



Project Number: U2716-115-191

April 6, 2021

Sunmodo
14800 NE 65th Street
Vancouver, WA 98682

**REFERENCE: Sunmodo Sunturf Ground Mount A6 (85x45)
Ground Mount PV Array Installation**

To Whom It May Concern:

Per request of Sunmodo, we have been asked to prepare the structural design of a ground-mounted PV solar array system with several foundation options as shown in the attached calculations. The adopted building code in this jurisdiction is the Connecticut State Building Code, 2018 Edition (2015 IBC). Vector Structural Engineering requires that we review each site specific install, and we are not liable for installs at site specific locations we have not reviewed. The following design parameters are used in our analysis:

- Minimum Design Loads for Buildings and Other Structures (ASCE 7-10)
- Design wind speed for risk category I structures: 140 mph
- Wind exposure: C
- Ground snow load: 70 psf
- The ground screws and helical piers must be tested to 1.5 times uplift and 2.0 times lateral reactions found in the table below.

Load (ASD)	Value (lbs)	Factor of Safety	Test Value (lbs)
UPLIFT	2824	1.5	4236
LATERAL	1816	2	3632

Foundation concrete shall have a minimum compressive strength of 2500 psi at 28 days. Cement for all concrete shall be Type I or II with a minimum of 6% entrained air with a water/cement ratio of 0.50. Maximum aggregate size shall be 3/4". No special inspection of concrete strength is required.

Footings are designed based on an allowable soil bearing pressure of 1500 psf an allowable skin friction of 250 psf, an allowable lateral bearing pressure of 150 pcf, and a coefficient of friction of 0.3. Vector Structural Engineering strongly recommends independent soils testing be performed by a licensed geotechnical engineer to verify the assumed soil parameters.

All ground mounts are to be installed per manufacturer’s recommendations. The use of solar panel support span tables provided by the manufacturer is allowed only where the site conditions and solar panel configuration match the description of the span tables. Electrical engineering is beyond our scope. All work performed must be in accordance with accepted industry-wide methods and applicable safety standards. Vector Structural Engineering assumes no responsibility for improper installation of the solar panels.

Very truly yours,

VECTOR STRUCTURAL ENGINEERING, LLC
Firm License: PEC 0001229

Russell Emery, P.E.
License: PEN.0034376 - Expires: 01/31/2022
Project Engineer

Enclosures

RNE/stb

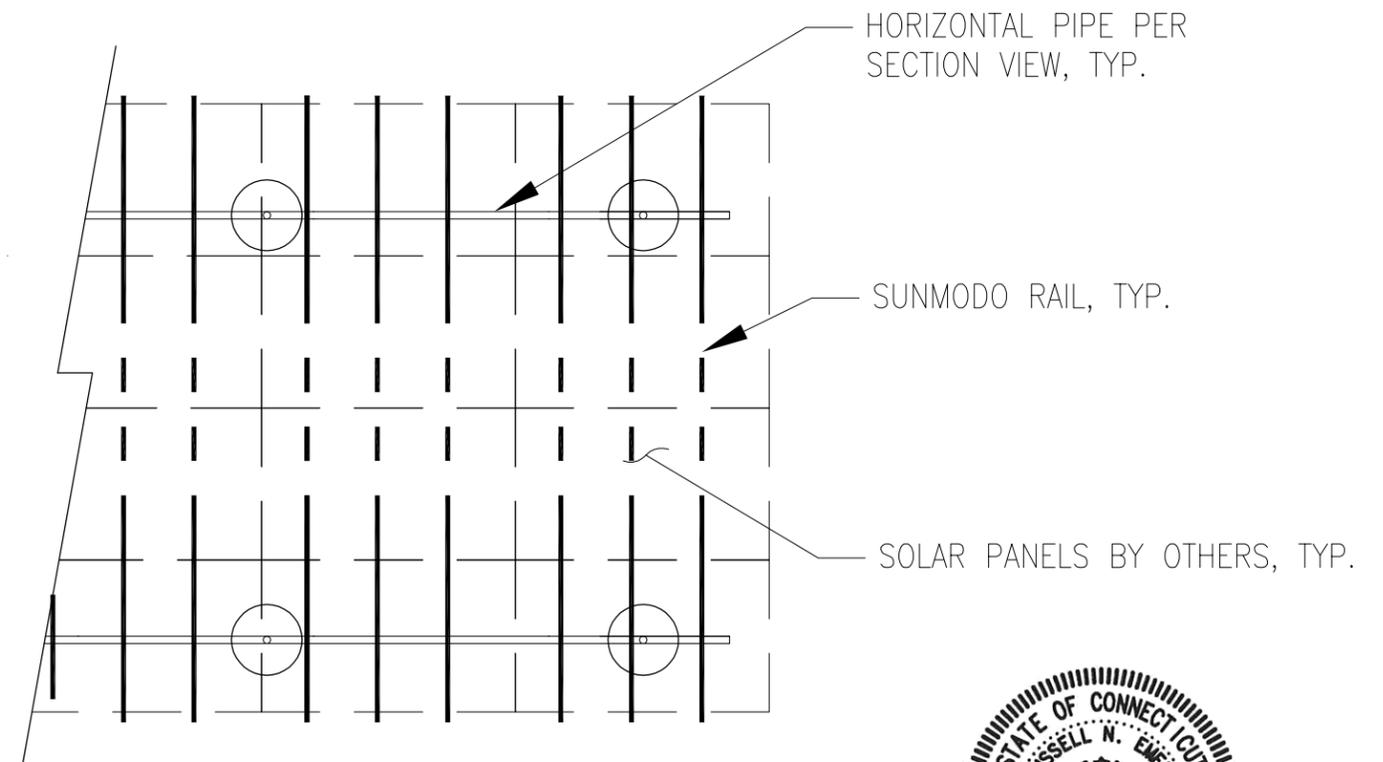
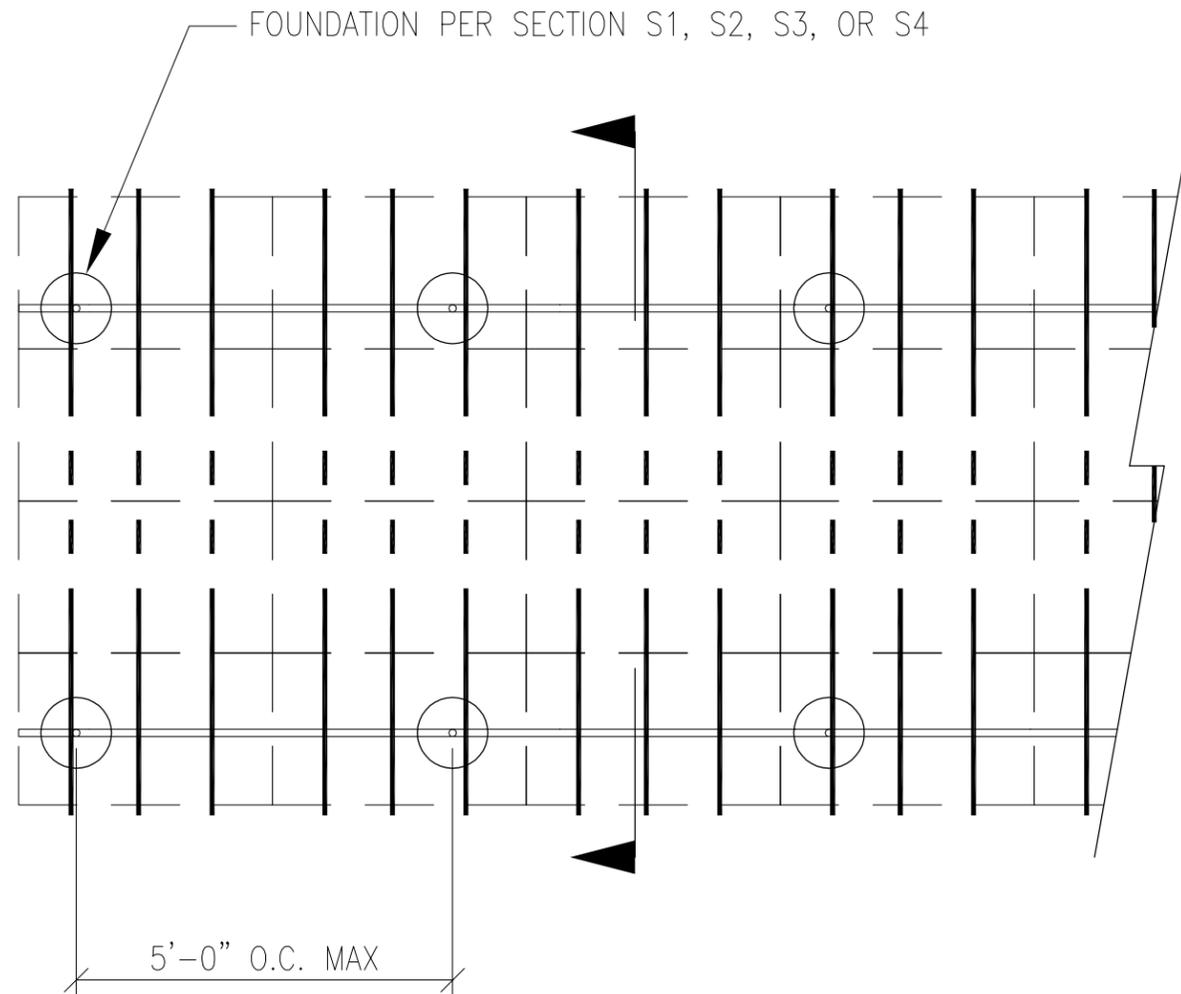


