B	0.5	
$> 2h$	A	-0.3	
	B	0.3	

Design Wind Pressure, p [psf]:

	Roof angle		
	Load Case	Obstructed Wind Flow	
$\leq h$	A	-36.3	
	B	36.3	
$> h, \leq 2h$	A	-27.3	
	B	22.7	
$> 2h$	A	-13.6	
	B	13.6	



JOB NO.: U2716-103-191

DESIGNED: STB

Foundation Option 1: Drilled Concrete Pier



PROJECT: D1 - Sunturf Ground Mount

DRILLED CONCRETE PIER DESIGN

Column Reactions:

Max. Shear, V [k]:	1.0	Max. Down, P _d [k]:	2.2
Max. Moment, M [k-ft]:	0.0	Max. Uplift, P _u [k]:	2.1

Pier Properties:

Pier Shape:	Round	Volume of Concrete [ft ³]:	8
Pier Diameter, b [ft]:	1.5	Volume of Concrete [yd ³]:	0.3
Top of Pier Elevation [ft]:	0.00	Weight of Concrete [k]:	1.2
Pier Depth, d [ft]:	4.5		

Soil Properties:

Allow. Bearing Pressure [psf]:	1,500	<u>Optional Parameters for Uplift:</u>	
1/3 increase for short term loads?	No		
Lateral Bearing, S [pcf]:	150	Skin Friction* [psf]:	250
Max. Lateral Bearing (opt'l) [psf]:		Top Length to Ignore [ft]:	0
Top Depth to Ignore [ft]:	0	1/3 increase for short term loads?	No
1/3 increase for short term loads?	No	Combine w/ Bearing:	No
1/2" deflection at t/o pier allowed:	Yes		

*per IBC Section 1810.3.3.1.4

Check Bearing:

Bearing Capacity [k]:	5.3
-----------------------	-----

Bearing capacity OK.

Check Uplift:

Uplift Capacity [k]:	6.4
----------------------	-----

Uplift capacity OK.

Check Lateral Bearing:

Top of Pier Constrained?:	No
Applied Lateral Force, P [lb]:	954
Point of Application, h [ft]:	0.0
S _{max} [psf]:	
S [psf]:	450
A = 2.34*P/(Sb):	3.31
Required Pier Depth, d _{reqd} [ft]:	3.30

IBC Section 1807.3.2.1

IBC Eq. 18-1

Result: **Lateral bearing capacity OK.**

Foundation Option 2: Helical Pier

The ground screws and helical piers must be tested to 1.5 times uplift and 2.0 times lateral reactions found in the table below.

Load (ASD)	Value (lbs)	Factor of Safety	Test Value (lbs)
UPLIFT	2059	1.5	3089
LATERAL	954	2	1908

Foundation Option 3: Ground Screw

The ground screws and helical piers must be tested to 1.5 times uplift and 2.0 times lateral reactions found in the table below.

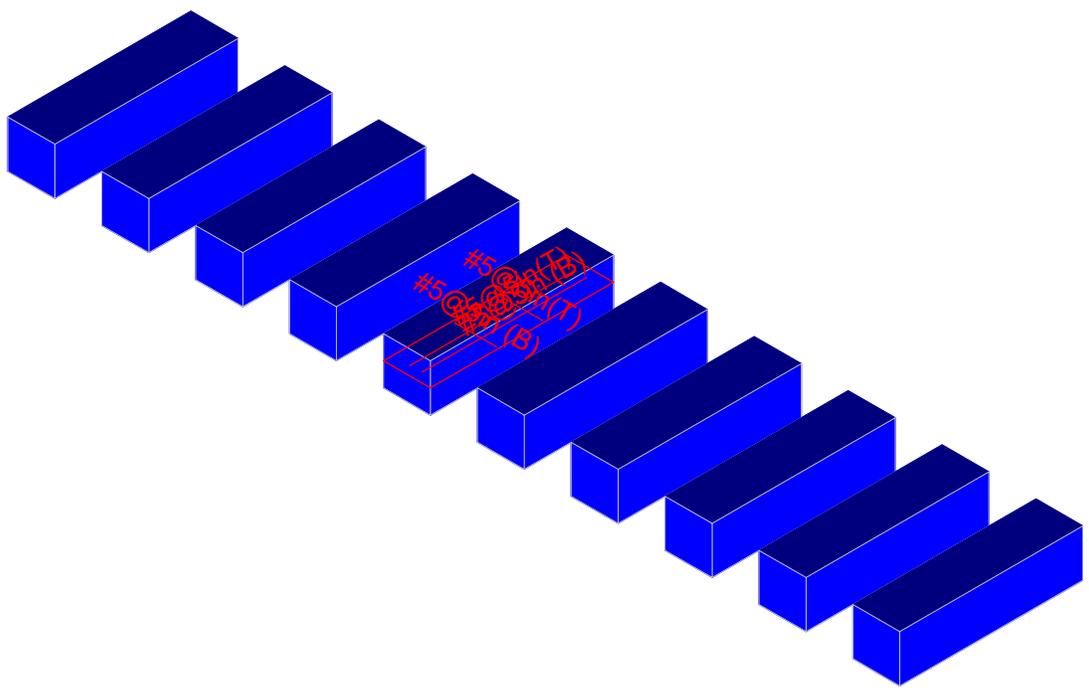
Load (ASD)	Value (lbs)	Factor of Safety	Test Value (lbs)
UPLIFT	2059	1.5	3089
LATERAL	954	2	1908



JOB NO.: U2716-103-191

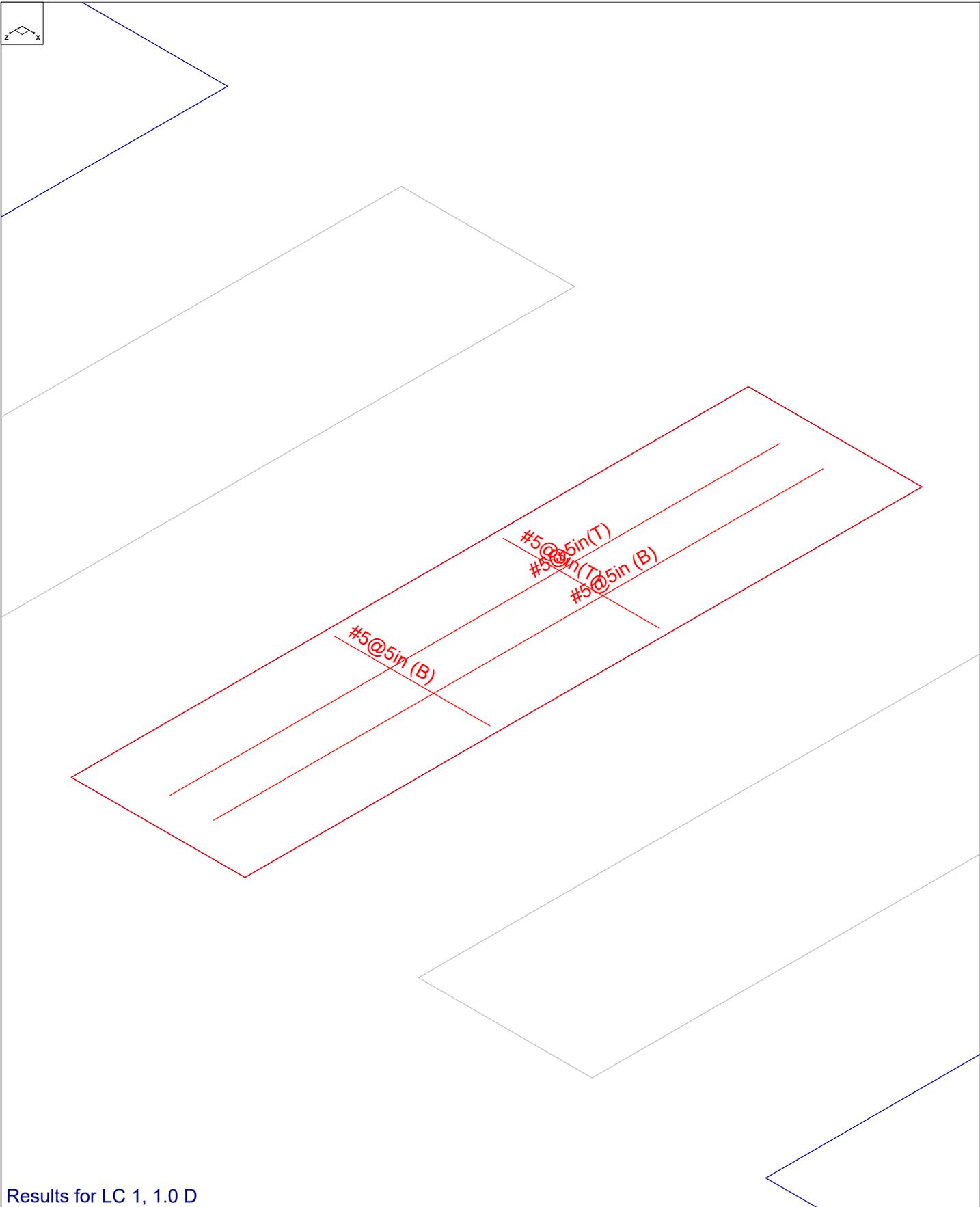
DESIGNED: STB

Foundation Option 4: Ballasted Block



Results for LC 1, 1.0 D

Vector Structural Engineeri..	Ground Mount	SK - 2
STB		Apr 6, 2021 at 3:46 PM
U2716.103.191		Sunmodo Sunturf D1 GM v7 85x45...



Results for LC 1, 1.0 D

Vector Structural Engineeri...	Ground Mount	SK - 1
STB		Apr 6, 2021 at 3:46 PM
U2716.103.191		Sunmodo Sunturf D1 GM v7 85x45...



(Global) Model Settings

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	100
Mesh Size (in)	12
Max Iterations	10
Merge Tolerance (in)	.12
Solver	Sparse Accelerated
Coefficient of Friction	.3
No. of Shear Regions	4
Shear Region Spacing Increment (in)	4
Min 1 Bar Dia Spacing for Beams?	No
Optimize footings for OTM / Sliding?	Yes
Parme Beta Factor	.65
Pile Safety Factor	3
Concrete Stress Block	0
Concrete Rebar Set	Rectangular
Concrete Code	ASTM A615
HR Steel Pile Code	ACI 318-14
Wood Pile Code	AISC 14th (360-10): ASD AWC NDS-15: ASD

Concrete Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (/...	Density[lb/ft^3]	fc[psi]	Lambda	Flex Stee...	Shear St...
1	Conc3000NW	3156	1372	.15	.6	145	3000	1	60000	60000
2	Conc3500NW	3409	1482	.15	.6	145	3500	1	60000	60000
3	Conc4000NW	3644	1584	.15	.6	145	4000	1	60000	60000
4	Conc3000LW	2085	907	.15	.6	109.999	3000	.75	60000	60000
5	Conc3500LW	2252	979	.15	.6	109.999	3500	.75	60000	60000
6	Conc4000LW	2408	1047	.15	.6	109.999	4000	.75	60000	60000
7	Conc2500NW	3156	1372	.15	.6	145	2500	1	60000	60000

General Design Parameters

	Label	Max Bending Chk	Max Shear Chk	Top Cover[in]	Bottom Cover[in]
1	Typical	1	1	3	3

Slab Rebar Parameters

	Label	Top Bar	Bottom Bar	Max Top Bar Sp...	Min Top Bar Sp...	Max Bot Bar Sp...	Min Bot Bar Sp...	Spacing Incr...	Rebar Options
1	Typical	#5	#5	5	5	5	5	1	Optimize

Soil Definitions

	Label	Subgrade Modulus[lb/ft^3]	Allowable Bearing[psf]	Depth Properties	Default?
1	Default	1e+5	1500	None	Yes

Point Loads and Moments (Cat 1 : DL)

	Label	Direction	Magnitude[lb.-ft]
1	R3D_N1	X	-4.683
2	R3D_N1	Y	142.732
3	R3D_N2	X	2.573
4	R3D_N2	Y	117.681
5	R3D_N132	Y	136.914
6	R3D_N133	Y	125.902
7	R3D_N109	Y	129.582
8	R3D_N110A	Y	127.35

Point Loads and Moments (Cat 1 : DL) (Continued)

	Label	Direction	Magnitude[lb,lb-ft]
9	R3D_N121	Y	136.631
10	R3D_N122	X	-1.829
11	R3D_N122	Y	123.802
12	R3D_N133B	Y	130.489
13	R3D_N134B	Y	119.568
14	R3D_N151	Y	136.962
15	R3D_N152	Y	125.996
16	R3D_N157A_1	Y	130.684
17	R3D_N158A_1	Y	119.759
18	R3D_N155B_1	Y	136.18
19	R3D_N156B_1	X	1.8
20	R3D_N156B_1	Y	123.373
21	R3D_N167	Y	130.856
22	R3D_N168	Y	128.257
23	R3D_N179	X	4.672
24	R3D_N179	Y	140.105
25	R3D_N180	X	-2.317
26	R3D_N180	Y	115.378

Point Loads and Moments (Cat 16 : OL1)

	Label	Direction	Magnitude[lb,lb-ft]
1	R3D_N1	X	82.314
2	R3D_N1	Y	-2653.524
3	R3D_N1	Z	1229.601
4	R3D_N2	X	-41.341
5	R3D_N2	Y	-683.242
6	R3D_N2	Z	-39.73
7	R3D_N132	X	-8.969
8	R3D_N132	Y	-2735.224
9	R3D_N132	Z	1333.352
10	R3D_N133	X	-17.038
11	R3D_N133	Y	-745.904
12	R3D_N133	Z	-41.549
13	R3D_N109	X	1.76
14	R3D_N109	Y	-2554.778
15	R3D_N109	Z	1265.706
16	R3D_N110A	X	-1.292
17	R3D_N110A	Y	-815.91
18	R3D_N110A	Z	-40.666
19	R3D_N121	X	16.144
20	R3D_N121	Y	-2738.005
21	R3D_N121	Z	1348.65
22	R3D_N122	X	30.461
23	R3D_N122	Y	-699.342
24	R3D_N122	Z	-41.44
25	R3D_N133B	X	-5.928
26	R3D_N133B	Y	-2468.262
27	R3D_N133B	Z	1169.176
28	R3D_N134B	X	-9.933
29	R3D_N134B	Y	-779.691
30	R3D_N134B	Z	-39.579
31	R3D_N151	X	8.06
32	R3D_N151	Y	-2730.432
33	R3D_N151	Z	1327.395
34	R3D_N152	X	16.583
35	R3D_N152	Y	-753.228

Point Loads and Moments (Cat 16 : OL1) (Continued)

	Label	Direction	Magnitude[lb,lb-ft]
36	R3D_N152	Z	-41.504
37	R3D_N157A_1	X	6.759
38	R3D_N157A_1	Y	-2478.353
39	R3D_N157A_1	Z	1176.259
40	R3D_N158A_1	X	12.584
41	R3D_N158A_1	Y	-776.476
42	R3D_N158A_1	Z	-39.646
43	R3D_N155B_1	X	-16.639
44	R3D_N155B_1	Y	-2725.095
45	R3D_N155B_1	Z	1342.683
46	R3D_N156B_1	X	-30.06
47	R3D_N156B_1	Y	-697.254
48	R3D_N156B_1	Z	-41.369
49	R3D_N167	X	-1.802
50	R3D_N167	Y	-2582.799
51	R3D_N167	Z	1276.59
52	R3D_N168	X	2.472
53	R3D_N168	Y	-822.633
54	R3D_N168	Z	-40.598
55	R3D_N179	X	-81.699
56	R3D_N179	Y	-2592.503
57	R3D_N179	Z	1201.749
58	R3D_N180	X	37.565
59	R3D_N180	Y	-666.94
60	R3D_N180	Z	-39.217

Point Loads and Moments (Cat 17 : OL2)