04/06/2021



JOB NO. U2716-115-191
 PROJECT SUNMODO SUNTURF GROUND MOUNTS A6
 SUBJECT ALL OPTIONS

651 W GALENA PARK BLVD. #101 (801) 990-1775
 DRAPER, UTAH 84020 (801) 990-1776 FAX



04/06/2021

PV ARRAY PLAN

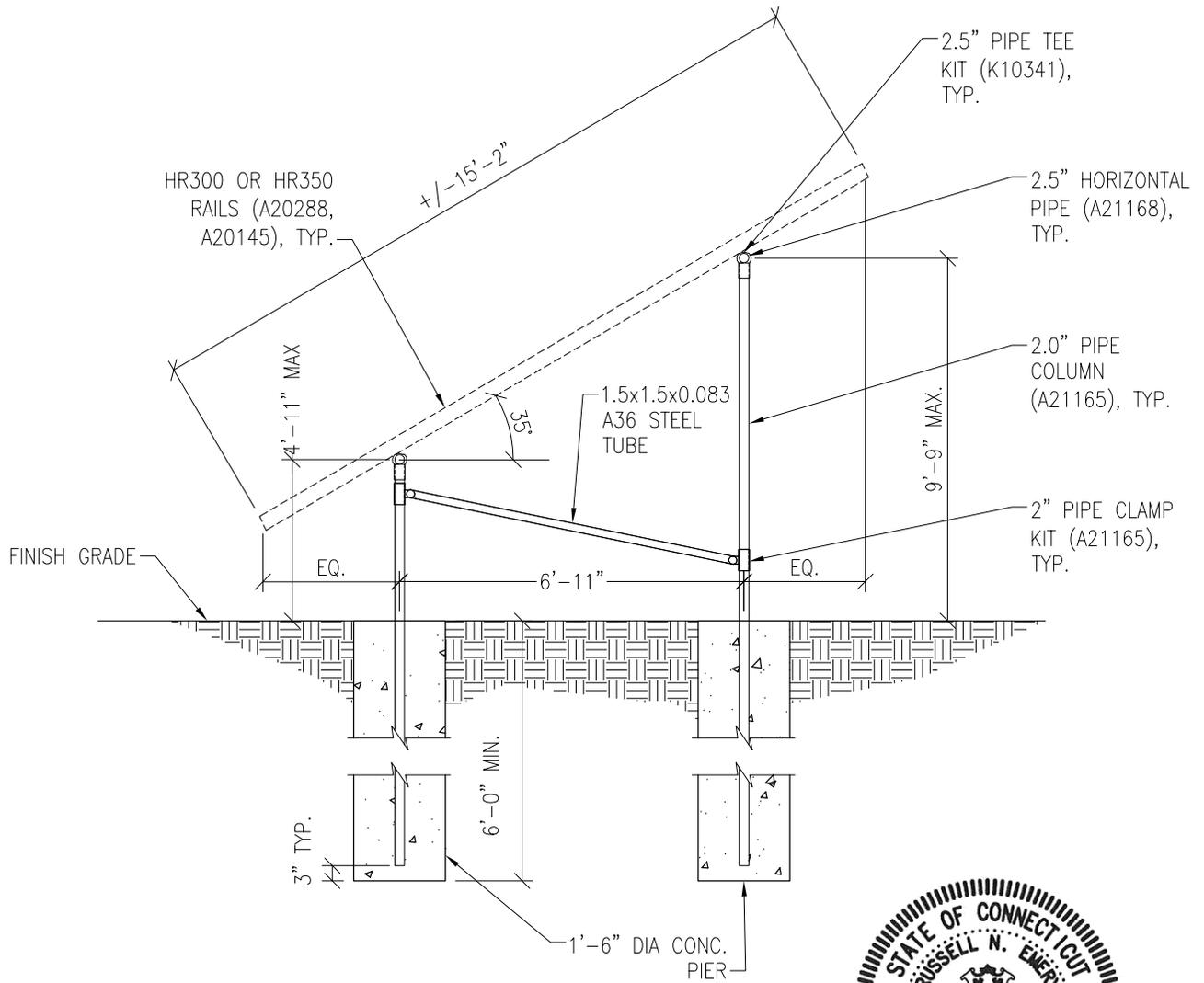
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P1

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PROJECT SUNMODO SUNTURF GROUND MOUNTS A6

SUBJECT DRILLED PIER OPTION



04/06/2021

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PV ARRAY SECTION

N.T.S.