	Label	Direction	Magnitude[lb,lb-ft]
1	R3D_N1	X	118.152
2	R3D_N1	Y	-3491.962
3	R3D_N1	Z	1036.151
4	R3D_N2	X	-2.017
5	R3D_N2	Y	568.882
6	R3D_N2	Z	-33.451
7	R3D_N132	X	-12.933
8	R3D_N132	Y	-3563.876
9	R3D_N132	Z	1148.566
10	R3D_N133	X	-2.514
11	R3D_N133	Y	629.736
12	R3D_N133	Z	-35.361
13	R3D_N109	X	3.674
14	R3D_N109	Y	-3259.136
15	R3D_N109	Z	1063.433
16	R3D_N110A	Y	539.789
17	R3D_N110A	Z	-34.392
18	R3D_N121	X	23.023
19	R3D_N121	Y	-3569.614
20	R3D_N121	Z	1167.154
21	R3D_N122	X	1.611
22	R3D_N122	Y	650.869
23	R3D_N122	Z	-35.25
24	R3D_N133B	X	-8.499
25	R3D_N133B	Y	-3190.949
26	R3D_N133B	Z	941.832
27	R3D_N134B	Y	445.367
28	R3D_N134B	Z	-32.85

Point Loads and Moments (Cat 17 : OL2) (Continued)

	Label	Direction	Magnitude[lb,lb-ft]
29	R3D_N151	X	11.629
30	R3D_N151	Y	-3557.007
31	R3D_N151	Z	1139.845
32	R3D_N152	X	2.452
33	R3D_N152	Y	621.425
34	R3D_N152	Z	-35.292
35	R3D_N157A_1	X	9.706
36	R3D_N157A_1	Y	-3205.323
37	R3D_N157A_1	Z	951.322
38	R3D_N158A_1	Y	453.849
39	R3D_N158A_1	Z	-32.942
40	R3D_N155B_1	X	-23.757
41	R3D_N155B_1	Y	-3551.226
42	R3D_N155B_1	Z	1160.336
43	R3D_N156B_1	X	-1.548
44	R3D_N156B_1	Y	645.55
45	R3D_N156B_1	Z	-35.167
46	R3D_N167	X	-3.679
47	R3D_N167	Y	-3296.391
48	R3D_N167	Z	1072.263
49	R3D_N168	Y	543.564
50	R3D_N168	Z	-34.312
51	R3D_N179	X	-117.32
52	R3D_N179	Y	-3412.664
53	R3D_N179	Z	1013.155
54	R3D_N180	X	1.563
55	R3D_N180	Y	557.762
56	R3D_N180	Z	-33.043

Point Loads and Moments (Cat 18 : OL3)

	Label	Direction	Magnitude[lb,lb-ft]
1	R3D_N1	X	-105.512
2	R3D_N1	Y	3343.441
3	R3D_N1	Z	-1442.99
4	R3D_N2	X	42.521
5	R3D_N2	Y	592.727
6	R3D_N2	Z	46.62
7	R3D_N132	X	11.507
8	R3D_N132	Y	3439.88
9	R3D_N132	Z	-1569.311
10	R3D_N133	X	17.831
11	R3D_N133	Y	645.502
12	R3D_N133	Z	48.823
13	R3D_N109	X	-2.466
14	R3D_N109	Y	3200.228
15	R3D_N109	Z	-1484.787
16	R3D_N110A	X	1.34
17	R3D_N110A	Y	733.312
18	R3D_N110A	Z	47.746
19	R3D_N121	X	-20.666
20	R3D_N121	Y	3443.763
21	R3D_N121	Z	-1588.305
22	R3D_N122	X	-31.353
23	R3D_N122	Y	594.166
24	R3D_N122	Z	48.692
25	R3D_N133B	X	7.597

Point Loads and Moments (Cat 18 : OL3) (Continued)

	Label	Direction	Magnitude[lb,lb-ft]
26	R3D_N133B	Y	3099.56
27	R3D_N133B	Z	-1364.151
28	R3D_N134B	X	10.216
29	R3D_N134B	Y	713.63
30	R3D_N134B	Z	46.356
31	R3D_N151	X	-10.342
32	R3D_N151	Y	3433.739
33	R3D_N151	Z	-1561.644
34	R3D_N152	X	-17.357
35	R3D_N152	Y	654.488
36	R3D_N152	Z	48.764
37	R3D_N157A_1	X	-8.664
38	R3D_N157A_1	Y	3112.474
39	R3D_N157A_1	Z	-1373.106
40	R3D_N158A_1	X	-12.944
41	R3D_N158A_1	Y	708.803
42	R3D_N158A_1	Z	46.441
43	R3D_N155B_1	X	21.305
44	R3D_N155B_1	Y	3427.242
45	R3D_N155B_1	Z	-1580.975
46	R3D_N156B_1	X	30.932
47	R3D_N156B_1	Y	593.01
48	R3D_N156B_1	Z	48.604
49	R3D_N167	X	2.509
50	R3D_N167	Y	3235.604
51	R3D_N167	Z	-1497.498
52	R3D_N168	X	-2.6
53	R3D_N168	Y	739.477
54	R3D_N168	Z	47.662
55	R3D_N179	X	104.732
56	R3D_N179	Y	3266.738
57	R3D_N179	Z	-1410.391
58	R3D_N180	X	-38.588
59	R3D_N180	Y	578.137
60	R3D_N180	Z	46.022

Point Loads and Moments (Cat 19 : OL4)

	Label	Direction	Magnitude[lb,lb-ft]
1	R3D_N1	X	-54.868
2	R3D_N1	Y	1941.593
3	R3D_N1	Z	-1217.193
4	R3D_N2	X	58.829
5	R3D_N2	Y	1300.795
6	R3D_N2	Z	39.345
7	R3D_N132	X	5.947
8	R3D_N132	Y	2020.795
9	R3D_N132	Z	-1306.266
10	R3D_N133	X	23.327
11	R3D_N133	Y	1424.831
12	R3D_N133	Z	40.939
13	R3D_N109	Y	1925.465
14	R3D_N109	Z	-1254.649
15	R3D_N110A	X	1.807
16	R3D_N110A	Y	1477.244
17	R3D_N110A	Z	40.188
18	R3D_N121	X	-10.843

Point Loads and Moments (Cat 19 : OL4) (Continued)

	Label	Direction	Magnitude[lb.-ft]
19	R3D_N121	Y	2021.694
20	R3D_N121	Z	-1318.303
21	R3D_N122	X	-43.278
22	R3D_N122	Y	1368.862
23	R3D_N122	Z	40.842
24	R3D_N133B	X	3.957
25	R3D_N133B	Y	1837.248
26	R3D_N133B	Z	-1181.055
27	R3D_N134B	X	14.135
28	R3D_N134B	Y	1373.228
29	R3D_N134B	Z	39.454
30	R3D_N151	X	-5.339
31	R3D_N151	Y	2017.595
32	R3D_N151	Z	-1302.388
33	R3D_N152	X	-22.702
34	R3D_N152	Y	1430.914
35	R3D_N152	Z	40.912
36	R3D_N157A_1	X	-4.502
37	R3D_N157A_1	Y	1844.034
38	R3D_N157A_1	Z	-1186.145
39	R3D_N158A_1	X	-17.905
40	R3D_N158A_1	Y	1373.195
41	R3D_N158A_1	Z	39.501
42	R3D_N155B_1	X	11.16
43	R3D_N155B_1	Y	2013.013
44	R3D_N155B_1	Z	-1313.372
45	R3D_N156B_1	X	42.731
46	R3D_N156B_1	Y	1362.935
47	R3D_N156B_1	Z	40.784
48	R3D_N167	Y	1945.761
49	R3D_N167	Z	-1265.61
50	R3D_N168	X	-3.344
51	R3D_N168	Y	1489.049
52	R3D_N168	Z	40.133
53	R3D_N179	X	54.43
54	R3D_N179	Y	1896.396
55	R3D_N179	Z	-1189.363
56	R3D_N180	X	-53.603
57	R3D_N180	Y	1271.099
58	R3D_N180	Z	38.824

Point Loads and Moments (Cat 20 : OL5)

	Label	Direction	Magnitude[lb.-ft]
1	R3D_N1	X	-25.104
2	R3D_N1	Y	1632.075
3	R3D_N1	Z	-736.718
4	R3D_N2	X	31.658
5	R3D_N2	Y	349.128
6	R3D_N2	Z	21.681
7	R3D_N132	X	2.047
8	R3D_N132	Y	582.213
9	R3D_N132	Z	-274.17
10	R3D_N133	Y	127.422
11	R3D_N133	Z	9.716
12	R3D_N109	X	-3.781
13	R3D_N109	Y	1219.843

Point Loads and Moments (Cat 20 : OL5) (Continued)

	Label	Direction	Magnitude[lb,lb-ft]
14	R3D_N109	Z	-586.988
15	R3D_N110A	X	-9.073
16	R3D_N110A	Y	338.335
17	R3D_N110A	Z	19.045
18	R3D_N121	X	-3.956
19	R3D_N121	Y	1144.505
20	R3D_N121	Z	-538.922
21	R3D_N122	X	-9.579
22	R3D_N122	Y	264.541
23	R3D_N122	Z	16.115
24	R3D_N133B	X	4.763
25	R3D_N133B	Y	696.868
26	R3D_N133B	Z	-319.94
27	R3D_N134B	X	4.871
28	R3D_N134B	Y	175.573
29	R3D_N134B	Z	11.734
30	R3D_N151	X	-1.09
31	R3D_N151	Y	610.188
32	R3D_N151	Z	-284.128
33	R3D_N152	X	-5.116
34	R3D_N152	Y	138.874
35	R3D_N152	Z	9.078
36	R3D_N157A_1	Y	548.808
37	R3D_N157A_1	Z	-249.951
38	R3D_N158A_1	X	-4.648
39	R3D_N158A_1	Y	143.55
40	R3D_N158A_1	Z	8.502
41	R3D_N155B_1	X	4.273
42	R3D_N155B_1	Y	607.123
43	R3D_N155B_1	Z	-286.638
44	R3D_N156B_1	X	3.912
45	R3D_N156B_1	Y	124.35
46	R3D_N156B_1	Z	8.838
47	R3D_N167	X	1.346
48	R3D_N167	Y	558.286
49	R3D_N167	Z	-271.537
50	R3D_N168	X	-2.832
51	R3D_N168	Y	156.244
52	R3D_N168	Z	8.662
53	R3D_N179	X	22.433
54	R3D_N179	Y	585.065
55	R3D_N179	Z	-256.886
56	R3D_N180	X	-9.015
57	R3D_N180	Y	118.915
58	R3D_N180	Z	8.365

Point Loads and Moments (Cat 21 : OL6)

	Label	Direction	Magnitude[lb,lb-ft]
1	R3D_N1	X	27.232
2	R3D_N1	Y	-1642.708
3	R3D_N1	Z	739.564
4	R3D_N2	X	-30.173
5	R3D_N2	Y	-352.788
6	R3D_N2	Z	-21.106
7	R3D_N132	X	-2.253
8	R3D_N132	Y	-588.633

Point Loads and Moments (Cat 21 : OL6) (Continued)

	Label	Direction	Magnitude[lb.-ft]
9	R3D_N132	Z	276.726
10	R3D_N133	Y	-129.474
11	R3D_N133	Z	-9.45
12	R3D_N109	X	2.114
13	R3D_N109	Y	-1084.706
14	R3D_N109	Z	526.911
15	R3D_N110A	X	5.751
16	R3D_N110A	Y	-303.44
17	R3D_N110A	Z	-17.519
18	R3D_N121	X	3.507
19	R3D_N121	Y	-955.33
20	R3D_N121	Z	450.557
21	R3D_N122	X	9.28
22	R3D_N122	Y	-218.246
23	R3D_N122	Z	-14.202
24	R3D_N133B	X	-3.92
25	R3D_N133B	Y	-646.438
26	R3D_N133B	Z	295.71
27	R3D_N134B	X	-3.166
28	R3D_N134B	Y	-165.144
29	R3D_N134B	Z	-10.753
30	R3D_N151	X	1.068
31	R3D_N151	Y	-609.355
32	R3D_N151	Z	284.11
33	R3D_N152	X	5.331
34	R3D_N152	Y	-137.961
35	R3D_N152	Z	-9.01
36	R3D_N157A_1	Y	-548.927
37	R3D_N157A_1	Z	249.886
38	R3D_N158A_1	X	4.764
39	R3D_N158A_1	Y	-143.776
40	R3D_N158A_1	Z	-8.482
41	R3D_N155B_1	X	-4.323
42	R3D_N155B_1	Y	-607.18
43	R3D_N155B_1	Z	286.635
44	R3D_N156B_1	X	-3.778
45	R3D_N156B_1	Y	-124.302
46	R3D_N156B_1	Z	-8.833
47	R3D_N167	X	-1.418
48	R3D_N167	Y	-557.594
49	R3D_N167	Z	271.541
50	R3D_N168	X	2.966
51	R3D_N168	Y	-156.297
52	R3D_N168	Z	-8.66
53	R3D_N179	X	-22.877
54	R3D_N179	Y	-585.757
55	R3D_N179	Z	256.883
56	R3D_N180	X	9.128
57	R3D_N180	Y	-118.769
58	R3D_N180	Z	-8.365

Slabs

	Label	Thickness [in]	Material	Local Axis Angle ...	Analysis Offset [in]	Passive Pressur...	Soil Overburden [psf]
1	S1	30	Conc2500NW	0	0	0	0
2	S2	30	Conc2500NW	0	0	0	0
3	S3	30	Conc2500NW	0	0	0	0



Company : Vector Structural Engineering
 Designer : STB
 Job Number : U2716.103.191
 Model Name : Ground Mount

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Slab Sliding Safety Factors (Continued)

	LC	Slab	Angle[deg]	Va-xx[lb]	Vr-xx[lb]	Va-zz[lb]	Vr-zz[lb]	SR-xx	SR-zz
3	1	S3	0	1.829	2728.916	0	2728.916	9.999+	9.999+
4	1	S4	0	0	2725.803	0	2725.803	9.999+	9.999+
5	1	S5	0	0	2729.63	0	2729.63	9.999+	9.999+
6	1	S6	0	0	2729.673	0	2729.673	9.999+	9.999+
7	1	S7	0	0	2725.919	0	2725.919	9.999+	9.999+
8	1	S8	0	1.8	2728.652	0	2728.652	9.999+	9.999+
9	1	S9	0	0	2728.52	0	2728.52	9.999+	9.999+
10	1	S10	0	2.355	2727.431	0	2727.431	9.999+	9.999+
11	2	S1	0	2.111	2728.91	0	2728.91	9.999+	9.999+
12	2	S2	0	0	2727.865	0	2727.865	9.999+	9.999+
13	2	S3	0	1.829	2728.916	0	2728.916	9.999+	9.999+
14	2	S4	0	0	2725.803	0	2725.803	9.999+	9.999+
15	2	S5	0	0	2729.63	0	2729.63	9.999+	9.999+
16	2	S6	0	0	2729.673	0	2729.673	9.999+	9.999+
17	2	S7	0	0	2725.919	0	2725.919	9.999+	9.999+
18	2	S8	0	1.8	2728.652	0	2728.652	9.999+	9.999+
19	2	S9	0	0	2728.52	0	2728.52	9.999+	9.999+
20	2	S10	0	2.355	2727.431	0	2727.431	9.999+	9.999+
21	3	S1	0	22.473	2128.292	713.923	2128.292	9.999+	2.981
22	3	S2	0	.281	2121.141	735.024	2121.141	9.999+	2.886
23	3	S3	0	26.134	2110.194	784.326	2110.194	9.999+	2.69
24	3	S4	0	9.516	2141.171	677.758	2141.171	9.999+	3.159
25	3	S5	0	15.604	2103.027	775.082	2103.027	9.999+	2.713
26	3	S6	0	14.786	2102.614	771.535	2102.614	9.999+	2.725
27	3	S7	0	11.605	2140.049	681.968	2140.049	9.999+	3.138
28	3	S8	0	26.22	2112.629	780.789	2112.629	9.999+	2.706
29	3	S9	0	.402	2115.542	741.596	2115.542	9.999+	2.853
30	3	S10	0	24.126	2140.731	697.519	2140.731	9.999+	3.069
31	4	S1	0	67.571	2202.755	601.62	2202.755	9.999+	3.661
32	4	S2	0	2.205	2238.383	617.424	2238.383	9.999+	3.625
33	4	S3	0	12.952	2203.542	679.142	2203.542	9.999+	3.245
34	4	S4	0	5.099	2231.598	545.389	2231.598	9.999+	4.092
35	4	S5	0	9.268	2201.485	667.923	2201.485	9.999+	3.296
36	4	S6	0	8.449	2201.268	662.732	2201.268	9.999+	3.322
37	4	S7	0	5.823	2230.653	551.028	2230.653	9.999+	4.048
38	4	S8	0	13.383	2205.63	675.102	2205.63	9.999+	3.267
39	4	S9	0	2.207	2233.011	622.771	2233.011	9.999+	3.586
40	4	S10	0	67.099	2213.549	588.067	2213.549	9.999+	3.764
41	5	S1	0	39.905	3437.42	837.823	3437.42	9.999+	4.103
42	5	S2	0	.676	3435.902	862.225	3435.902	9.999+	3.985
43	5	S3	0	33.04	3455.743	923.768	3455.743	9.999+	3.741
44	5	S4	0	10.688	3412.177	790.677	3412.177	9.999+	4.316
45	5	S5	0	17.603	3464.999	912.293	3464.999	9.999+	3.798
46	5	S6	0	16.619	3465.554	907.728	3465.554	9.999+	3.818
47	5	S7	0	12.965	3413.749	795.999	3413.749	9.999+	4.289
48	5	S8	0	33.142	3452.297	919.423	3452.297	9.999+	3.755
49	5	S9	0	.054	3444.034	869.902	3444.034	9.999+	3.959
50	5	S10	0	42.041	3419.508	818.621	3419.508	9.999+	4.177
51	6	S1	0	.266	3312.539	706.709	3312.539	9.999+	4.687
52	6	S2	0	1.084	3340.353	728.677	3340.353	9.999+	4.584
53	6	S3	0	34.301	3339.216	766.476	3339.216	9.999+	4.357
54	6	S4	0	10.855	3303.689	684.96	3303.689	9.999+	4.823
55	6	S5	0	17.564	3349.843	759.196	3349.843	9.999+	4.412
56	6	S6	0	16.825	3350.405	756.886	3350.405	9.999+	4.427
57	6	S7	0	13.444	3305.02	687.987	3305.02	9.999+	4.804
58	6	S8	0	34.135	3336.322	763.553	3336.322	9.999+	4.369
59	6	S9	0	2.006	3346.785	735.286	3346.785	9.999+	4.552