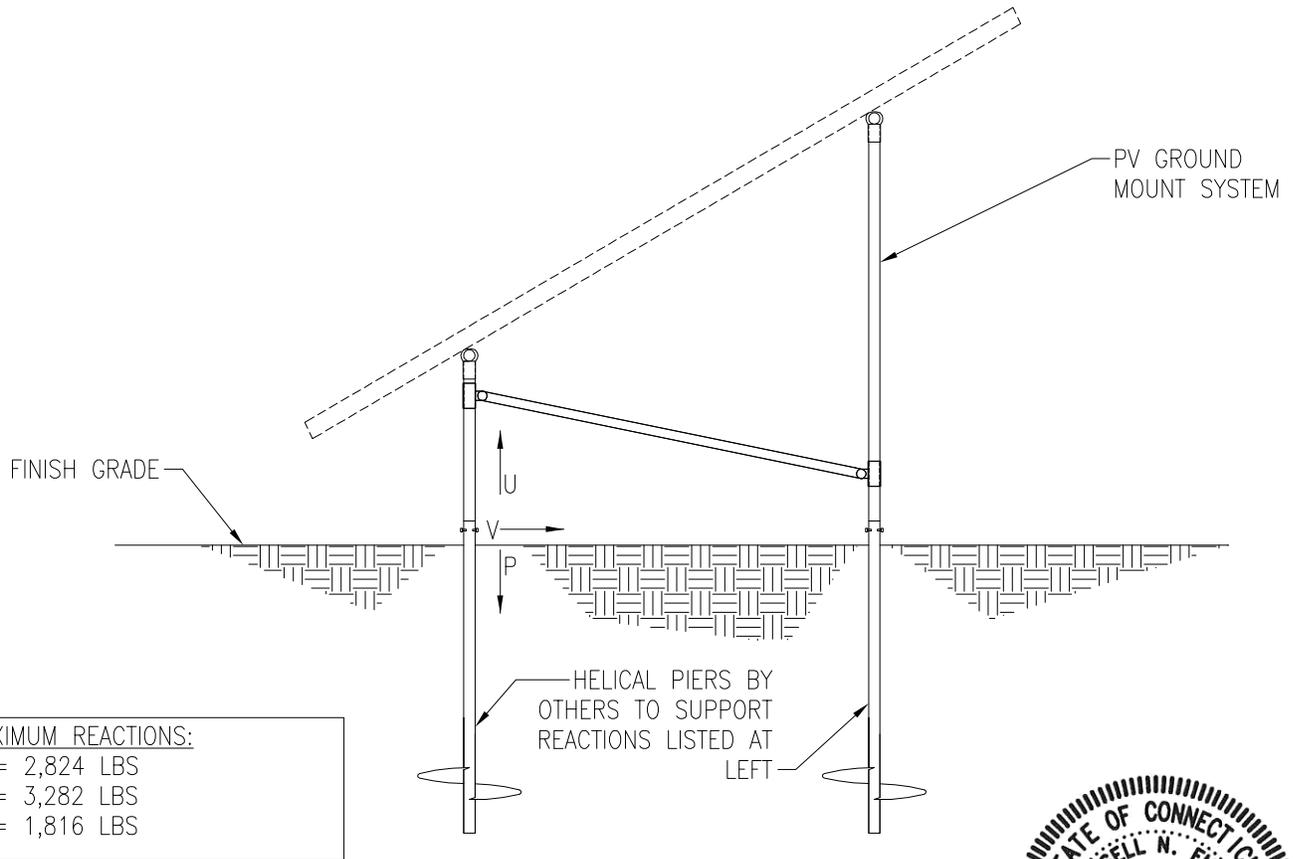
S1

PROJECT SUNMODO SUNTURF GROUND MOUNTS A6

SUBJECT HELICAL PIER OPTION

NOTES:

1. For ground mount components see Section S1.
2. A minimum of (1) helical pier must be load-tested as follows:
 - 2.1. Safety factor for uplift = 1.5,
 - 2.2. Safety factor for lateral loads = 2.0
 - 2.3. Upward deflection limit = 1/2"
 - 2.4. Lateral deflection limit = 1"
 - 2.5. The load tests must be performed by an approved contractor.



MAXIMUM REACTIONS:
 U = 2,824 LBS
 P = 3,282 LBS
 V = 1,816 LBS



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PV ARRAY SECTION

04/06/2021

N.T.S.

S2



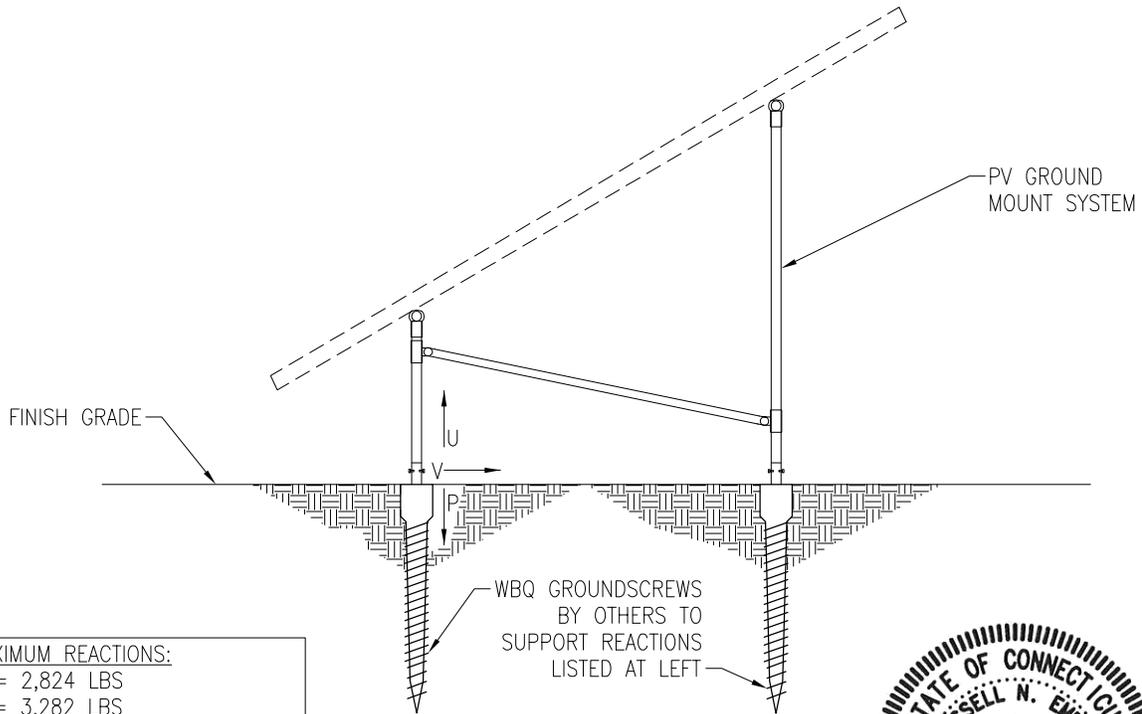
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PROJECT SUNMODO SUNTURF GROUND MOUNTS A6

SUBJECT GROUND SCREW OPTION

NOTES:

- 1. For ground mount components see Section S1.
- 2. A minimum of (1) ground screw must be load-tested as follows:
 - 2.1. Safety factor for uplift = 1.5,
 - 2.2. Safety factor for lateral loads = 2.0
 - 2.3. Upward deflection limit = 1/2"
 - 2.4. Lateral deflection limit = 1"
 - 2.5. The load tests must be performed by an approved contractor.



MAXIMUM REACTIONS:
 U = 2,824 LBS
 P = 3,282 LBS
 V = 1,816 LBS



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PV ARRAY SECTION

N.T.S.

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S3

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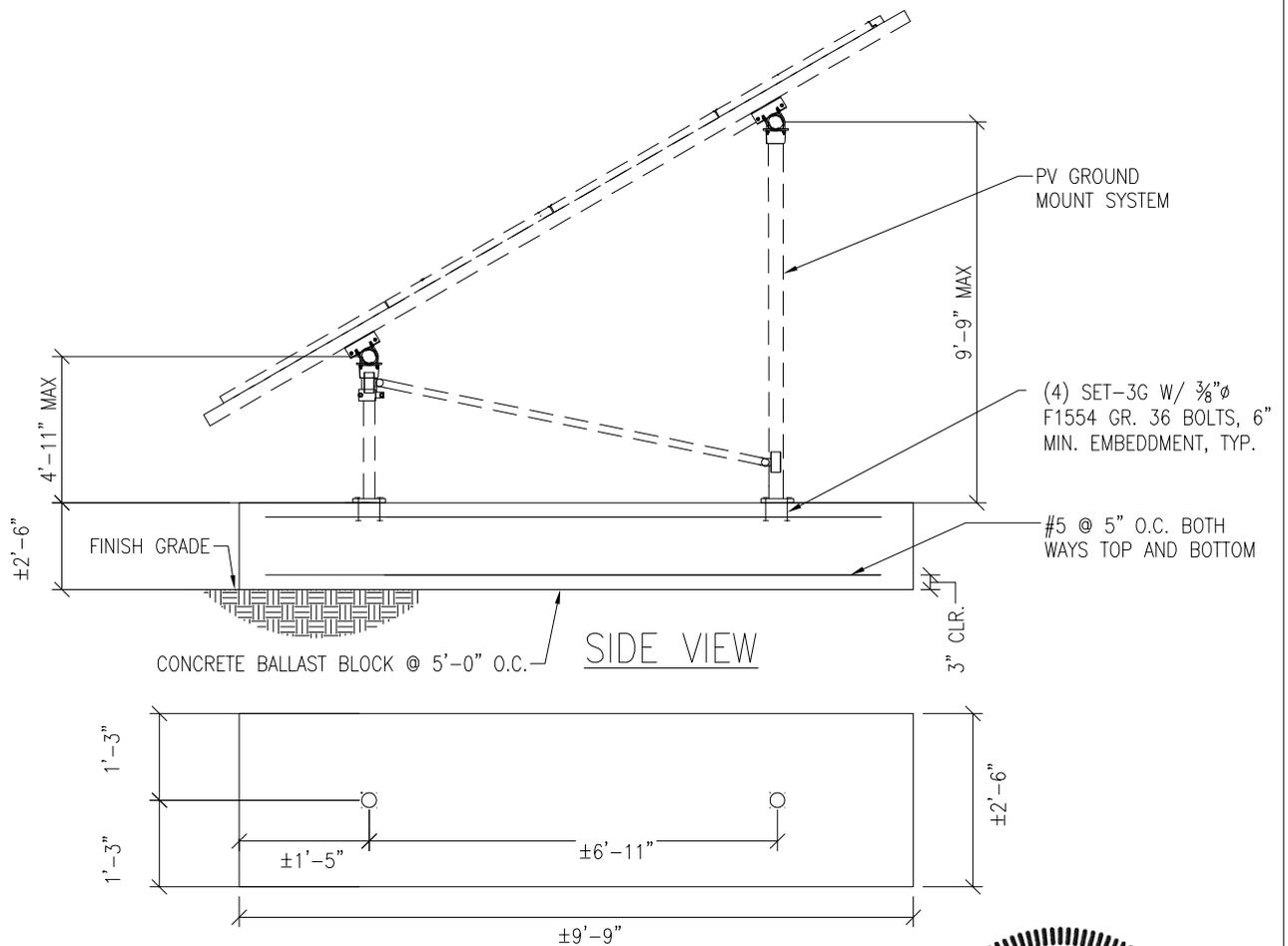
JOB NO. U2716-115-191

PROJECT SUNMOD0 SUNTURF GROUND MOUNTS A6

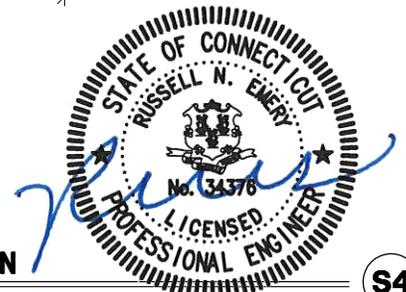
SUBJECT BALLASTED BLOCK OPTION

NOTES:

1. For ground mount components see Section S1.



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PV ARRAY SECTION

N.T.S.

S4

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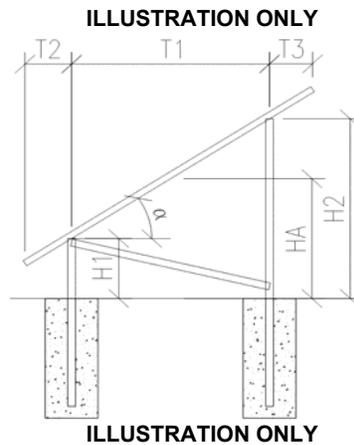
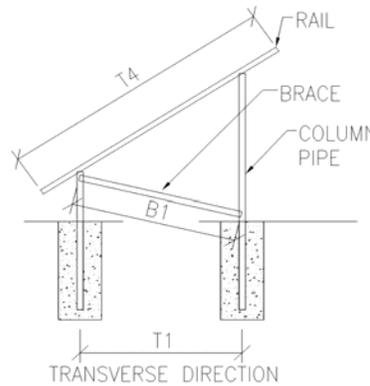
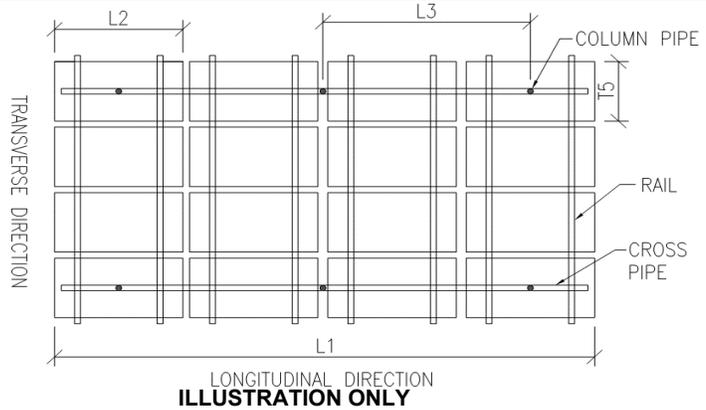
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PROJECT: A6 – Sunmodo Sunturf GM

SUBJECT: Dead Load

Design Weight:

Individual Panel Weight [lb]:	50.7
Panel Transverse Length (T5) [in]:	41.2
Panel Transverse Length (T5) [ft]:	3.4
Panel Longitudinal Length (L2) [in]:	81.4
Panel Longitudinal Length (L2) [ft]:	6.8
Individual Panel Area [ft ²]:	23.3
Individual Panel Weight [psf]:	2.2
# of Panels in Transverse Direction:	4
Approximate Transverse Length (T4) [ft]:	13.7
# of Panels in Longitudinal Direction:	12
Approximate Longitudinal Length (L1) [ft]:	81.4
Transverse Column Spacing (T1) [ft]:	6.7
Longitudinal Column Spacing (L3) [ft]:	6.0
# of Columns in Longitudinal Direction:	14
# of Columns in Transverse Direction:	2
Total Number of Columns:	28
Panel Slope from Horizontal (a) [°]:	35.0
Short Column Height (H1) [ft]:	4.6
Approximate Tall Column Height (H2) [ft]:	9.3
Transverse Brace between Columns :	Yes
Approximate Brace Length (B1) [ft]:	9.0
Weight of Columns [plf]:	3.7
Weight of Cross Pipe [plf]:	3.7
Weight of Brace [plf]:	3.7
Tributary Transverse Length per Column [ft]:	6.9
Tributary Longitudinal Length per Column [ft]:	6.0
Tributary Area per Column [ft ²]:	41.2
Rail Weight [plf]:	1.0
Transverse Rail Weight per Column [lb]:	13.7
Longitudinal Rail Weight per Column [lb]:	21.9
Tall Column Weight [lb]:	33.9
Panel Weight per Column [lb]:	89.7
Rail Weight per Column [lb]:	13.7
Cross Pipe Weight per Column [lb]:	21.9
Brace Weight per Column [lb]:	16.4
Total Weight per Column (1.0 D) [lb]:	175.7



Assumptions:

- T2 = T3



JOB NO.: U2716-115-191

DESIGNED: STB

DATE: 08/01/19

PROJECT: A6 – Sunmodo Sunturf GM

SUBJECT: Snow Load

SNOW LOAD (S):

ASCE 7 Standard:	10	
Panel Slope from Horizontal [°]:	35.0	
Snow Ground Load, p_g [psf]:	70.0	(Section 7.2)
Terrain Category:	C	(Table 7-2)
Exposure of Roof:	Fully Exposed	(Table 7-2)
Exposure Factor, C_e :	0.9	(Table 7-2)
Thermal Factor, C_t :	1.2	(Table 7-3)
Risk Category:	I	(Table 1.5-1)
Importance Factor, I_s :	0.8	(Table 1.5-2)
Flat Roof Snow Load, p_f [psf]:	42	(Equation 7.3-1)
Minimum Roof Snow Load, p_m [psf]:	0	(Section 7.3.4)
Unobstructed Slippery Surface?	Yes	(Section 7.4)
Slope Factor Figure:	Figure 7-2c	(Section 7.4)
Roof Slope Factor, C_s :	0.636	(Figure 7-2)
Sloped Roof Snow Load, p_s [psf]:	27	(Equation 7.4-1)
Design Snow Load, S [psf]:	27	
Tributary Transverse Length [ft]:	5.6	
Tributary Longitudinal Length [ft]:	6	
Tributary Area per Column [ft ²]:	33.7	
Snow Load per Column (1.0 S) [lb]:	909.2	



PROJECT: A6 – Sunmodo Sunturf GM

SUBJECT: Wind Pressure

Design Wind Load:

ASCE 7 Standard:	10	
Basic Wind Speed, V [mph]:	140	
Risk Category:	I	
Exposure Category	C	(Section 26.7.3)
Velocity Pressure Exposure Coefficient, K_h :	0.85	(Table 27.3-1)
Topographic Factor, K_{ht} :	1.0	(Section 26.8.2)
Wind Directionality Factor, K_d :	0.85	(Table 26.6-1)
Internal Pressure Coefficient, GC_{pi} :	0.00	(Table 26.11-1)
Velocity Pressure, q_h [psf]:	36.3	(Equation 27.3-1)
Gust Effect Factor, G:	0.85	(Section 26.9.1)
Panel Slope [degrees]:	35.0	

Net Pressure Coefficients (C_N) per: (Figure 27.4-4)