Company : Vector Structural Engineering
 Designer : STB
 Job Number : U2716.103.191
 Model Name : Ground Mount

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Slab Sliding Safety Factors (Continued)

	LC	Slab	Angle[deg]	Va-xx[lb]	Vr-xx[lb]	Va-zz[lb]	Vr-zz[lb]	SR-xx	SR-zz
60	6	S10	0	2.851	3297.58	690.324	3297.58	9.999+	4.777
61	7	S1	0	1.822	3085.526	429.022	3085.526	9.999+	7.192
62	7	S2	0	7.712	3008.337	340.766	3008.337	9.999+	8.828
63	7	S3	0	9.949	2982.544	313.684	2982.544	9.999+	9.508
64	7	S4	0	5.781	2882.842	184.924	2882.842	9.999+	9.999+
65	7	S5	0	1.228	2857.365	158.673	2857.365	9.999+	9.999+
66	7	S6	0	3.724	2864.504	165.03	2864.504	9.999+	9.999+
67	7	S7	0	2.789	2850.543	144.869	2850.543	9.999+	9.999+
68	7	S8	0	6.711	2860.317	166.68	2860.317	9.999+	9.999+
69	7	S9	0	.892	2857.135	157.725	2857.135	9.999+	9.999+
70	7	S10	0	10.405	2854.147	149.113	2854.147	9.999+	9.999+
71	8	S1	0	3.875	2369.72	431.075	2369.72	9.999+	5.497
72	8	S2	0	4.719	2477.999	305.635	2477.999	9.999+	8.108
73	8	S3	0	5.844	2517.672	261.813	2517.672	9.999+	9.616
74	8	S4	0	4.252	2579.718	170.974	2579.718	9.999+	9.999+
75	8	S5	0	1.352	2600.371	160.365	2600.371	9.999+	9.999+
76	8	S6	0	3.84	2595.156	165.06	2595.156	9.999+	9.999+
77	8	S7	0	2.859	2601.232	144.842	2601.232	9.999+	9.999+
78	8	S8	0	3.061	2596.985	166.681	2596.985	9.999+	9.999+
79	8	S9	0	.929	2600.019	157.729	2600.019	9.999+	9.999+
80	8	S10	0	5.895	2600.616	149.111	2600.616	9.999+	9.999+
81	9	S1	0	16.327	2278.446	535.442	2278.446	9.999+	4.255
82	9	S2	0	.211	2272.822	551.268	2272.822	9.999+	4.123
83	9	S3	0	19.144	2264.874	588.245	2264.874	9.999+	3.85
84	9	S4	0	7.137	2287.329	508.318	2287.329	9.999+	4.5
85	9	S5	0	11.703	2259.678	581.312	2259.678	9.999+	3.887
86	9	S6	0	11.089	2259.379	578.651	2259.379	9.999+	3.905
87	9	S7	0	8.704	2286.517	511.476	2286.517	9.999+	4.47
88	9	S8	0	19.215	2266.634	585.592	2266.634	9.999+	3.871
89	9	S9	0	.301	2268.786	556.197	2268.786	9.999+	4.079
90	9	S10	0	17.506	2287.406	523.139	2287.406	9.999+	4.372
91	10	S1	0	50.15	2334.294	451.215	2334.294	9.999+	5.173
92	10	S2	0	1.653	2360.753	463.068	2360.753	9.999+	5.098
93	10	S3	0	9.257	2334.885	509.357	2334.885	9.999+	4.584
94	10	S4	0	3.824	2355.149	409.042	2355.149	9.999+	5.758
95	10	S5	0	6.951	2333.522	500.942	2333.522	9.999+	4.658
96	10	S6	0	6.336	2333.37	497.049	2333.37	9.999+	4.694
97	10	S7	0	4.368	2354.47	413.271	2354.47	9.999+	5.697
98	10	S8	0	9.587	2336.385	506.326	2336.385	9.999+	4.614
99	10	S9	0	1.655	2356.888	467.078	2356.888	9.999+	5.046
100	10	S10	0	49.736	2342.019	441.05	2342.019	9.999+	5.31
101	11	S1	0	30.456	3260.292	628.367	3260.292	9.999+	5.189
102	11	S2	0	.507	3258.893	646.669	3258.893	9.999+	5.04
103	11	S3	0	25.238	3274.036	692.826	3274.036	9.999+	4.726
104	11	S4	0	8.016	3240.584	593.008	3240.584	9.999+	5.465
105	11	S5	0	13.202	3281.157	684.22	3281.157	9.999+	4.795
106	11	S6	0	12.464	3281.584	680.796	3281.584	9.999+	4.82
107	11	S7	0	9.723	3241.791	596.999	3241.791	9.999+	5.43
108	11	S8	0	25.306	3271.385	689.567	3271.385	9.999+	4.744
109	11	S9	0	.041	3265.156	652.426	3265.156	9.999+	5.005
110	11	S10	0	32.119	3246.489	613.966	3246.489	9.999+	5.288
111	12	S1	0	.328	3166.632	530.032	3166.632	9.999+	5.974
112	12	S2	0	.813	3187.231	546.507	3187.231	9.999+	5.832
113	12	S3	0	26.183	3186.641	574.857	3186.641	9.999+	5.543
114	12	S4	0	8.142	3159.217	513.72	3159.217	9.999+	6.15
115	12	S5	0	13.173	3194.79	569.397	3194.79	9.999+	5.611
116	12	S6	0	12.619	3195.222	567.665	3195.222	9.999+	5.629



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Slab Sliding Safety Factors (Continued)

	LC	Slab	Angle[deg]	Va-xx[lb]	Vr-xx[lb]	Va-zz[lb]	Vr-zz[lb]	SR-xx	SR-zz
117	12	S7	0	10.083	3160.245	515.99	3160.245	9.999+	6.125
118	12	S8	0	26.051	3184.404	572.664	3184.404	9.999+	5.561
119	12	S9	0	1.505	3192.219	551.465	3192.219	9.999+	5.789
120	12	S10	0	2.727	3155.042	517.743	3155.042	9.999+	6.094
121	13	S1	0	.839	2996.372	321.767	2996.372	9.999+	9.312
122	13	S2	0	5.784	2938.219	255.575	2938.219	9.999+	9.999+
123	13	S3	0	7.919	2919.137	235.263	2919.137	9.999+	9.999+
124	13	S4	0	4.336	2843.582	138.693	2843.582	9.999+	9.999+
125	13	S5	0	.921	2825.431	119.004	2825.431	9.999+	9.999+
126	13	S6	0	2.793	2830.796	123.773	2830.796	9.999+	9.999+
127	13	S7	0	2.092	2819.387	108.652	2819.387	9.999+	9.999+
128	13	S8	0	5.483	2827.4	125.01	2827.4	9.999+	9.999+
129	13	S9	0	.669	2824.981	118.294	2824.981	9.999+	9.999+
130	13	S10	0	8.393	2822.468	111.834	2822.468	9.999+	9.999+
131	14	S1	0	3.434	2459.518	323.306	2459.518	9.999+	7.607
132	14	S2	0	3.539	2540.465	229.227	2540.465	9.999+	9.999+
133	14	S3	0	3.926	2570.483	196.36	2570.483	9.999+	9.999+
134	14	S4	0	3.189	2616.24	128.231	2616.24	9.999+	9.999+
135	14	S5	0	1.014	2632.686	120.274	2632.686	9.999+	9.999+
136	14	S6	0	2.88	2628.785	123.795	2628.785	9.999+	9.999+
137	14	S7	0	2.144	2632.404	108.632	2632.404	9.999+	9.999+
138	14	S8	0	1.846	2629.901	125.011	2629.901	9.999+	9.999+
139	14	S9	0	.697	2632.145	118.297	2632.145	9.999+	9.999+
140	14	S10	0	3.833	2632.32	111.833	2632.32	9.999+	9.999+
141	15	S1	0	23.317	1036.728	713.923	1036.728	9.999+	1.452
142	15	S2	0	.281	1029.995	735.024	1029.995	9.999+	1.401
143	15	S3	0	26.866	1018.627	784.326	1018.627	9.999+	1.299
144	15	S4	0	9.516	1050.85	677.758	1050.85	9.999+	1.55
145	15	S5	0	15.604	1011.175	775.082	1011.175	9.999+	1.305
146	15	S6	0	14.786	1010.745	771.535	1010.745	9.999+	1.31
147	15	S7	0	11.605	1049.682	681.968	1049.682	9.999+	1.539
148	15	S8	0	26.94	1021.168	780.789	1021.168	9.999+	1.308
149	15	S9	0	.402	1024.134	741.596	1024.134	9.999+	1.381
150	15	S10	0	25.068	1049.759	697.519	1049.759	9.999+	1.505
151	16	S1	0	68.415	1111.191	601.62	1111.191	9.999+	1.847
152	16	S2	0	2.205	1147.237	617.424	1147.237	9.999+	1.858
153	16	S3	0	13.683	1111.975	679.142	1111.975	9.999+	1.637
154	16	S4	0	5.099	1141.277	545.389	1141.277	9.999+	2.093
155	16	S5	0	9.268	1109.633	667.923	1109.633	9.999+	1.661
156	16	S6	0	8.449	1109.399	662.732	1109.399	9.999+	1.674
157	16	S7	0	5.823	1140.286	551.028	1140.286	9.999+	2.069
158	16	S8	0	14.103	1114.169	675.102	1114.169	9.999+	1.65
159	16	S9	0	2.207	1141.603	622.771	1141.603	9.999+	1.833
160	16	S10	0	68.041	1122.576	588.067	1122.576	9.999+	1.909
161	17	S1	0	39.061	2345.856	837.823	2345.856	9.999+	2.8
162	17	S2	0	.676	2344.756	862.225	2344.756	9.999+	2.719
163	17	S3	0	32.309	2364.177	923.768	2364.177	9.999+	2.559
164	17	S4	0	10.688	2321.856	790.677	2321.856	9.999+	2.937
165	17	S5	0	17.603	2373.147	912.293	2373.147	9.999+	2.601
166	17	S6	0	16.619	2373.685	907.728	2373.685	9.999+	2.615
167	17	S7	0	12.965	2323.381	795.999	2323.381	9.999+	2.919
168	17	S8	0	32.422	2360.836	919.423	2360.836	9.999+	2.568
169	17	S9	0	.054	2352.627	869.902	2352.627	9.999+	2.704
170	17	S10	0	41.099	2328.536	818.621	2328.536	9.999+	2.844
171	18	S1	0	1.111	2220.976	706.709	2220.976	9.999+	3.143
172	18	S2	0	1.084	2249.207	728.677	2249.207	9.999+	3.087
173	18	S3	0	33.57	2247.65	766.476	2247.65	9.999+	2.932



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Slab Sliding Safety Factors (Continued)

	LC	Slab	Angle[deg]	Va-xx[lb]	Vr-xx[lb]	Va-zz[lb]	Vr-zz[lb]	SR-xx	SR-zz
174	18	S4	0	10.855	2213.367	684.96	2213.367	9.999+	3.231
175	18	S5	0	17.564	2257.991	759.196	2257.991	9.999+	2.974
176	18	S6	0	16.825	2258.536	756.886	2258.536	9.999+	2.984
177	18	S7	0	13.444	2214.653	687.987	2214.653	9.999+	3.219
178	18	S8	0	33.415	2244.861	763.553	2244.861	9.999+	2.94
179	18	S9	0	2.006	2255.378	735.286	2255.378	9.999+	3.067
180	18	S10	0	1.909	2206.607	690.324	2206.607	9.999+	3.196
181	19	S1	0	2.666	1993.962	429.022	1993.962	9.999+	4.648
182	19	S2	0	7.712	1917.191	340.766	1917.191	9.999+	5.626
183	19	S3	0	9.218	1890.978	313.684	1890.978	9.999+	6.028
184	19	S4	0	5.781	1792.521	184.924	1792.521	9.999+	9.693
185	19	S5	0	1.228	1765.513	158.673	1765.513	9.999+	9.999+
186	19	S6	0	3.724	1772.635	165.03	1772.635	9.999+	9.999+
187	19	S7	0	2.789	1760.176	144.869	1760.176	9.999+	9.999+
188	19	S8	0	5.991	1768.856	166.68	1768.856	9.999+	9.999+
189	19	S9	0	.892	1765.727	157.725	1765.727	9.999+	9.999+
190	19	S10	0	9.463	1763.175	149.113	1763.175	9.999+	9.999+
191	20	S1	0	3.031	1278.156	431.075	1278.156	9.999+	2.965
192	20	S2	0	4.719	1386.853	305.635	1386.853	9.999+	4.538
193	20	S3	0	6.575	1426.106	261.813	1426.106	9.999+	5.447
194	20	S4	0	4.252	1489.397	170.974	1489.397	9.999+	8.711
195	20	S5	0	1.352	1508.519	160.365	1508.519	9.999+	9.407
196	20	S6	0	3.84	1503.287	165.06	1503.287	9.999+	9.107
197	20	S7	0	2.859	1510.865	144.842	1510.865	9.999+	9.999+
198	20	S8	0	3.78	1505.524	166.681	1505.524	9.999+	9.032
199	20	S9	0	.929	1508.612	157.729	1508.612	9.999+	9.565
200	20	S10	0	6.837	1509.644	149.111	1509.644	9.999+	9.999+

Envelope Slab Soil Pressures

	Label	UC	LC	Soil Pressure[psf]	Allowable Bearing[psf]	Point
1	S1	.415	5	622.308	1500	N282
2	S2	.404	5	606.136	1500	N289
3	S3	.419	5	629.027	1500	N296
4	S4	.399	5	599.177	1500	N303
5	S5	.418	5	627.572	1500	N310
6	S6	.418	5	626.905	1500	N317
7	S7	.4	5	600.299	1500	N324
8	S8	.418	5	627.732	1500	N331
9	S9	.406	5	608.869	1500	N338
10	S10	.411	5	616.438	1500	N345



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Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-14
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: F1554 Grade 36
Diameter (inch): 0.375
Effective Embedment depth, h_{ef} (inch): 4.000
Code report: ICC-ES ESR-4057
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 5.25
 c_{ac} (inch): 7.12
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 30.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Ignore concrete breakout in tension: No
Ignore concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 150/110°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.75 x 4.75 x 0.31