Clear Wind Flow	C_{NW}	C_{NL}
Case 1 ($\gamma = 0^\circ$, Load Case A)	-1.80	-1.80
Case 2 ($\gamma = 0^\circ$, Load Case B)	-2.43	-0.57
Case 3 ($\gamma = 180^\circ$, Load Case A)	2.10	2.17
Case 4 ($\gamma = 180^\circ$, Load Case B)	2.67	1.07

Design Wind Pressures (p) [psf] per: (Equation 27.4-3)

Clear Wind Flow	$q_h GC_{NW}$	$q_h GC_{NL}$
Case 1 ($\gamma = 0^\circ$, Load Case A)	-55.5	-55.5
Case 2 ($\gamma = 0^\circ$, Load Case B)	-75.0	-17.5
Case 3 ($\gamma = 180^\circ$, Load Case A)	64.7	66.8
Case 4 ($\gamma = 180^\circ$, Load Case B)	82.2	32.9

Wind Pressure on Each Side of Panels [psf]

Clear Wind Flow	Short Col. Pressure	Long Col. Pressure
Case 1 ($\gamma = 0^\circ$, Load Case A)	-55.5	-55.5
Case 2 ($\gamma = 0^\circ$, Load Case B)	-17.5	-75.0
Case 3 ($\gamma = 180^\circ$, Load Case A)	64.7	66.8
Case 4 ($\gamma = 180^\circ$, Load Case B)	82.2	32.9



JOB NO.: U2716-115-191

PROJECT: A6 – Sunmodo Sunturf GM

SUBJECT: Open Building Wind Loads

Design Wind Load Per ASCE 7-10

$$p = q_h G C_n$$

Velocity Pressure Exposure Coefficient, K_{zt} :	0.85	(Table 27.3-1)
Topographic Factor, K_{ht} :	1.0	(Section 26.8.2)
Wind Directionality Factor, K_d :	0.85	(Table 26.6-1)
Ultimate Wind Speed, V [mph]:	140	

Velocity Pressure, q_h [psf]:	36.3	(Equation 27.3-1)
Gust Effect Factor, G :	0.85	(Section 26.9.1)

$\gamma = 90^\circ$ or 270°

Force Coefficient, C_N :

Horizontal Distance from Winward Edge	Roof angle		
	Load Case	Obstructed Wind Flow	
		CN	
$\leq h$	35	A	-0.8
		B	0.8
$> h, \leq 2h$	35	A	-0.6
		B	0.5
$> 2h$	35	A	-0.3
		B	0.3

Design Wind Pressure, p [psf]:

	Roof angle		
	Load Case	Obstructed Wind Flow	
$\leq h$	35	A	-24.7
		B	24.7
$> h, \leq 2h$	35	A	-18.5
		B	15.4
$> 2h$	35	A	-9.2
		B	9.2



JOB NO.: U2716-115-191

DESIGNED: STB

Foundation Option 1: Drilled Concrete Pier



PROJECT: A6 - Sunturf Ground Mount

DRILLED CONCRETE PIER DESIGN

Column Reactions:

Max. Shear, V [k]:	1.8	Max. Down, P _d [k]:	3.3
Max. Moment, M [k-ft]:	0.0	Max. Uplift, P _u [k]:	2.8

Pier Properties:

Pier Shape:	Round	Volume of Concrete [ft ³]:	11
Pier Diameter, b [ft]:	1.5	Volume of Concrete [yd ³]:	0.4
Top of Pier Elevation [ft]:	0.00	Weight of Concrete [k]:	1.6
Pier Depth, d [ft]:	6.0		

Soil Properties:

Allow. Bearing Pressure [psf]:	1,500	<u>Optional Parameters for Uplift:</u>	
1/3 increase for short term loads?	No		
Lateral Bearing, S [pcf]:	150	Skin Friction* [psf]:	250
Max. Lateral Bearing (opt'l) [psf]:		Top Length to Ignore [ft]:	0
Top Depth to Ignore [ft]:	0	1/3 increase for short term loads?	No
1/3 increase for short term loads?	No	Combine w/ Bearing:	No
1/2" deflection at t/o pier allowed:	Yes		

*per IBC Section 1810.3.3.1.4

Check Bearing:

Bearing Capacity [k]:	7.1
-----------------------	-----

Bearing capacity OK.

Check Uplift:

Uplift Capacity [k]:	8.5
----------------------	-----

Uplift capacity OK.

Check Lateral Bearing:

Top of Pier Constrained?:	No
Applied Lateral Force, P [lb]:	1,816
Point of Application, h [ft]:	0.0
S _{max} [psf]:	
S [psf]:	600
A = 2.34*P/(Sb):	4.72
Required Pier Depth, d _{reqd} [ft]:	4.70

IBC Section 1807.3.2.1

IBC Eq. 18-1

Result: **Lateral bearing capacity OK.**

Foundation Option 2: Helical Pier

The ground screws and helical piers must be tested to 1.5 times uplift and 2.0 times lateral reactions found in the table below.

Load (ASD)	Value (lbs)	Factor of Safety	Test Value (lbs)
UPLIFT	2824	1.5	4236
LATERAL	1816	2	3632

Foundation Option 3: Ground Screw

The ground screws and helical piers must be tested to 1.5 times uplift and 2.0 times lateral reactions found in the table below.

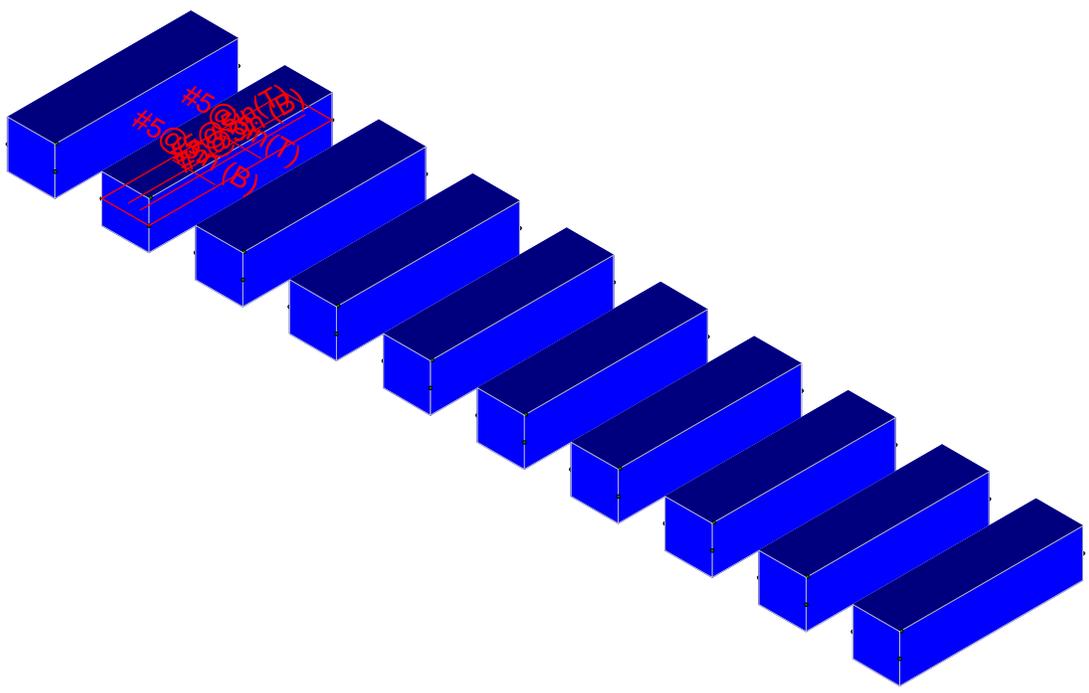
Load (ASD)	Value (lbs)	Factor of Safety	Test Value (lbs)
UPLIFT	2824	1.5	4236
LATERAL	1816	2	3632



JOB NO.: U2716-115-191

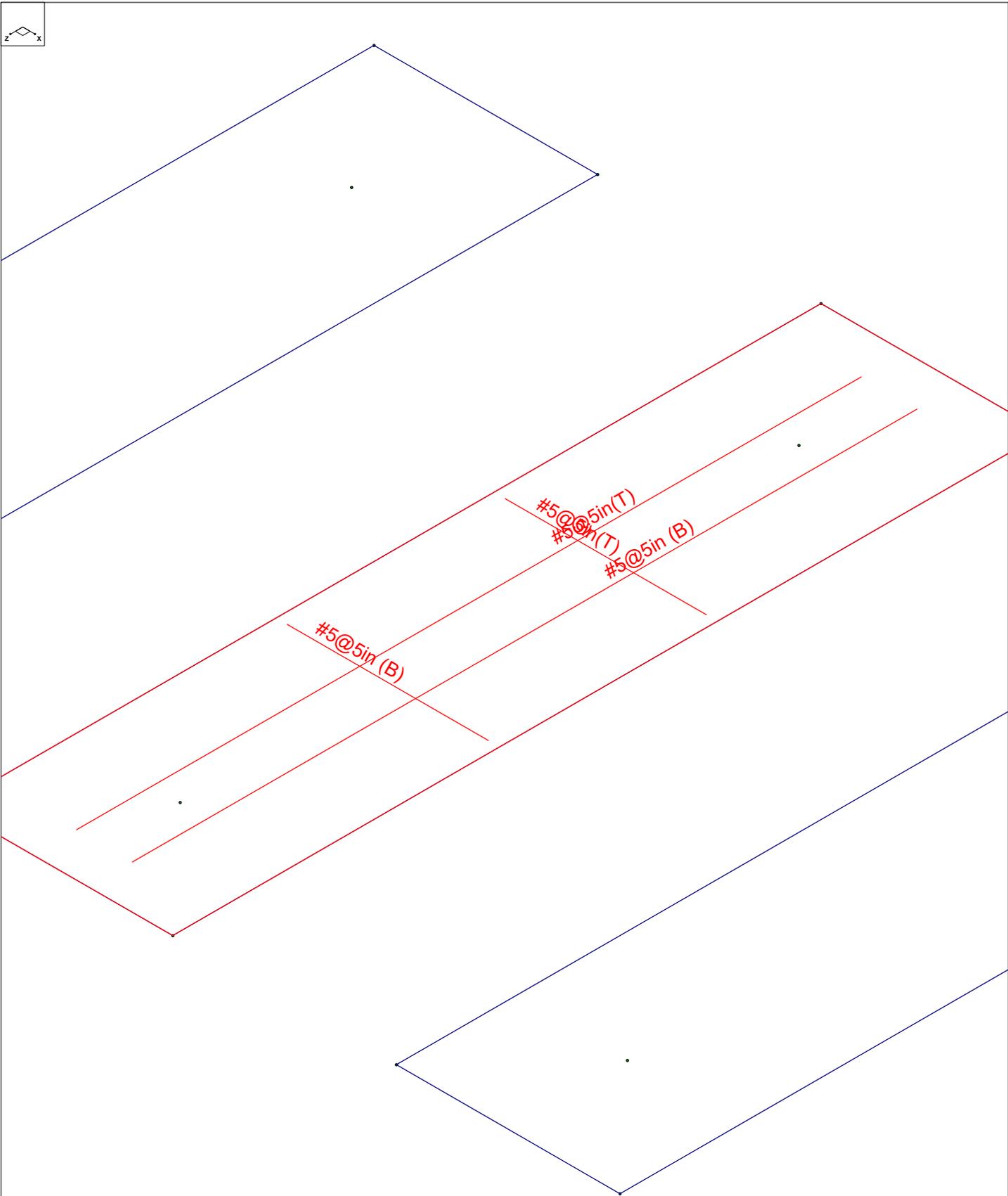
DESIGNED: STB

Foundation Option 4: Ballasted Block



Results for LC 1, 1.0 D

Vector Structural Engineeri...	Ground Mount	SK - 2
STB		Apr 6, 2021 at 12:52 PM
U2716.115.191		Sunmodo Sunturf A6 v3 85x45.r3d



Results for LC 1, 1.0 D

Vector Structural Engineeri..	Ground Mount	SK - 1
STB		Apr 6, 2021 at 12:52 PM
U2716.115.191		Sunmodo Sunturf A6 v3 85x45.r3d



(Global) Model Settings

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	100
Mesh Size (in)	12
Max Iterations	10
Merge Tolerance (in)	.12
Solver	Sparse Accelerated
Coefficient of Friction	.3
No. of Shear Regions	4
Shear Region Spacing Increment (in)	4
Min 1 Bar Dia Spacing for Beams?	No
Optimize footings for OTM / Sliding?	Yes
Parme Beta Factor	.65
Pile Safety Factor	3
Concrete Stress Block	0
Concrete Rebar Set	Rectangular
Concrete Code	ASTM A615
HR Steel Pile Code	ACI 318-14
Wood Pile Code	AISC 14th (360-10): ASD AWC NDS-15: ASD

Concrete Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (/...	Density[lb/ft^3]	fc[psi]	Lambda	Flex Stee...	Shear St...
1	Conc3000NW	3156	1372	.15	.6	145	3000	1	60000	60000
2	Conc3500NW	3409	1482	.15	.6	145	3500	1	60000	60000
3	Conc4000NW	3644	1584	.15	.6	145	4000	1	60000	60000
4	Conc3000LW	2085	907	.15	.6	109.999	3000	.75	60000	60000
5	Conc3500LW	2252	979	.15	.6	109.999	3500	.75	60000	60000
6	Conc4000LW	2408	1047	.15	.6	109.999	4000	.75	60000	60000
7	Conc2500NW	3156	1372	.15	.6	145	2500	1	60000	60000

General Design Parameters

	Label	Max Bending Chk	Max Shear Chk	Top Cover[in]	Bottom Cover[in]
1	Typical	1	1	3	3

Slab Rebar Parameters

	Label	Top Bar	Bottom Bar	Max Top Bar Sp...	Min Top Bar Sp...	Max Bot Bar Sp...	Min Bot Bar Sp...	Spacing Incr...	Rebar Options
1	Typical	#5	#5	5	5	5	5	1	Optimize

Soil Definitions

	Label	Subgrade Modulus[lb/ft^3]	Allowable Bearing[psf]	Depth Properties	Default?
1	Default	1e+5	1500	None	Yes

Point Loads and Moments (Cat 1 : DL)

	Label	Direction	Magnitude[lb.-ft]
1	R3D_N1_1	X	-3.98
2	R3D_N1_1	Y	178.273
3	R3D_N2	Y	158.516
4	R3D_N132_1	Y	174.459
5	R3D_N133_1	Y	168.182
6	R3D_N109_1	Y	171.845
7	R3D_N110A_1	Y	169.721
8	R3D_N121_1	Y	175.462

Point Loads and Moments (Cat 1 : DL) (Continued)

	Label	Direction	Magnitude[lb,lb-ft]
9	R3D N122 1	Y	169.648
10	R3D N133B	Y	176.265
11	R3D N134B 1	Y	170.596
12	R3D N151 1	Y	174.762
13	R3D N152 1	Y	168.584
14	R3D N143A	Y	175.943
15	R3D N144A	Y	170.266
16	R3D N149A 1	Y	176.347
17	R3D N150A 1	Y	170.317
18	R3D N155B	X	3.82
19	R3D N155B	Y	188.842
20	R3D N156B	Y	169.101
21	R3D N161A	Y	167.594
22	R3D N162A	Y	166.106

Point Loads and Moments (Cat 6 : RLL)

	Label	Direction	Magnitude[lb,lb-ft]
1	R3D N1 1	X	-21.702
2	R3D N1 1	Y	785.405
3	R3D N2	X	-6.227
4	R3D N2	Y	793.566
5	R3D N132 1	X	-1.022
6	R3D N132 1	Y	804.458
7	R3D N133 1	X	-3.243
8	R3D N133 1	Y	851.582
9	R3D N109 1	Y	775.38
10	R3D N109 1	Z	-2.175
11	R3D N110A 1	Y	863.372
12	R3D N121 1	Y	812.243
13	R3D N121 1	Z	1.889
14	R3D N122 1	X	2.144
15	R3D N122 1	Y	860.793
16	R3D N133B	Y	815.3
17	R3D N134B 1	X	-1.204
18	R3D N134B 1	Y	866.267
19	R3D N151 1	X	1.092
20	R3D N151 1	Y	806.457
21	R3D N152 1	X	3.992
22	R3D N152 1	Y	854.235
23	R3D N143A	Y	813.328
24	R3D N144A	X	2.085
25	R3D N144A	Y	864.255
26	R3D N149A 1	Y	817.326
27	R3D N149A 1	Z	1.86
28	R3D N150A 1	X	-1.808
29	R3D N150A 1	Y	864.514
30	R3D N155B	X	20.773
31	R3D N155B	Y	846.639
32	R3D N156B	X	1.002
33	R3D N156B	Y	855.305
34	R3D N161A	X	1.057
35	R3D N161A	Y	750.771
36	R3D N161A	Z	-2.077
37	R3D N162A	X	2.549
38	R3D N162A	Y	842.455