Recommended Anchor

Anchor Name: SET-3G - SET-3G w/ 3/8"Ø F1554 Gr. 36
Code Report: ICC-ES ESR-4057





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Load and Geometry

Load factor source: ACI 318 Section 5.3

Load combination: not set

Seismic design: No

Anchors subjected to sustained tension: No

Apply entire shear load at front row: No

Anchors only resisting wind and/or seismic loads: No

Strength level loads:

N_{ua} [lb]: 3443

V_{uax} [lb]: 130

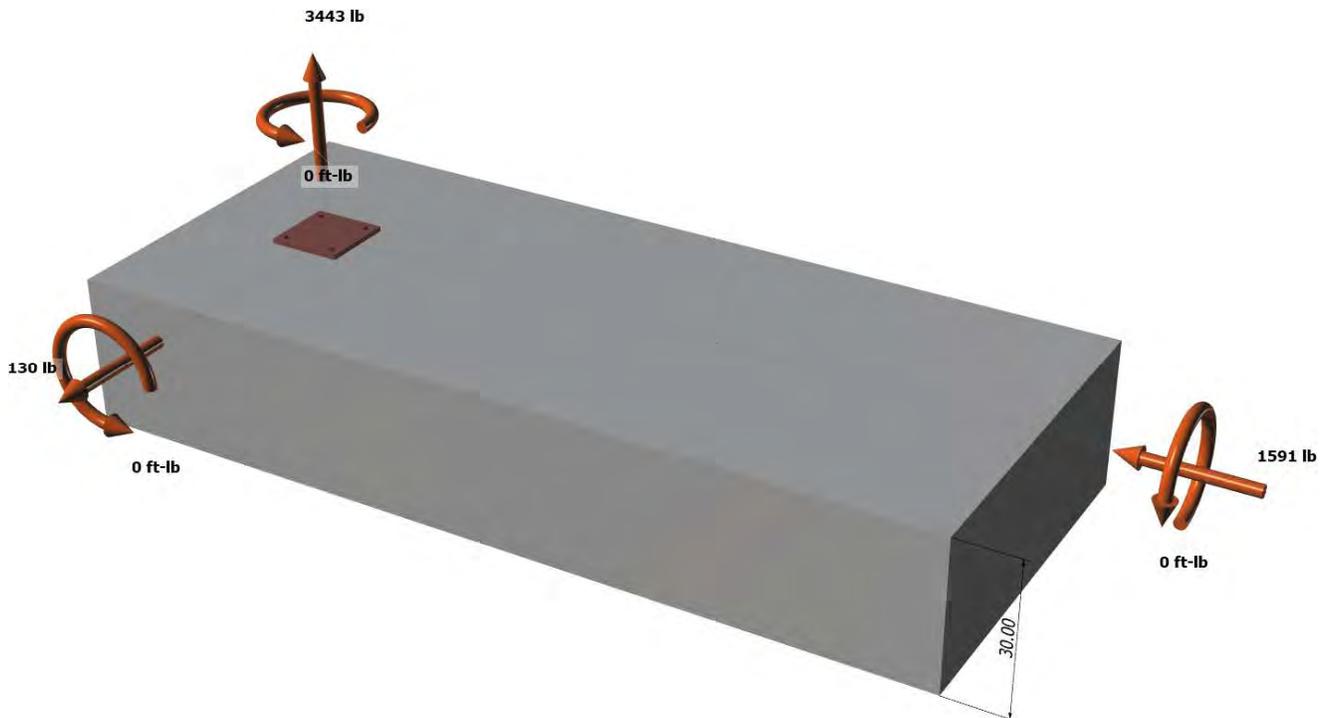
V_{uay} [lb]: -1591

M_{ux} [ft-lb]: 0

M_{uy} [ft-lb]: 0

M_{uz} [ft-lb]: 0

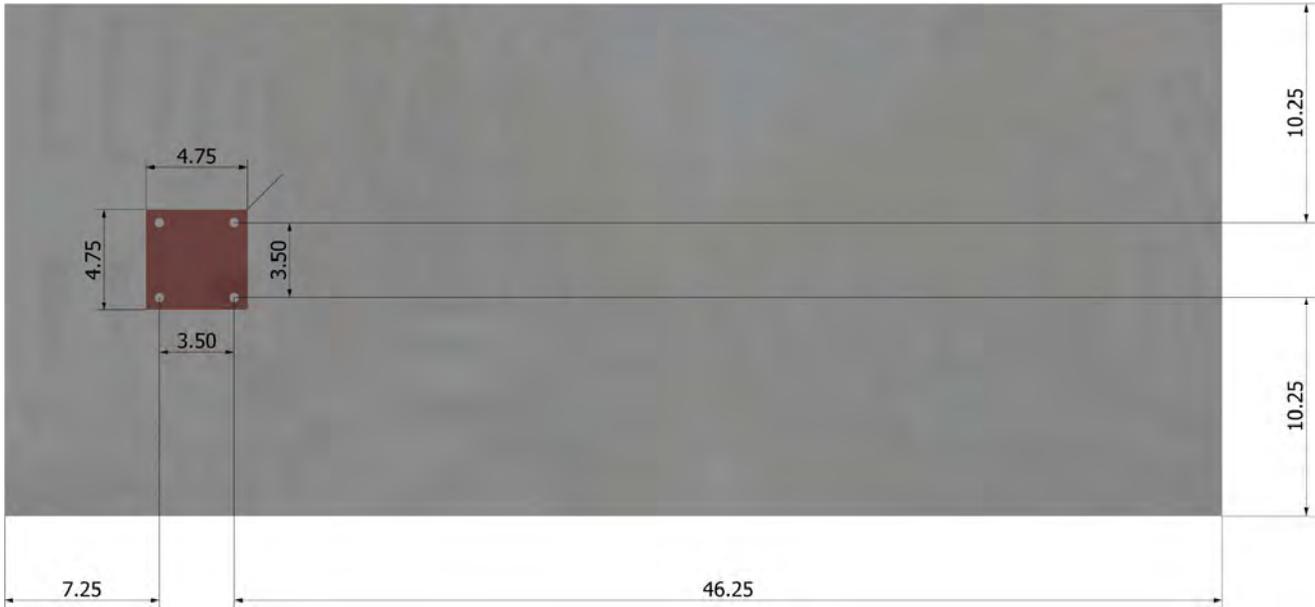
<Figure 1>





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<Figure 2>





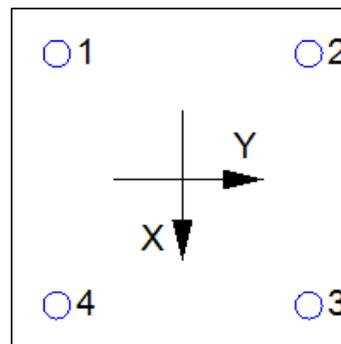
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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	860.8	32.5	-397.8	399.1
2	860.8	32.5	-397.8	399.1
3	860.8	32.5	-397.8	399.1
4	860.8	32.5	-397.8	399.1
Sum	3443.0	130.0	-1591.0	1596.3

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 3443
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. 17.4.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
4525	0.75	3394

5. Concrete Breakout Strength of Anchor in Tension (Sec. 17.4.2)

$$N_b = K_c \lambda_a \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. 17.4.2.2a)}$$

K _c	λ _a	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	4.000	6800

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. 17.3.1 \& Eq. 17.4.2.1b)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	c _{a,min} (in)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
240.25	144.00	7.25	1.000	1.000	1.00	1.000	6800	0.65	7374

6. Adhesive Strength of Anchor in Tension (Sec. 17.4.5)

$$\tau_{k,cr} = \tau_{k,cr,short-term} K_{sat} (f_c / 2,500)^n$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	f _c (psi)	n	τ _{k,cr} (psi)
1346	1.00	1.00	2500	0.24	1346

$$N_{ba} = \lambda_a \tau_{cr} \pi d_a h_{ef} \text{ (Eq. 17.4.5.2)}$$

λ _a	τ _{cr} (psi)	d _a (in)	h _{ef} (in)	N _{ba} (lb)
1.00	1346	0.38	4.000	6343

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ec,Na} \psi_{ed,Na} \psi_{cp,Na} N_{ba} \text{ (Sec. 17.3.1 \& Eq. 17.4.5.1b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	c _{Na} (in)	c _{a,min} (in)	ψ _{ec,Na}	ψ _{ed,Na}	ψ _{cp,Na}	N _{ba} (lb)	φ	φN _{ag} (lb)
198.45	112.09	5.29	7.25	1.000	1.000	1.000	6343	0.55	6176



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8. Steel Strength of Anchor in Shear (Sec. 17.5.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
2715	1.0	0.65	1765

9. Concrete Breakout Strength of Anchor in Shear (Sec. 17.5.2)

Shear perpendicular to edge in x-direction:

$V_{bx} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$ (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

l_e (in)	d_a (in)	λ_a	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
3.00	0.375	1.00	2500	13.75	16564

$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \Psi_{ec,v} \Psi_{ed,v} \Psi_{c,v} \Psi_{h,v} V_{bx}$ (Sec. 17.3.1 & Eq. 17.5.2.1b)

A_{Vc} (in ²)	A_{Vco} (in ²)	$\Psi_{ec,v}$	$\Psi_{ed,v}$	$\Psi_{c,v}$	$\Psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
647.11	850.78	1.000	0.805	1.000	1.000	16564	0.70	7103

Shear perpendicular to edge in y-direction:

$V_{by} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$ (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

l_e (in)	d_a (in)	λ_a	f_c (psi)	c_{a1} (in)	V_{by} (lb)
3.00	0.375	1.00	2500	10.75	11450

$\phi V_{cbgy} = \phi (A_{Vc} / A_{Vco}) \Psi_{ec,v} \Psi_{ed,v} \Psi_{c,v} \Psi_{h,v} V_{by}$ (Sec. 17.3.1 & Eq. 17.5.2.1b)

A_{Vc} (in ²)	A_{Vco} (in ²)	$\Psi_{ec,v}$	$\Psi_{ed,v}$	$\Psi_{c,v}$	$\Psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgy} (lb)
387.00	520.03	1.000	0.891	1.000	1.000	11450	0.70	5313

Shear parallel to edge in x-direction:

$V_{by} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$ (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

l_e (in)	d_a (in)	λ_a	f_c (psi)	c_{a1} (in)	V_{by} (lb)
3.00	0.375	1.00	2500	7.25	6342

$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \Psi_{ec,v} \Psi_{ed,v} \Psi_{c,v} \Psi_{h,v} V_{by}$ (Sec. 17.3.1, 17.5.2.1(c) & Eq. 17.5.2.1b)

A_{Vc} (in ²)	A_{Vco} (in ²)	$\Psi_{ec,v}$	$\Psi_{ed,v}$	$\Psi_{c,v}$	$\Psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
261.00	236.53	1.000	1.000	1.000	1.000	6342	0.70	9797

Shear parallel to edge in y-direction:

$V_{bx} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$ (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

l_e (in)	d_a (in)	λ_a	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
3.00	0.375	1.00	2500	10.25	10661

$\phi V_{cbgy} = \phi (2)(A_{Vc} / A_{Vco}) \Psi_{ec,v} \Psi_{ed,v} \Psi_{c,v} \Psi_{h,v} V_{bx}$ (Sec. 17.3.1, 17.5.2.1(c) & Eq. 17.5.2.1b)

A_{Vc} (in ²)	A_{Vco} (in ²)	$\Psi_{ec,v}$	$\Psi_{ed,v}$	$\Psi_{c,v}$	$\Psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbgy} (lb)
401.67	472.78	1.000	1.000	1.000	1.000	10661	0.70	12680

10. Concrete Pryout Strength of Anchor in Shear (Sec. 17.5.3)

$\phi V_{cp} = \phi \min|k_{cp} N_{ba}; k_{cp} N_{cb}| = \phi \min|k_{cp} (A_{Na} / A_{Na0}) \Psi_{ec,Na} \Psi_{ed,Na} \Psi_{cp,Na} N_{ba}; k_{cp} (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{cp,N} N_{cb}|$ (Sec. 17.3.1 & Eq. 17.5.3.1b)

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\Psi_{ed,Na}$	$\Psi_{ec,Na}$	$\Psi_{cp,Na}$	N_{ba} (lb)	N_a (lb)
2.0	198.45	112.09	1.000	1.000	1.000	6343	11230

A_{Nc} (in ²)	A_{Nco} (in ²)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
240.25	144.00	1.000	1.000	1.000	1.000	6800	11345	0.70

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Company:		Date:	5/14/2018
Engineer:		Page:	6/6
Project:			
Address:			
Phone:			
E-mail:			

ϕV_{cpq} (lb)
15722

11. Results

Interaction of Tensile and Shear Forces (Sec. 17.6.)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	861	3394	0.25	Pass	
Concrete breakout	3443	7374	0.47	Pass	
Adhesive	3443	6176	0.56	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	399	1765	0.23	Pass	
T Concrete breakout x+	130	7103	0.02	Pass	
T Concrete breakout y-	1591	5313	0.30	Pass	
Concrete breakout y-	65	9797	0.01	Pass	
Concrete breakout x-	796	12680	0.06	Pass	
Concrete breakout, combined	-	-	0.30	Pass (Governs)	
Pryout	1596	15722	0.10	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. 17.6..1	0.56	0.00	55.7%	1.0	Pass

SET-3G w/ 3/8"Ø F1554 Gr. 36 with hef = 4.000 inch meets the selected design criteria.

12. Warnings

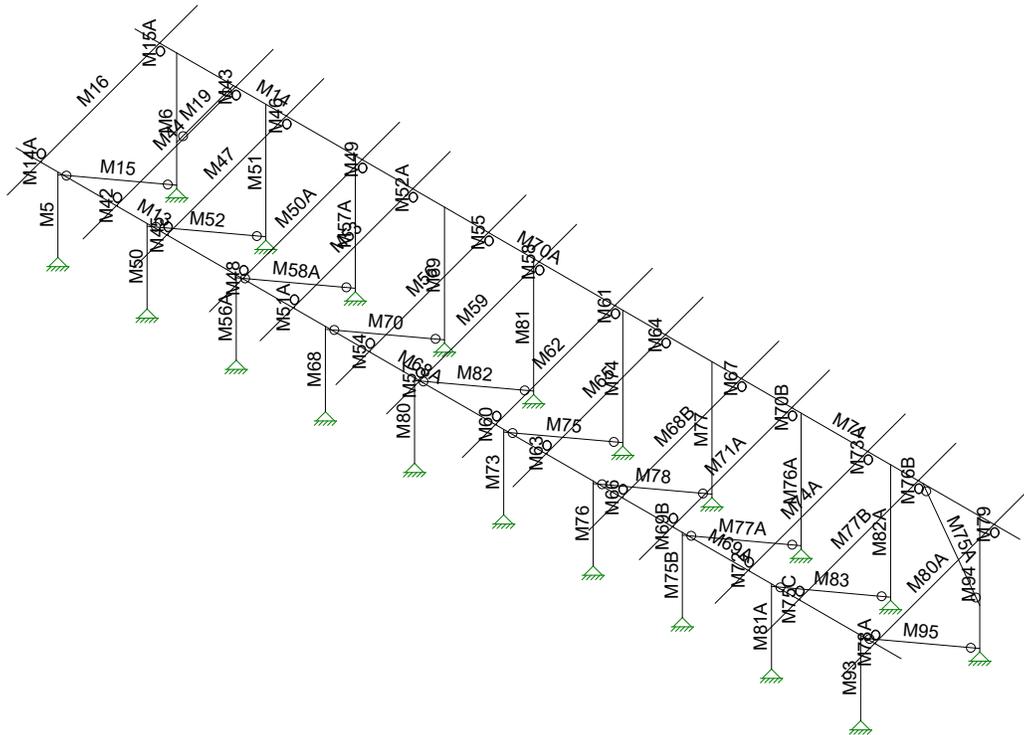
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