Point Loads and Moments (Cat 16 : OL1) (Continued)

	Label	Direction	Magnitude[lb,lb-ft]
58	R3D_N162A	Y	741.788
59	R3D_N162A	Z	-108.909

Point Loads and Moments (Cat 17 : OL2)

	Label	Direction	Magnitude[lb,lb-ft]
1	R3D_N1_1	X	84.227
2	R3D_N1_1	Y	-4529.418
3	R3D_N1_1	Z	1962.399
4	R3D_N2	Y	1695.467
5	R3D_N2	Z	-88.307
6	R3D_N132_1	X	4.131
7	R3D_N132_1	Y	-4683.677
8	R3D_N132_1	Z	2090.855
9	R3D_N133_1	X	-4.271
10	R3D_N133_1	Y	1809.081
11	R3D_N133_1	Z	-92.558
12	R3D_N109_1	X	1.226
13	R3D_N109_1	Y	-4551.312
14	R3D_N109_1	Z	2098.328
15	R3D_N110A_1	Y	1823.971
16	R3D_N110A_1	Z	-91.267
17	R3D_N121_1	X	-1.568
18	R3D_N121_1	Y	-4812.607
19	R3D_N121_1	Z	2154.688
20	R3D_N122_1	X	-4.428
21	R3D_N122_1	Y	1900.345
22	R3D_N122_1	Z	-93.354
23	R3D_N133B	X	2.189
24	R3D_N133B	Y	-4761.157
25	R3D_N133B	Z	2124.75
26	R3D_N134B_1	X	-4.241
27	R3D_N134B_1	Y	1855.365
28	R3D_N134B_1	Z	-93.169
29	R3D_N151_1	X	-4.258
30	R3D_N151_1	Y	-4744.303
31	R3D_N151_1	Z	2129.749
32	R3D_N152_1	X	1.786
33	R3D_N152_1	Y	1861.026
34	R3D_N152_1	Z	-93.106
35	R3D_N143A	X	-2.599
36	R3D_N143A	Y	-4721.489
37	R3D_N143A	Z	2100.687
38	R3D_N144A	X	2.607
39	R3D_N144A	Y	1822.669
40	R3D_N144A	Z	-92.938
41	R3D_N149A_1	X	1.533
42	R3D_N149A_1	Y	-4843.049
43	R3D_N149A_1	Z	2168.876
44	R3D_N150A_1	X	3.74
45	R3D_N150A_1	Y	1913.375
46	R3D_N150A_1	Z	-93.603
47	R3D_N155B	X	-80.636
48	R3D_N155B	Y	-4878.367
49	R3D_N155B	Z	2116.916
50	R3D_N156B	X	5.078
51	R3D_N156B	Y	1831.929



Point Loads and Moments (Cat 17 : OL2) (Continued)

	Label	Direction	Magnitude[lb.lb-ft]
52	R3D_N156B	Z	-92.46
53	R3D_N161A	X	-4.251
54	R3D_N161A	Y	-4422.713
55	R3D_N161A	Z	2043.051
56	R3D_N162A	Y	1774.973
57	R3D_N162A	Z	-91.533

Point Loads and Moments (Cat 18 : OL3)

	Label	Direction	Magnitude[lb.lb-ft]
1	R3D_N1_1	X	-77.163
2	R3D_N1_1	Y	4794.763
3	R3D_N1_1	Z	-2749.119
4	R3D_N2	X	-10.396
5	R3D_N2	Y	-890.924
6	R3D_N2	Z	124.006
7	R3D_N132_1	X	-3.809
8	R3D_N132_1	Y	4991.044
9	R3D_N132_1	Z	-2949.286
10	R3D_N133_1	X	-1.49
11	R3D_N133_1	Y	-953.436
12	R3D_N133_1	Z	130.296
13	R3D_N109_1	X	-1.235
14	R3D_N109_1	Y	4877.106
15	R3D_N109_1	Z	-2976.431
16	R3D_N110A_1	X	2.014
17	R3D_N110A_1	Y	-952.006
18	R3D_N110A_1	Z	128.777
19	R3D_N121_1	X	1.428
20	R3D_N121_1	Y	5112.723
21	R3D_N121_1	Z	-3010.008
22	R3D_N122_1	X	7.709
23	R3D_N122_1	Y	-1028.821
24	R3D_N122_1	Z	130.973
25	R3D_N133B	X	-2.036
26	R3D_N133B	Y	5064.046
27	R3D_N133B	Z	-2986.866
28	R3D_N134B_1	X	1.978
29	R3D_N134B_1	Y	-971.952
30	R3D_N134B_1	Z	130.936
31	R3D_N151_1	X	3.917
32	R3D_N151_1	Y	5049.05
33	R3D_N151_1	Z	-2987.909
34	R3D_N152_1	X	5.08
35	R3D_N152_1	Y	-999.634
36	R3D_N152_1	Z	130.842
37	R3D_N143A	X	2.398
38	R3D_N143A	Y	5026.019
39	R3D_N143A	Z	-2962.3
40	R3D_N144A	X	1.048
41	R3D_N144A	Y	-943.364
42	R3D_N144A	Z	130.722
43	R3D_N149A_1	X	-1.392
44	R3D_N149A_1	Y	5144.149
45	R3D_N149A_1	Z	-3029.265
46	R3D_N150A_1	X	-6.52
47	R3D_N150A_1	Y	-1037.95

Point Loads and Moments (Cat 18 : OL3) (Continued)

	Label	Direction	Magnitude[lb,lb-ft]
48	R3D_N150A_1	Z	131.318
49	R3D_N155B	X	73.921
50	R3D_N155B	Y	5168.703
51	R3D_N155B	Z	-2968.668
52	R3D_N156B	X	-3.251
53	R3D_N156B	Y	-968.136
54	R3D_N156B	Z	129.917
55	R3D_N161A	X	3.977
56	R3D_N161A	Y	4738.267
57	R3D_N161A	Z	-2894.591
58	R3D_N162A	X	3.833
59	R3D_N162A	Y	-921.034
60	R3D_N162A	Z	129.051

Point Loads and Moments (Cat 19 : OL4)

	Label	Direction	Magnitude[lb,lb-ft]
1	R3D_N1_1	X	-40.202
2	R3D_N1_1	Y	3140.962
3	R3D_N1_1	Z	-2380.15
4	R3D_N2	X	-15.657
5	R3D_N2	Y	195.771
6	R3D_N2	Z	107.556
7	R3D_N132_1	X	-2.01
8	R3D_N132_1	Y	3297.983
9	R3D_N132_1	Z	-2566.622
10	R3D_N133_1	X	-6.16
11	R3D_N133_1	Y	204.625
12	R3D_N133_1	Z	113.221
13	R3D_N109_1	Y	3245.923
14	R3D_N109_1	Z	-2600.989
15	R3D_N110A_1	X	2.985
16	R3D_N110A_1	Y	220.386
17	R3D_N110A_1	Z	112.094
18	R3D_N121_1	Y	3364.929
19	R3D_N121_1	Z	-2600.494
20	R3D_N122_1	X	7.655
21	R3D_N122_1	Y	173.544
22	R3D_N122_1	Z	113.52
23	R3D_N133B	X	-1.092
24	R3D_N133B	Y	3338.032
25	R3D_N133B	Z	-2592.705
26	R3D_N134B_1	Y	218.778
27	R3D_N134B_1	Z	113.634
28	R3D_N151_1	X	2.057
29	R3D_N151_1	Y	3330.661
30	R3D_N151_1	Z	-2589.724
31	R3D_N152_1	X	9.338
32	R3D_N152_1	Y	181.94
33	R3D_N152_1	Z	113.549
34	R3D_N143A	X	1.266
35	R3D_N143A	Y	3316.539
36	R3D_N143A	Z	-2577.394
37	R3D_N144A	X	3.97
38	R3D_N144A	Y	232.31
39	R3D_N144A	Z	113.52
40	R3D_N149A_1	Y	3384.826