JOB NO.: U2716-103-191

DESIGNED: STB

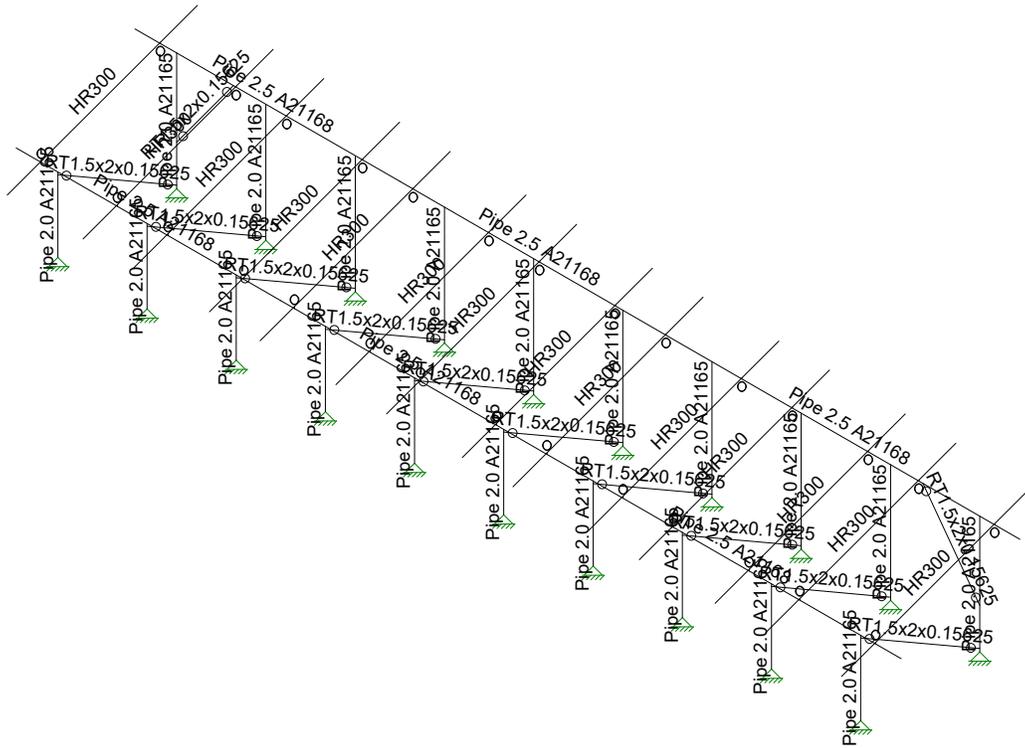
Framing Analysis



Vector Structural Engineeri...
STB
U2716.103.191

Ground Mount

SK - 3
Apr 6, 2021 at 3:35 PM
Sunmodo Sunturf D1 GM v7 85x45...



Vector Structural Engineeri...

STB

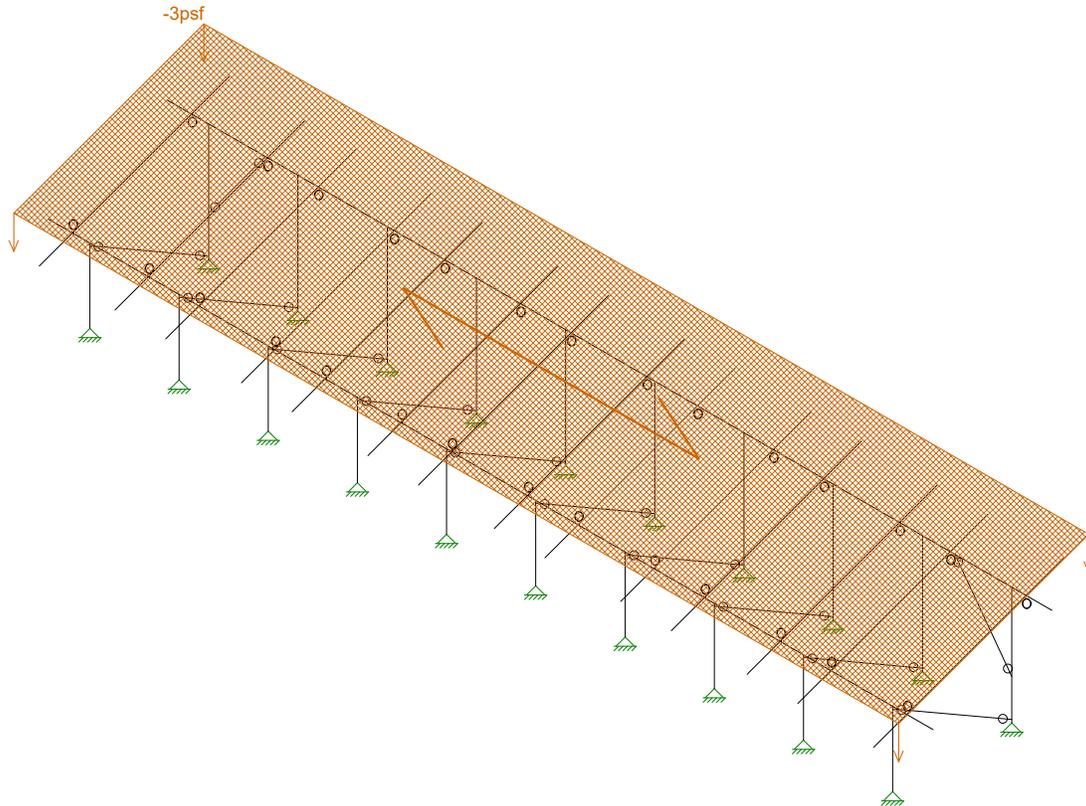
U2716.103.191

Ground Mount

SK - 4

Apr 6, 2021 at 3:35 PM

Sunmodo Sunturf D1 GM v7 85x45...



Loads: BLC 2, Solar Panel Weight

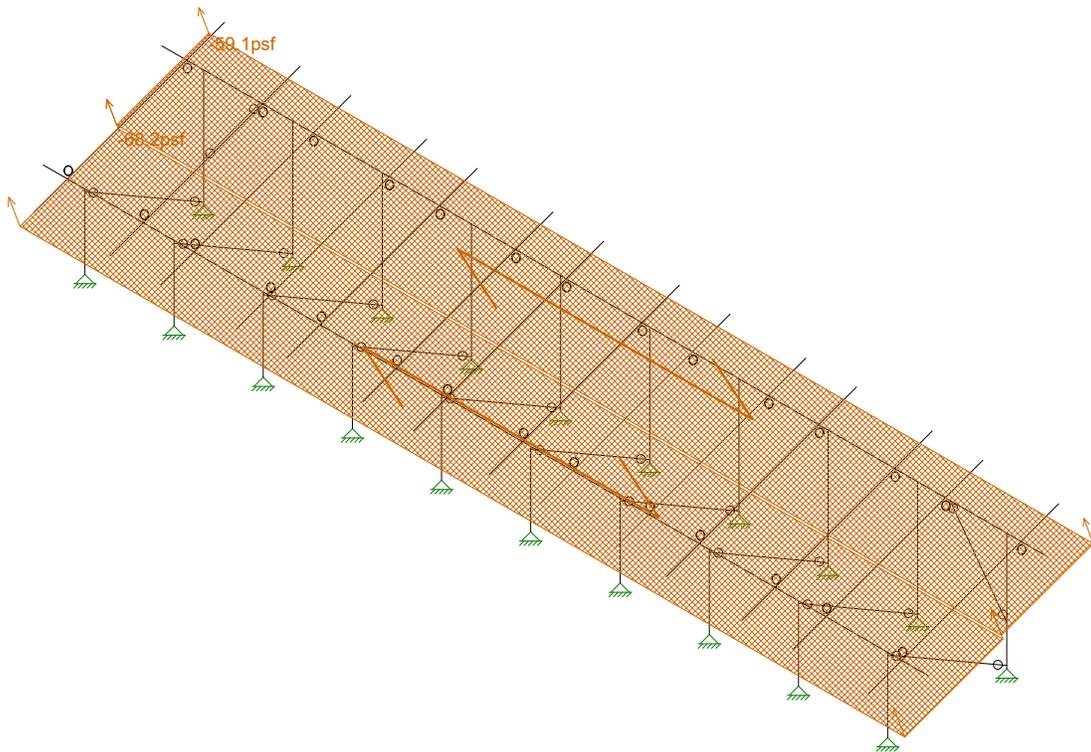
Vector Structural Engineeri...
STB
U2716.103.191

Ground Mount

SK - 5

Apr 6, 2021 at 3:36 PM

Sunmodo Sunturf D1 GM v7 85x45...



Loads: BLC 4, Wind A 0 deg

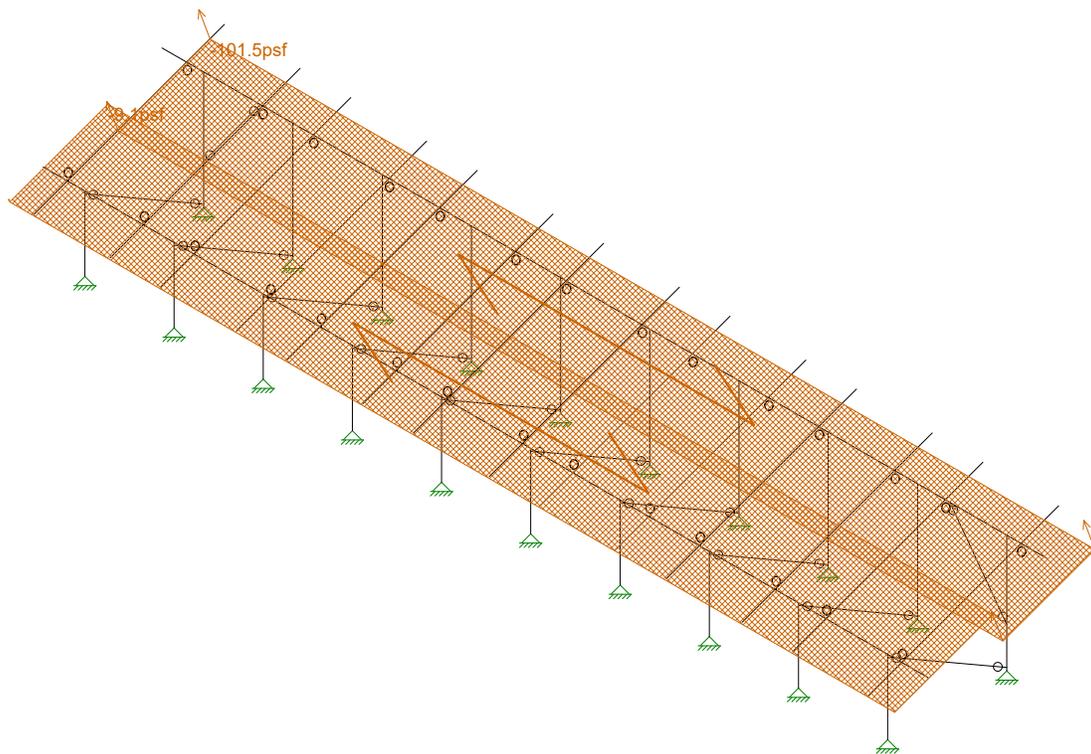
Vector Structural Engineeri...
STB
U2716.103.191

Ground Mount

SK - 6

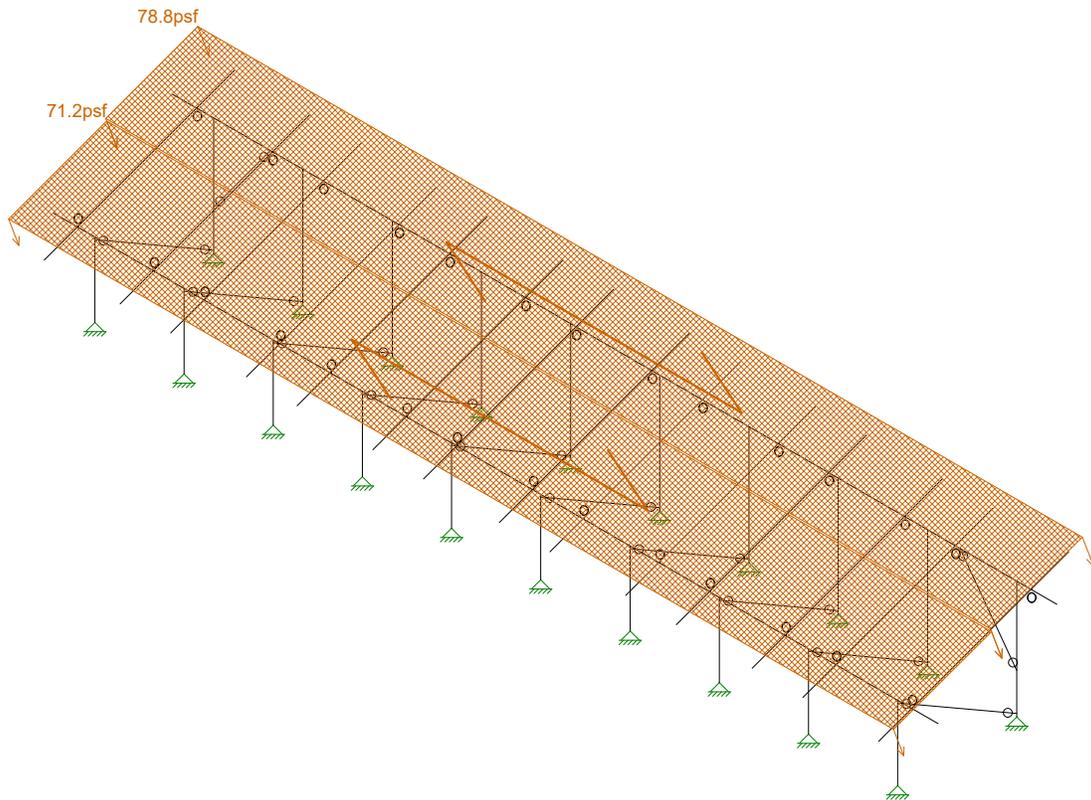
Apr 6, 2021 at 3:36 PM

Sunmodo Sunturf D1 GM v7 85x45...



Loads: BLC 5, Wind B 0 deg

Vector Structural Engineeri...	Ground Mount	SK - 7
STB		Apr 6, 2021 at 3:36 PM
U2716.103.191		Sunmodo Sunturf D1 GM v7 85x45...



Loads: BLC 6, Wind A 180 deg

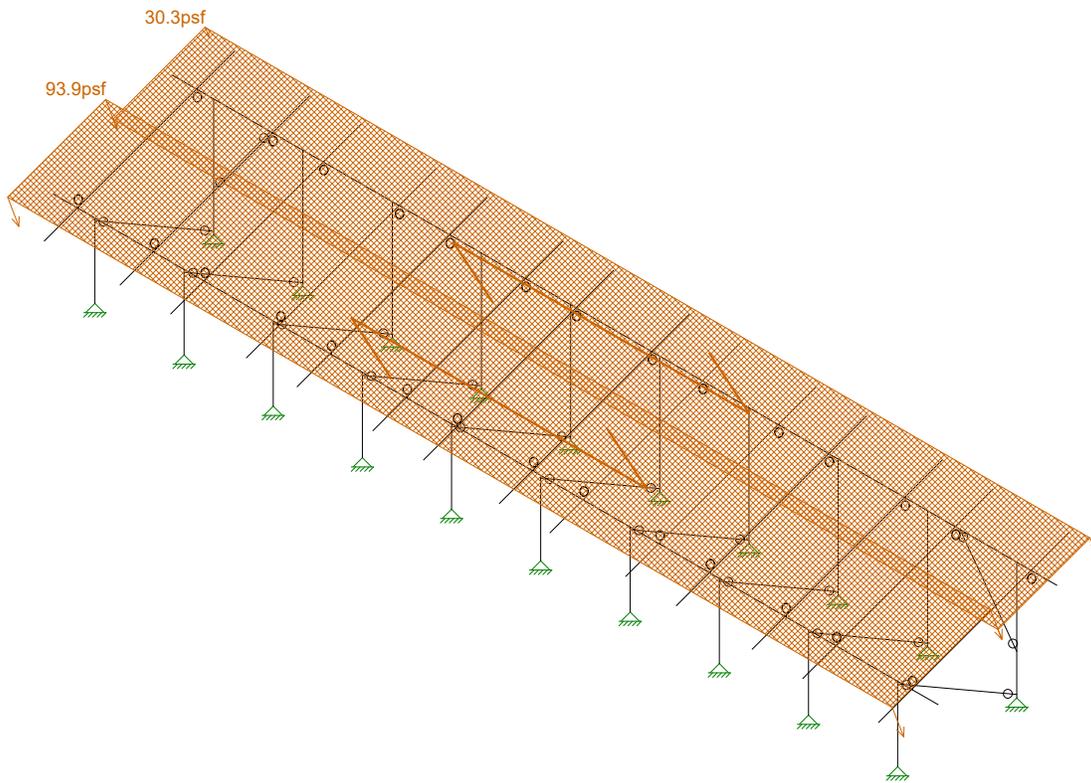
Vector Structural Engineeri...
STB
U2716.103.191

Ground Mount

SK - 8

Apr 6, 2021 at 3:36 PM

Sunmodo Sunturf D1 GM v7 85x45...

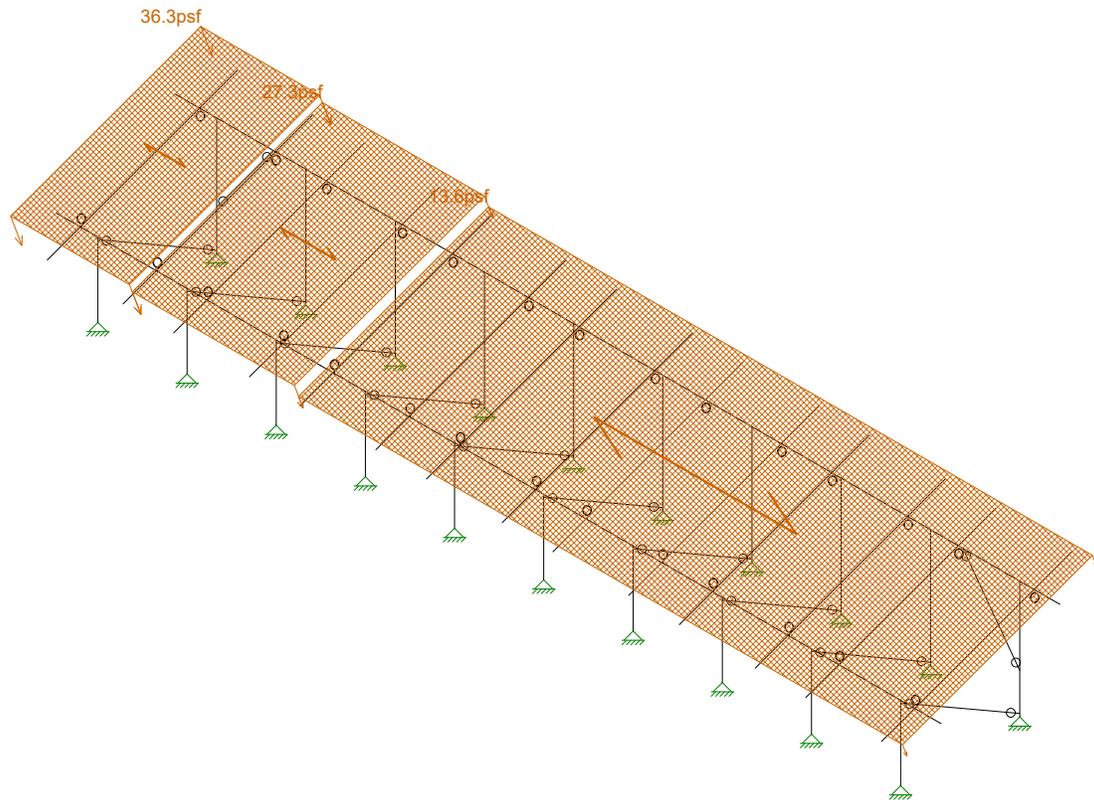


Loads: BLC 7, Wind B 180 deg

Vector Structural Engineeri...
STB
U2716.103.191

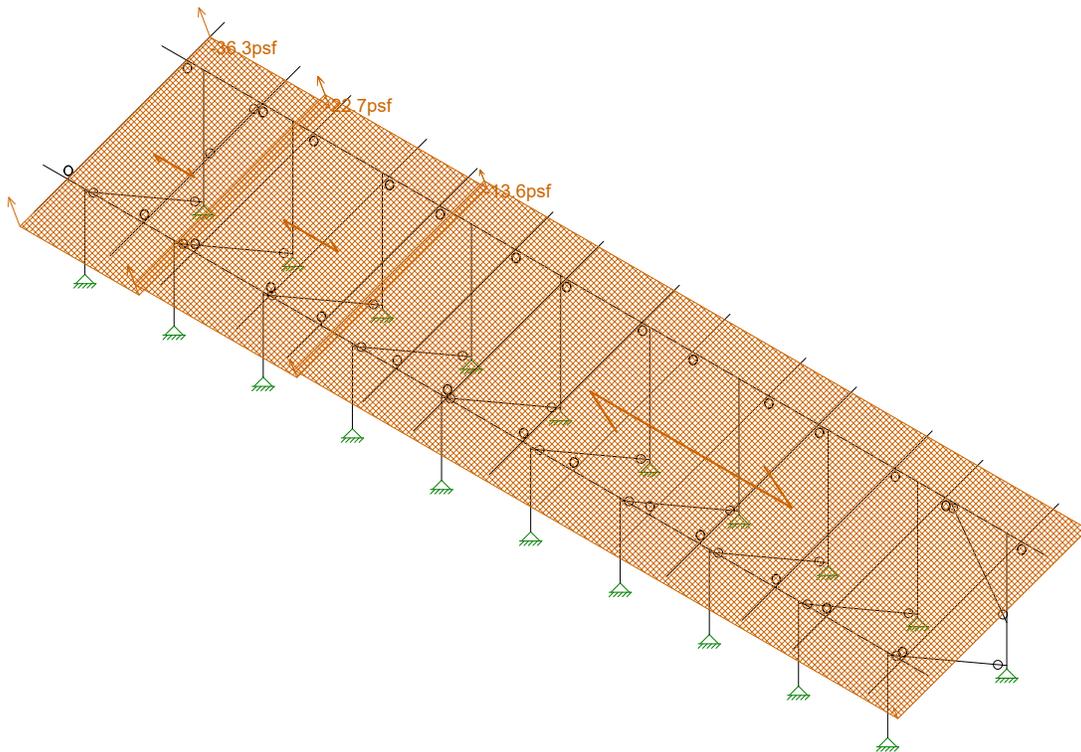
Ground Mount

SK - 9
Apr 6, 2021 at 3:36 PM
Sunmodo Sunturf D1 GM v7 85x45...



Loads: BLC 8, Wind A 90

Vector Structural Engineeri..	Ground Mount	SK - 10
STB		Apr 6, 2021 at 3:36 PM
U2716.103.191		Sunmodo Sunturf D1 GM v7 85x45...



Loads: BLC 9, Wind B 90

Vector Structural Engineeri...

STB

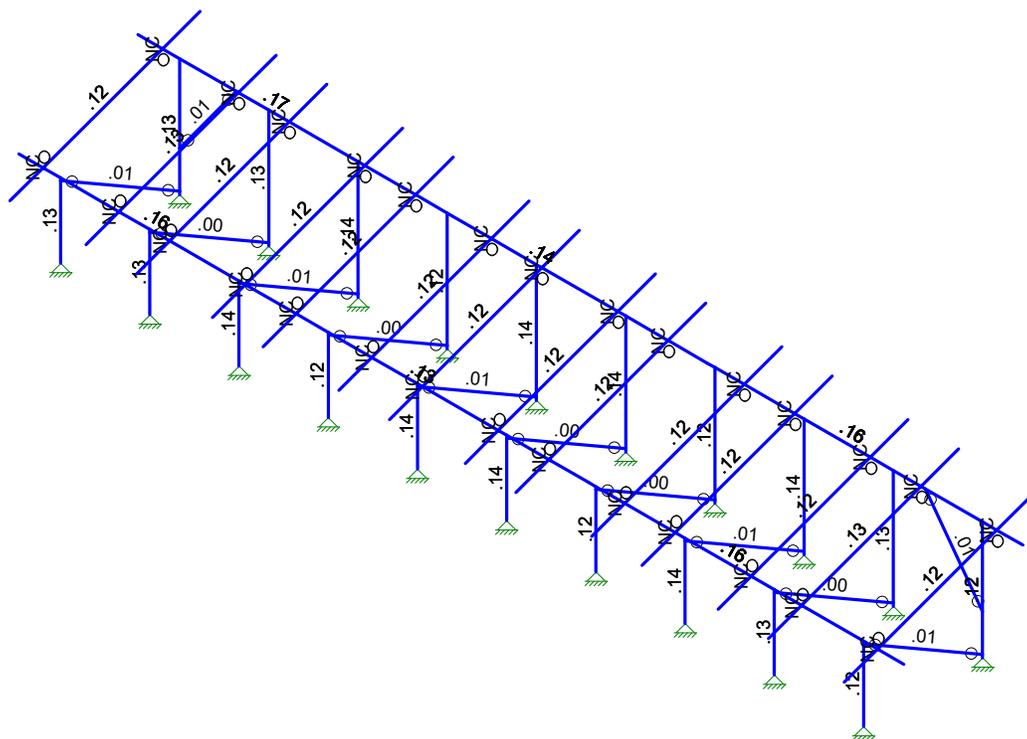
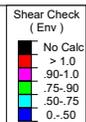
U2716.103.191

Ground Mount

SK - 11

Apr 6, 2021 at 3:36 PM

Sunmodo Sunturf D1 GM v7 85x45...



Member Shear Checks Displayed (Enveloped)
Envelope Only Solution

Vector Structural Engineeri...	Ground Mount	SK - 2
STB		Apr 6, 2021 at 3:35 PM
U2716.103.191		Sunmodo Sunturf D1 GM v7 85x45...



(Global) Model Settings

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include Warping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P-Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (in/sec^2)	386.4
Wall Mesh Size (in)	24
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Y
Global Member Orientation Plane	XZ
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver

Hot Rolled Steel Code	AISC 14th(360-10): ASD
Adjust Stiffness?	Yes(Iterative)
RISACONNECTION CODE	AISC 14th(360-10): ASD
Cold Formed Steel Code	AISI S100-16: ASD
Wood Code	AWC NDS-15: ASD
Wood Temperature	< 100F
Concrete Code	ACI 318-14
Masonry Code	ACI 530-13: ASD
Aluminum Code	AA ADM1-15: ASD - Building
Stainless Steel Code	AISC 14th(360-10): ASD
Adjust Stiffness?	Yes(Iterative)

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	Exact Integration
Parame Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections?	Yes
Use Cracked Sections Slab?	No
Bad Framing Warnings?	No
Unused Force Warnings?	Yes
Min 1 Bar Diam. Spacing?	No
Concrete Rebar Set	REBAR SET ASTMA615
Min % Steel for Column	1
Max % Steel for Column	8



Company : Vector Structural Engineering
 Designer : STB
 Job Number : U2716.103.191
 Model Name : Ground Mount

Apr 6, 2021
 3:36 PM
 Checked By: _____

Aluminum Section Sets

	Label	Shape	Type	Design List	Material	Design R...	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	AL Posts	2.375ODX0.188	Column	Pipe	6005-T5	Typical	1.29	.778	.778	1.54
2	AL Brace	RT1.5x2x0.15625	VBrace	Rectangular Tubes	6005-T5	Typical	.996	.327	.524	.602
3	AL Rails	HR300	Beam	Rectangular Tubes	6005-T5	Typical	.74	.253	.727	.578
4	AL Cross Beam	Cross Rail	Beam	Rectangular Tubes	6005-T5	Typical	1.909	1.97	4.366	4.017

Member Area Loads (BLC 2 : Solar Panel Weight)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N199	N196	Y	A-B	-3

Member Area Loads (BLC 3 : Roof Live/Snow)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N199	N196	PY	A-B	0

Member Area Loads (BLC 4 : Wind A 0 deg)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	-59.1
2	N198	N201	N199	N196	Perp	A-B	-68.2

Member Area Loads (BLC 5 : Wind B 0 deg)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	-101.5
2	N198	N201	N199	N196	Perp	A-B	-9.1

Member Area Loads (BLC 6 : Wind A 180 deg)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	78.8
2	N198	N201	N199	N196	Perp	A-B	71.2

Member Area Loads (BLC 7 : Wind B 180 deg)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	30.3
2	N198	N201	N199	N196	Perp	A-B	93.9

Member Area Loads (BLC 8 : Wind A 90)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N203	N202	N196	Perp	A-B	36.3
2	N203	N209	N208	N202	Perp	A-B	27.3
3	N209	N200	N199	N208	Perp	A-B	13.6

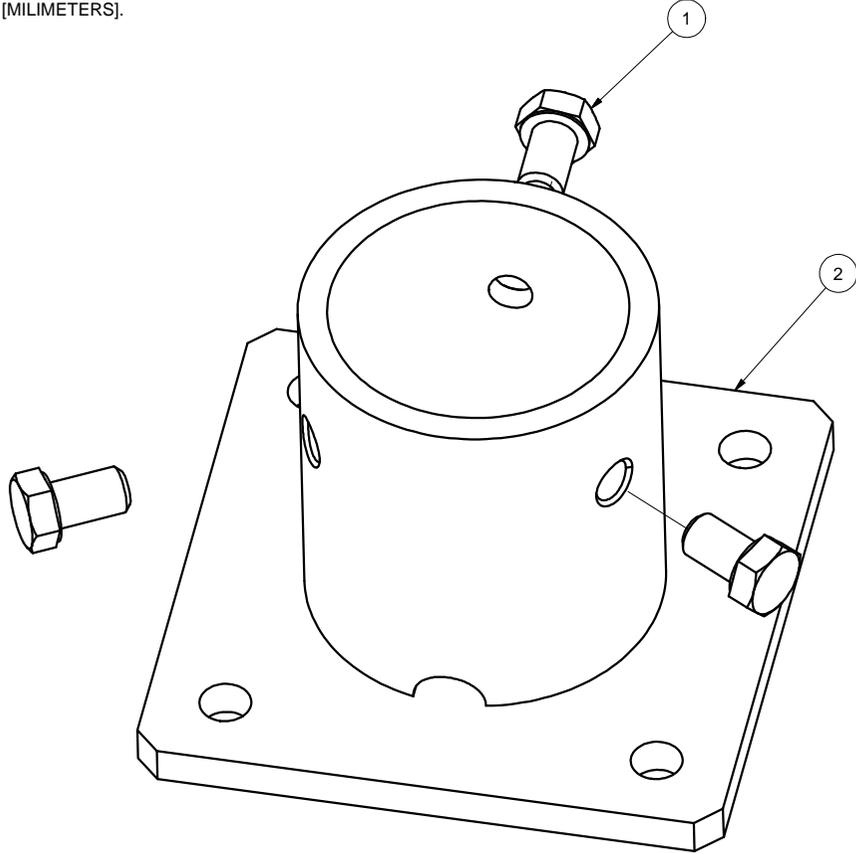
Member Area Loads (BLC 9 : Wind B 90)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N203	N202	N196	Perp	A-B	-36.3
2	N203	N209	N208	N202	Perp	A-B	-22.7
3	N209	N200	N199	N208	Perp	A-B	-13.6

Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed	Area(M...Surface...
1	Self Weight	DL		-1.05					
2	Solar Panel Weight	DL							1

NOTES: UNLESS OTHERWISE SPECIFIED
 1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].



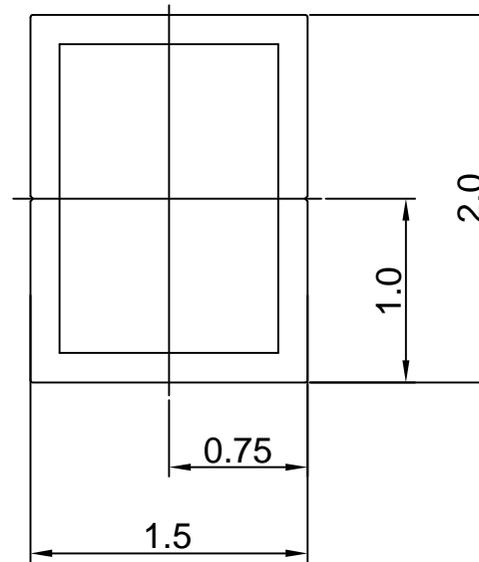
REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	10/20/2016

2	A21120-001	2" PIPE BASE	1
1	B15018-011	HEX CAP SCREW 3/8-16 X 5/8	3
ITEM	PART NUMBER	DESCRIPTION	QTY
MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		Sunmodo Corp. 1905 E 5TH STREET, STE A, VANCOUVER, WA 98661	
Tolerances		TITLE	
X.XXX ±0.01 [0.25mm]		2" PIPE BASE KIT	
X.XX ±0.02 [0.50mm]		DRAWING NUMBER	
X.X ±0.039 [1.0mm]		B K10268-001	
Unless otherwise spec'd		SCALE: NONE	
DRAWN BY		SHEET 1 of 1	
LWF		DATE	
CHECKED BY		10/20/2016	
APPROVALS			

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NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERAIL: ALUMINUM 6005-T5.
FINISH: CLEAR ANODIZED 10 μ m THICK.
3. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.



Section properties:

Weight: 1.156 lbs/ft

Area: 0.992 in²

Perimeter: 12.601 in

Bounding Box: X: -1.000,1.000

Y: -0.750, 0.750

Centroid:(0.000,0.000)

Moments of Inertia(in⁴): I_x=0.506,I_y=0.322

Section modulus in bending(in³): W_x=0.675,W_y=0.322

Radii of Gyration: X: 0.714, Y: 0.570

MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS			
All Dimensions in inches [millimeters]			
Tolerances			
X.XXX ± 0.01 [0.25mm]		Break all sharp edges	
X.XX ± 0.02 [0.50mm]		.010-.020 unless	
X.X ± 0.039 [1.0mm]		otherwise specified.	
Unless otherwise specd			
DRAWN BY	DATE	TITLE	
zcg	03/12/2014	1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661	
CHECKED BY		B	DRAWING NUMBER
			A20164
APPROVALS		SCALE:	SHEET 1 of 1
		NONE	

Sunmodo Corp.

1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661

TITLE
1.5X2 AL TUBE BRACE EXTRUSION

DRAWING NUMBER
A20164

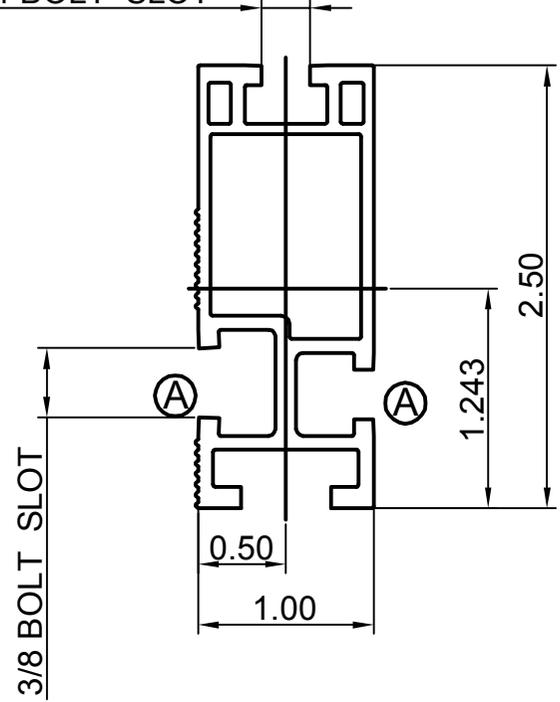
SCALE: NONE SHEET 1 of 1

REVISIONS			
REV	DESCRIPTION	BY	DATE
A	ADDED BOTTOM CHANNEL & CHANGED ONE 3/8 CHANNEL TO 1/4	zcg	02/21/2013

NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERAIL: ALUMINUM 6005-T5.
FINISH: CLEAR ANODIZED 15 μ m THICK.
3. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.

2X 1/4 BOLT SLOT



Section properties:

Weight: 0.850 lbs/ft
 Area: 0.723 in²
 Perimeter: 17.325 in
 Bounding Box: X: -0.500,0.500
 Y: -1.243,1.257

Centroid:(0.000,0.000)
 Moments of Incertia(in⁴): Ix=0.486,Iy=0.095
 Section modulus in bending(in³): Wx=0.387,Wy=0.190
 Radii of Gyration: X: 0.820, Y: 0.363

MATERIAL SEE NOTES		Sunmodo Corp.	
Third Angle Projection:			
GENERAL SPECIFICATIONS All Dimensions in inches [millimeters]		1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661	
Tolerances: X.XXX ± 0.01 [0.25mm] X.XX ± 0.02 [0.50mm] X.X ± 0.039 [1.0mm] Unless otherwise spec'd		TITLE HELIO STANDARD RAIL	
DRAWN BY zcg	DATE 02/21/2013	DRAWING NUMBER A20144	
CHECKED BY		SCALE: NONE SHEET 1 of 1	
APPROVALS			

4

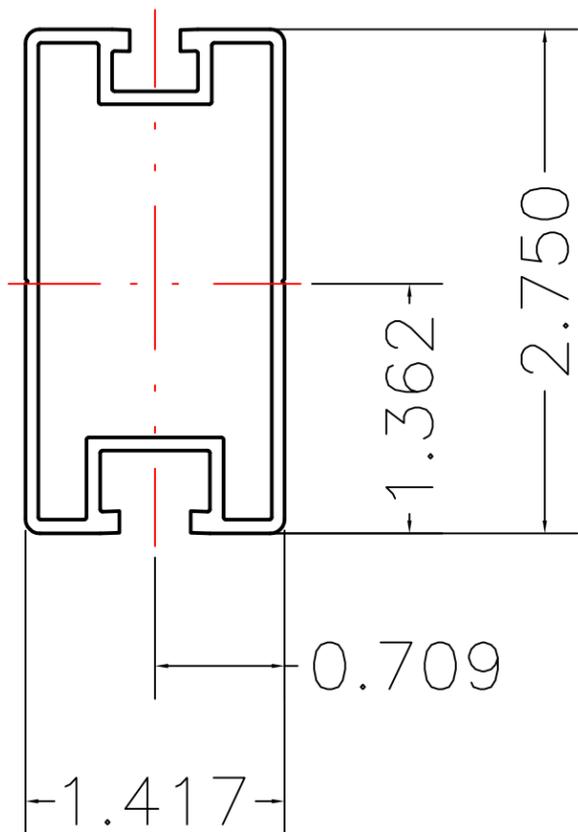
3

2

1

NOTES: UNLESS OTHERWISE SPECIFIED

- 1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
- 2. MATERIAL: 6005-T5.
FINISH: CLEAR ANODIZED 10um THICK.
- 3. THE UNSPECIFIED RADII ARE .02" MAX.
- 4. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.



Section properties:

Weight: 0.862 lbs/ft
 Area: 0.736 in²
 Perimeter: 19.824 in
 Bounding Box: X: -0.709,0.709
 Y: -1.362,1.388
 Centroid:(0.000,0.000)
 Moments of Inertia(in⁴): I_x=0.727,I_y=0.214
 Section modulus in bending(in³): W_x=0.524,W_y=0.302
 Radii of Gyration: X: 994, Y: 0.539

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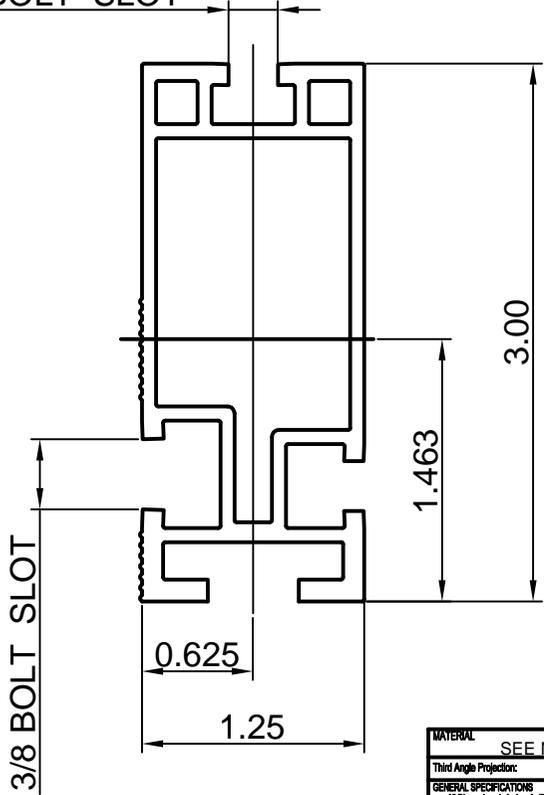
MATERIAL SEE NOTES		SunModo Corp. 14800 NE 65TH STREET, VANCOUVER WA 98682	
Third Angle Projection:			
GENERAL SPECIFICATIONS All Dimensions in inches [millimeters]		TITLE RAIL, HR300 (SUNRAY), EXTRUSION	
Tolerances X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] Break all sharp edges X.X ±0.039 [1.0mm] .010-.020 unless otherwise specified. Unless otherwise spec'd			
DRAWN BY KYY	DATE 01/18/2018	B	DRAWING NUMBER A20288
CHECKED BY			
APPROVALS		SCALE: NONE	SHEET 1 of 1

REV	DESCRIPTON	BY	DATE
A	0.44 WAS 0.41, 0.44 WAS 0.33	LWF	11/30/2015

NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERIAL: ALUMINUM 6005-T5.
FINISH: CLEAR ANODIZED 15 μm THICK.
3. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.

2X 1/4 BOLT SLOT



Section properties:

Weight: 1.151 lbs/ft
 Area: 0.980 in²
 Perimeter: 22.104 in
 Bounding Box: X: -0.625,0.625
 Y: -1.463,1.537
 Centroid:(0.000,0.000)
 Moments of Inertia(in⁴): Ix=1.047,Iy=0.207
 Section modulus in bending(in³): Wx=0.681,Wy=0.331
 Radii of Gyration: X: 1.034, Y: 0.460

MATERIAL		SEE NOTES	
Third Angle Projection			
GENERAL SPECIFICATIONS			
All Dimensions in Inches (millimeters)			
Tolerances			
XXX ±0.01 (0.25mm)			
XX ±0.02 (0.50mm)			
X ±0.03 (1.0mm)			
Unless otherwise specified.			
DRAWN BY		DATE	
ZCJ		02/21/2013	
CHECKED BY		DATE	
APPROVALS		DATE	
		Break all sharp edges 0.10-0.25 unless otherwise specified.	
		TITLE	
		HELIO HEAVY RAIL	
		DRAWING NUMBER	
		A20145	
		SCALE: NONE	
		SHEET 1 of 1	

Sunmodo Corp.

1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661

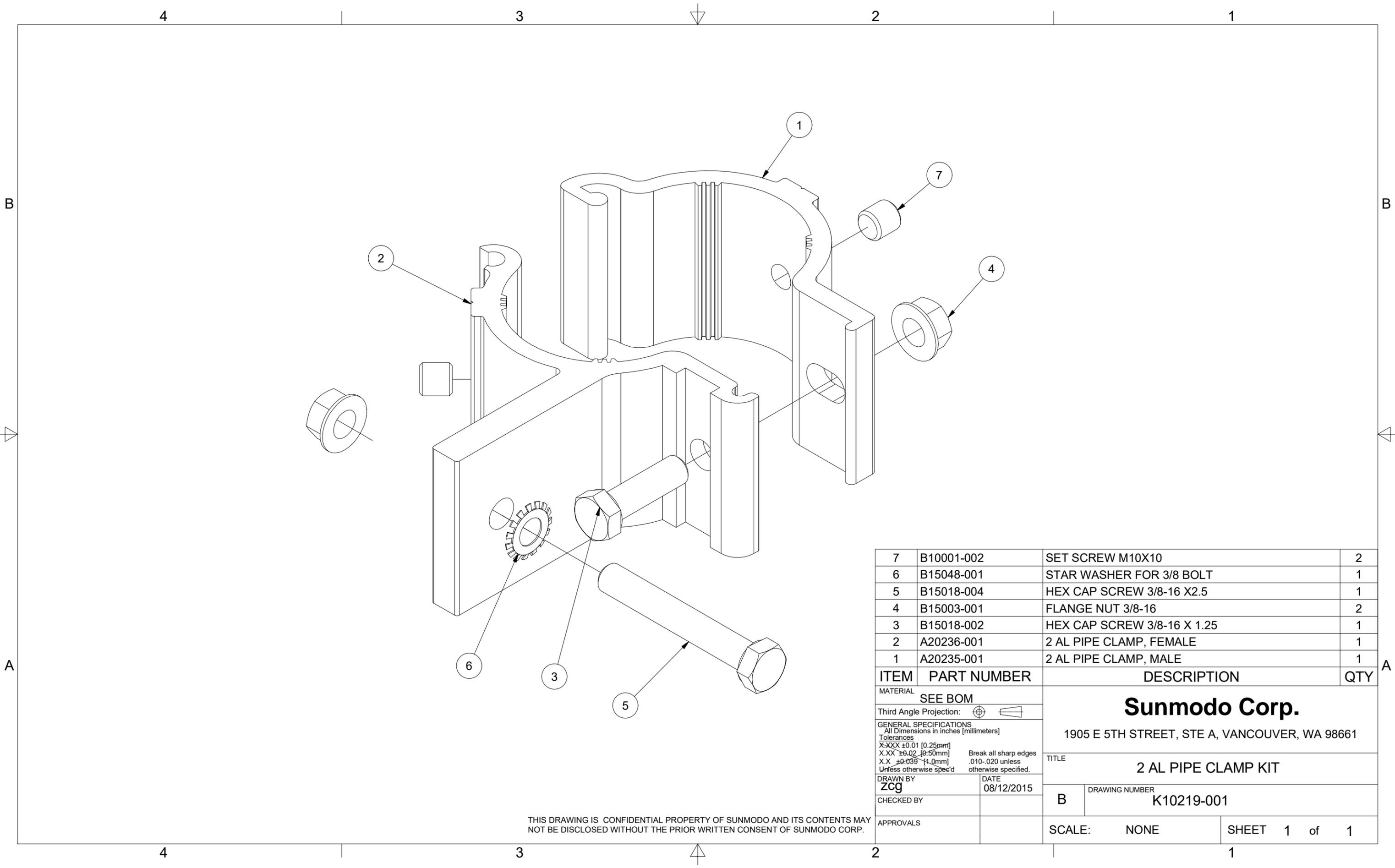
HELIO HEAVY RAIL

DRAWING NUMBER

A20145

SCALE: NONE

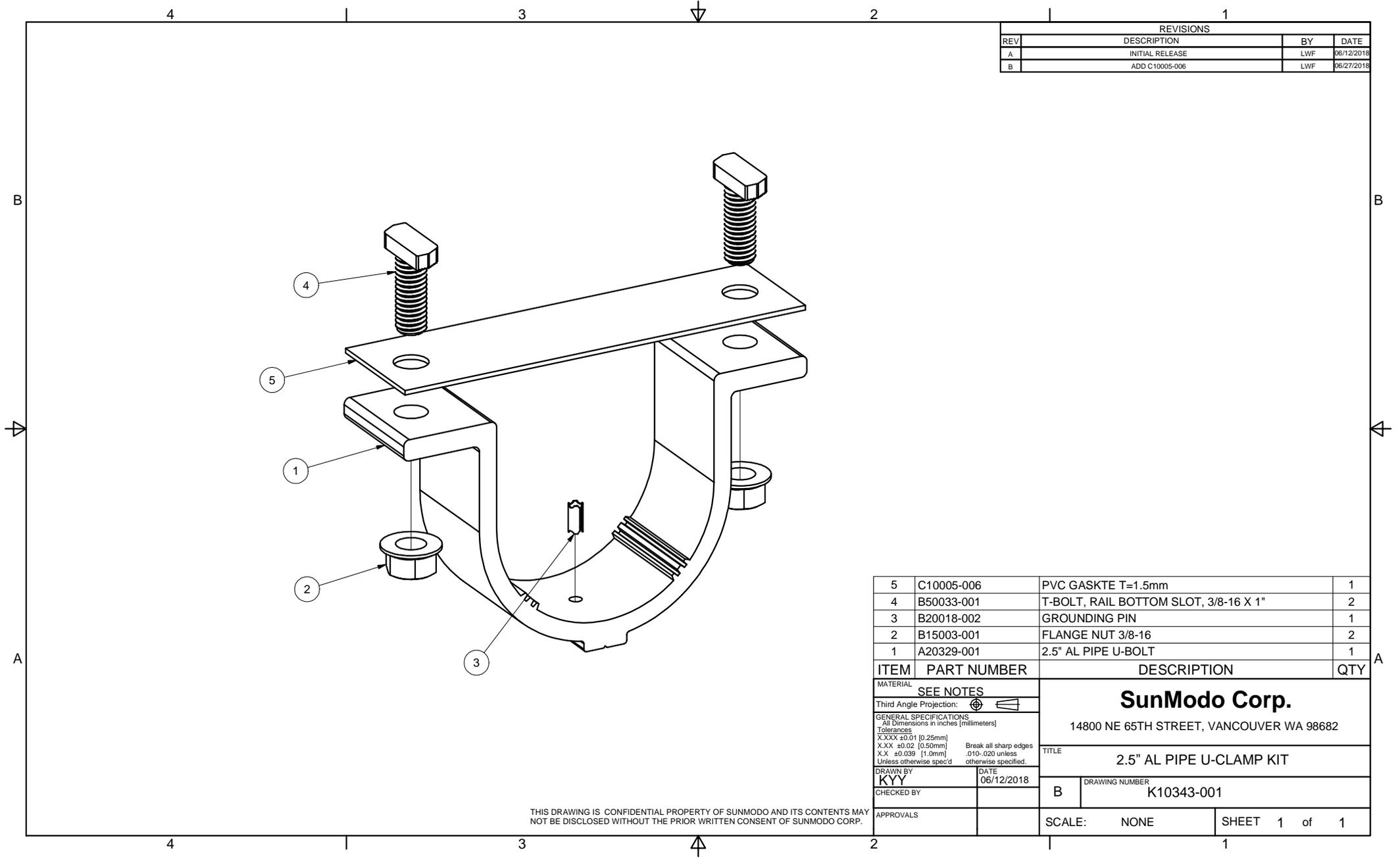
SHEET 1 of 1



7	B10001-002	SET SCREW M10X10	2
6	B15048-001	STAR WASHER FOR 3/8 BOLT	1
5	B15018-004	HEX CAP SCREW 3/8-16 X2.5	1
4	B15003-001	FLANGE NUT 3/8-16	2
3	B15018-002	HEX CAP SCREW 3/8-16 X 1.25	1
2	A20236-001	2 AL PIPE CLAMP, FEMALE	1
1	A20235-001	2 AL PIPE CLAMP, MALE	1

ITEM	PART NUMBER	DESCRIPTION	QTY
MATERIAL		SEE BOM	
Third Angle Projection:			
GENERAL SPECIFICATIONS All Dimensions in inches (millimeters)			
Tolerances			
X.XXX ±0.01 [0.25mm]		Break all sharp edges	
X.XX ±0.02 [0.50mm]		.010-.020 unless	
X.X ±0.039 [1.0mm]		otherwise specified.	
DRAWN BY		DATE	
zcg		08/12/2015	
CHECKED BY		TITLE	
		2 AL PIPE CLAMP KIT	
APPROVALS		DRAWING NUMBER	
		B K10219-001	
SCALE:		NONE	SHEET 1 of 1

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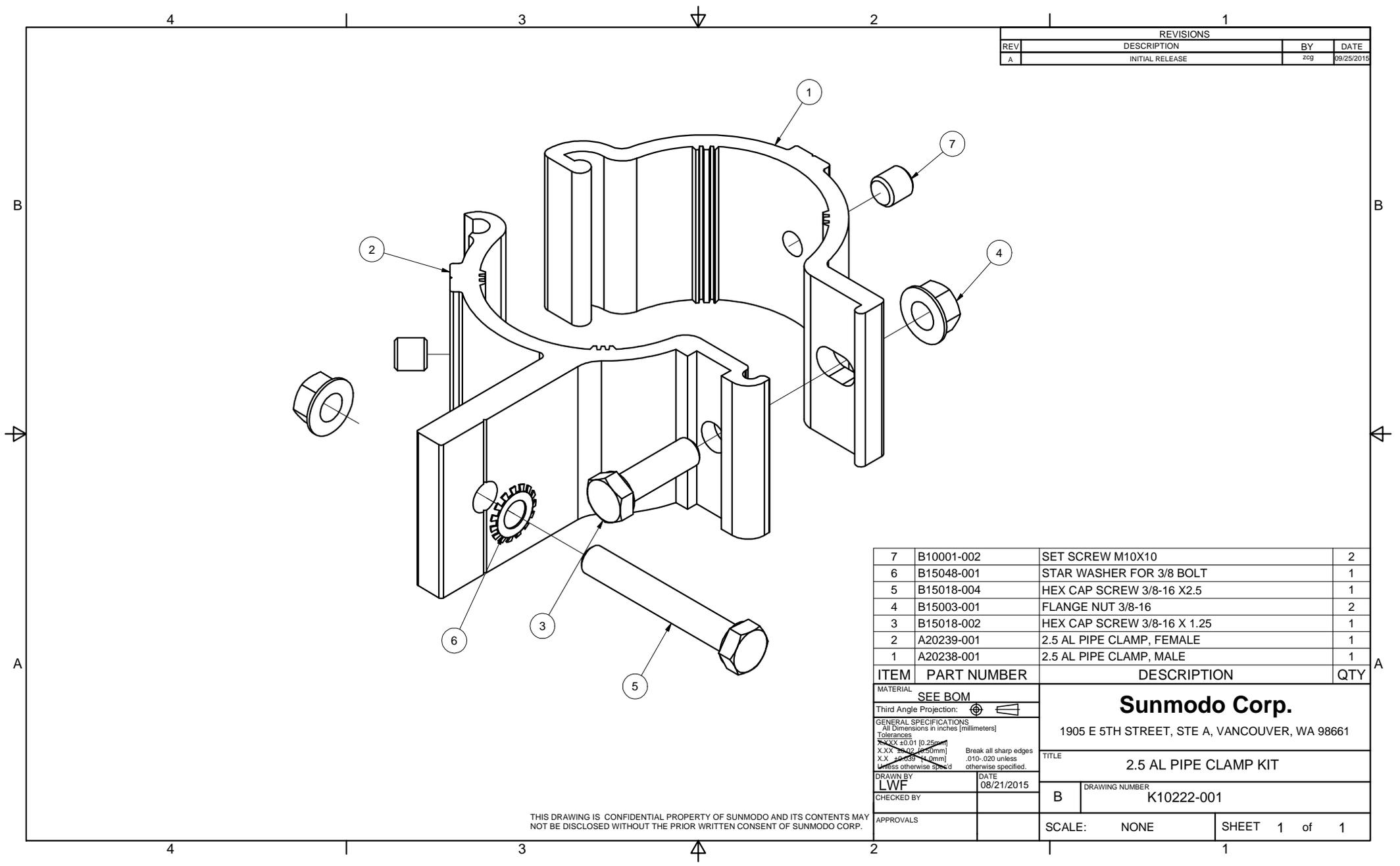


REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	06/12/2018
B	ADD C10005-006	LWF	06/27/2018

ITEM	PART NUMBER	DESCRIPTION	QTY
5	C10005-006	PVC GASKTE T=1.5mm	1
4	B50033-001	T-BOLT, RAIL BOTTOM SLOT, 3/8-16 X 1"	2
3	B20018-002	GROUNDING PIN	1
2	B15003-001	FLANGE NUT 3/8-16	2
1	A20329-001	2.5" AL PIPE U-BOLT	1

MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		SunModo Corp. 14800 NE 65TH STREET, VANCOUVER WA 98682	
Tolerances		TITLE	
X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] X.X ±0.039 [1.0mm] Unless otherwise spec'd		2.5" AL PIPE U-CLAMP KIT	
DRAWN BY	DATE	DRAWING NUMBER	
KYY	06/12/2018	B K10343-001	
CHECKED BY		SCALE: NONE SHEET 1 of 1	
APPROVALS			

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REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	zcg	09/25/2015

ITEM	PART NUMBER	DESCRIPTION	QTY
7	B10001-002	SET SCREW M10X10	2
6	B15048-001	STAR WASHER FOR 3/8 BOLT	1
5	B15018-004	HEX CAP SCREW 3/8-16 X2.5	1
4	B15003-001	FLANGE NUT 3/8-16	2
3	B15018-002	HEX CAP SCREW 3/8-16 X 1.25	1
2	A20239-001	2.5 AL PIPE CLAMP, FEMALE	1
1	A20238-001	2.5 AL PIPE CLAMP, MALE	1

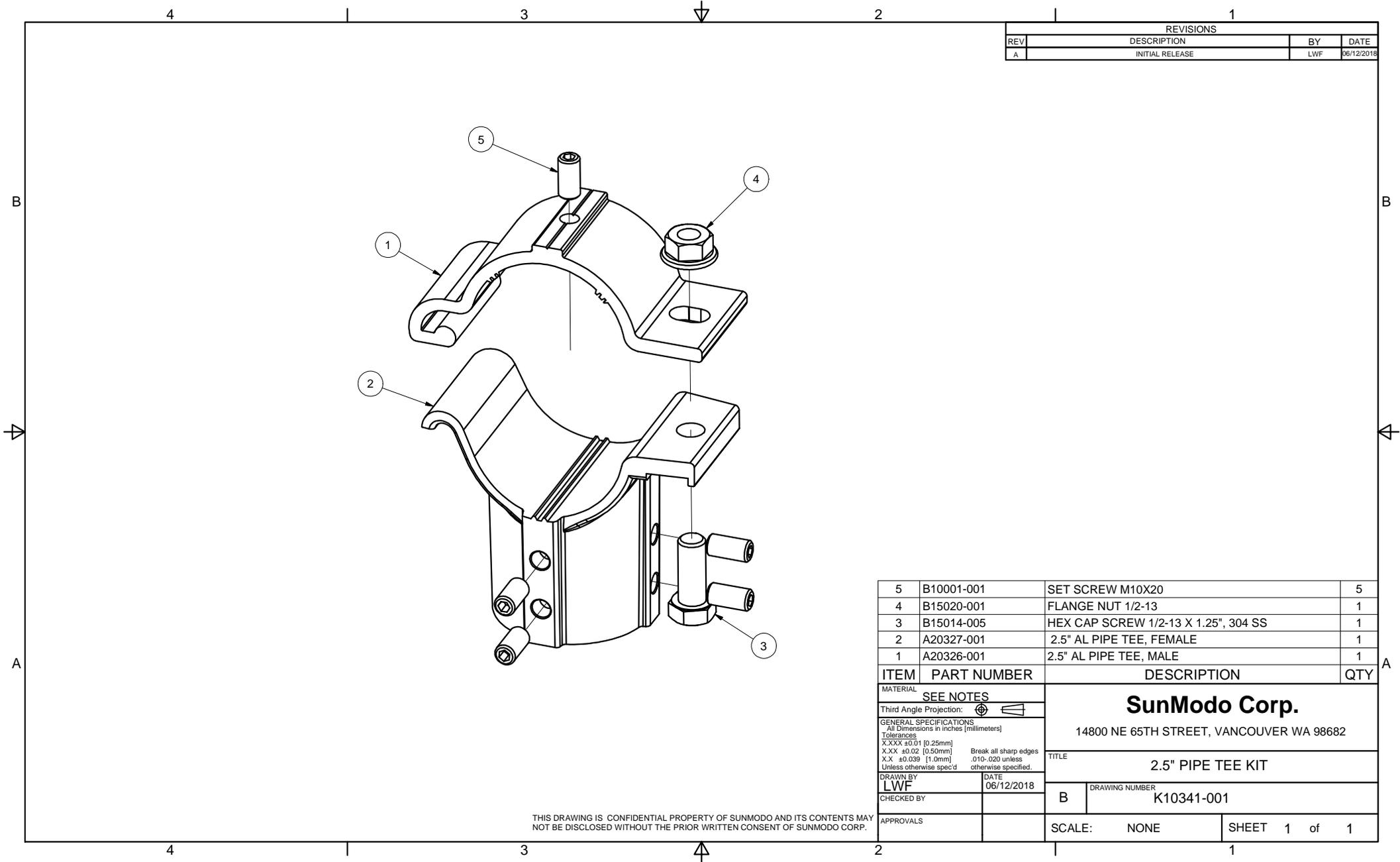
MATERIAL		SEE BOM	
Third Angle Projection:			
GENERAL SPECIFICATIONS All Dimensions in inches [millimeters] Tolerances X.XX ±0.01 (0.25mm) X.X ±0.02 (0.5mm) X.X ±0.03 (0.75mm) Unless otherwise specified.			
DRAWN BY		DATE	
LWF		08/21/2015	
CHECKED BY		B	
APPROVALS		SCALE: NONE	
		SHEET 1 of 1	

Sunmodo Corp.
 1905 E 5TH STREET, STE A, VANCOUVER, WA 98661

TITLE
2.5 AL PIPE CLAMP KIT

DRAWING NUMBER
K10222-001

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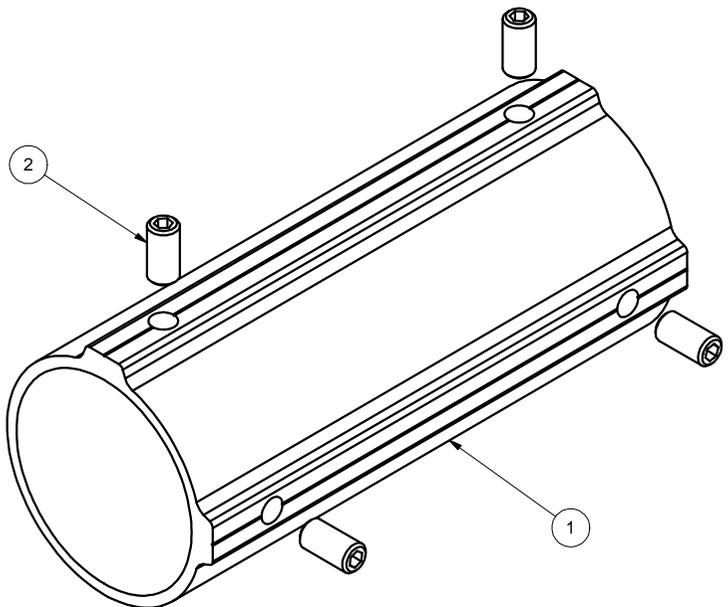
REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	06/12/2018

5	B10001-001	SET SCREW M10X20	5
4	B15020-001	FLANGE NUT 1/2-13	1
3	B15014-005	HEX CAP SCREW 1/2-13 X 1.25", 304 SS	1
2	A20327-001	2.5" AL PIPE TEE, FEMALE	1
1	A20326-001	2.5" AL PIPE TEE, MALE	1
ITEM	PART NUMBER	DESCRIPTION	QTY

MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		SunModo Corp. 14800 NE 65TH STREET, VANCOUVER WA 98682	
All Dimensions in inches [millimeters] Tolerances X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] X.X ±0.039 [1.0mm] Unless otherwise spec'd			
TITLE		2.5" PIPE TEE KIT	
DRAWN BY	DATE	DRAWING NUMBER	
LWF	06/12/2018	B K10341-001	
CHECKED BY			
APPROVALS		SCALE: NONE	SHEET 1 of 1

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REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	06/12/2018



2	B10001-001	SET SCREW M10X20	4
1	A20328-001	2.5" PIPE SPLICE	1
ITEM	PART NUMBER	DESCRIPTION	QTY
MATERIAL		<p style="text-align: center;">SunModo Corp.</p> <p style="text-align: center;">14800 NE 65TH STREET, VANCOUVER WA 98682</p>	
SEE NOTES			
Third Angle Projection:		TITLE	
GENERAL SPECIFICATIONS All Dimensions in inches [millimeters] Tolerances X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] X.X ±0.039 [1.0mm] Unless otherwise spec'd		2.5" PIPE SPLICE KIT	
DRAWN BY	DATE	DRAWING NUMBER	
LWF	06/12/2018	B K10342-001	
CHECKED BY		SCALE: NONE SHEET 1 of 1	
APPROVALS			

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