Point Loads and Moments (Cat 21 : OL6) (Continued)

	Label	Direction	Magnitude[lb.lb-ft]
40	R3D N150A_1	X	1.157
41	R3D N150A_1	Y	137.764
42	R3D N150A_1	Z	-18.402
43	R3D N155B	X	-18.212
44	R3D N155B	Y	-735.31
45	R3D N155B	Z	413.793
46	R3D N156B	Y	130.373
47	R3D N156B	Z	-18.15
48	R3D N161A	X	-1.514
49	R3D N161A	Y	-640.281
50	R3D N161A	Z	407.35
51	R3D N162A	Y	124.414
52	R3D N162A	Z	-18.105

Slabs

	Label	Thickness [in]	Material	Local Axis Angle ...	Analysis Offset [in]	Passive Pressur...	Soil Overburden [psf]
1	S1	30	Conc2500NW	0	0	0	0
2	S2	30	Conc2500NW	0	0	0	0
3	S3	30	Conc2500NW	0	0	0	0
4	S4	30	Conc2500NW	0	0	0	0
5	S5	30	Conc2500NW	0	0	0	0
6	S6	30	Conc2500NW	0	0	0	0
7	S7	30	Conc2500NW	0	0	0	0
8	S8	30	Conc2500NW	0	0	0	0
9	S9	30	Conc2500NW	0	0	0	0
10	S10	30	Conc2500NW	0	0	0	0

Load Combinations

Label	Solve	Service A...	SF	Cat...	Fa...	C...	F...	C...	F...								
1	1.0 D	Yes	Yes	1.5	DL	1											
2	1.0 D + 1....	Yes	Yes	1.5	DL	1	RLL	1									
3	1.0 D + 0....	Yes	Yes	1.5	DL	1	RLL	OL1	.6								
4	1.0 D + 0....	Yes	Yes	1.5	DL	1	RLL	OL2	.6								
5	1.0 D + 0....	Yes	Yes	1.5	DL	1	RLL	OL3	.6								
6	1.0 D + 0....	Yes	Yes	1.5	DL	1	RLL	OL4	.6								
7	1.0 D + 0....	Yes	Yes	1.5	DL	1	RLL	OL5	.6								
8	1.0 D + 0....	Yes	Yes	1.5	DL	1	RLL	OL6	.6								
9	1.0 D + 0....	Yes	Yes	1.5	DL	1	RLL	.75	OL1	.45							
10	1.0 D + 0....	Yes	Yes	1.5	DL	1	RLL	.75	OL2	.45							
11	1.0 D + 0....	Yes	Yes	1.5	DL	1	RLL	.75	OL3	.45							
12	1.0 D + 0....	Yes	Yes	1.5	DL	1	RLL	.75	OL4	.45							
13	1.0 D + 0....	Yes	Yes	1.5	DL	1	RLL	.75	OL5	.45							
14	1.0 D + 0....	Yes	Yes	1.5	DL	1	RLL	.75	OL6	.45							
15	0.9 D + 0....	Yes	Yes		DL	.9	RLL	OL1	.6								
16	0.9 D + 0....	Yes	Yes		DL	.9	RLL	OL2	.6								
17	0.9 D + 0....	Yes	Yes		DL	.9	RLL	OL3	.6								
18	0.9 D + 0....	Yes	Yes		DL	.9	RLL	OL4	.6								
19	0.9 D + 0....	Yes	Yes		DL	.9	RLL	OL5	.6								
20	0.9 D + 0....	Yes	Yes		DL	.9	RLL	OL6	.6								
21																	
22	1.4D	Yes			DL	1.4											
23	1.2D+1.6SL	Yes			DL	1.2	RLL	1.6									
24	1.2D+1.6S...	Yes			DL	1.2	RLL	1.6	OL1	.5							
25	1.2D+1.6S...	Yes			DL	1.2	RLL	1.6	OL2	.5							
26	1.2D+1.6S...	Yes			DL	1.2	RLL	1.6	OL3	.5							
27	1.2D+1.6S...	Yes			DL	1.2	RLL	1.6	OL4	.5							



Company : Vector Structural Engineering
 Designer : STB
 Job Number : U2716.115.191
 Model Name : Ground Mount

Apr 6, 2021
 12:52 PM
 Checked By: JSP

Slab Sliding Safety Factors (By Combination) (Continued)

LC	Slab	Angle[deg]	Va-xx[lb]	Vr-xx[lb]	Va-zz[lb]	Vr-zz[lb]	SR-xx	SR-zz	
42	5	S2	0	.467	3459.773	1708.592	3459.773	9.999+	2.025
43	5	S3	0	5.482	3489.421	1727.422	3489.421	9.999+	2.02
44	5	S4	0	.035	3491.421	1713.558	3491.421	9.999+	2.038
45	5	S5	0	3.18	3480.348	1691.394	3480.348	9.999+	2.058
46	5	S6	0	5.398	3482.685	1714.24	3482.685	9.999+	2.032
47	5	S7	0	2.068	3489.526	1698.946	3489.526	9.999+	2.054
48	5	S8	0	4.747	3493.901	1738.768	3493.901	9.999+	2.009
49	5	S9	0	4.686	3437.998	1659.324	3437.998	9.999+	2.072
50	5	S10	0	46.222	3514.271	1703.25	3514.271	9.999+	2.063
51	6	S1	0	37.496	3352.434	1363.556	3352.434	9.999+	2.459
52	6	S2	0	1.791	3377.191	1493.337	3377.191	9.999+	2.262
53	6	S3	0	4.593	3391.244	1492.184	3391.244	9.999+	2.273
54	6	S4	0	.655	3395.07	1487.443	3395.07	9.999+	2.282
55	6	S5	0	4.901	3384.048	1472.041	3384.048	9.999+	2.299
56	6	S6	0	6.837	3386.058	1485.705	3386.058	9.999+	2.279
57	6	S7	0	3.142	3393.441	1478.324	3393.441	9.999+	2.295
58	6	S8	0	3.888	3394.939	1501.767	3394.939	9.999+	2.261
59	6	S9	0	4.605	3358.449	1448.999	3358.449	9.999+	2.318
60	6	S10	0	26.956	3404.912	1475.705	3404.912	9.999+	2.307
61	7	S1	0	26.063	3023.034	609.802	3023.034	9.999+	4.957
62	7	S2	0	5.493	2983.875	561.173	2983.875	9.999+	5.317
63	7	S3	0	0	2955.865	475.203	2955.865	9.999+	6.22
64	7	S4	0	4.503	2896.197	325.809	2896.197	9.999+	8.889
65	7	S5	0	.952	2849.778	225.389	2849.778	9.999+	9.999+
66	7	S6	0	.807	2857.014	241.009	2857.014	9.999+	9.999+
67	7	S7	0	.643	2857.089	237.683	2857.089	9.999+	9.999+
68	7	S8	0	.734	2858.653	242.495	2858.653	9.999+	9.999+
69	7	S9	0	.889	2843.871	233.542	2843.871	9.999+	9.999+
70	7	S10	0	14.588	2866.928	237.387	2866.928	9.999+	9.999+
71	8	S1	0	18.904	2478.986	613.246	2478.986	9.999+	4.042
72	8	S2	0	6.9	2537.982	525.349	2537.982	9.999+	4.831
73	8	S3	0	0	2589.978	389.475	2589.978	9.999+	6.65
74	8	S4	0	3.589	2625.175	297.53	2625.175	9.999+	8.823
75	8	S5	0	.731	2655.9	228.772	2655.9	9.999+	9.999+
76	8	S6	0	.813	2650.88	240.754	2650.88	9.999+	9.999+
77	8	S7	0	.66	2652.144	237.702	2652.144	9.999+	9.999+
78	8	S8	0	.694	2650.914	242.492	2650.914	9.999+	9.999+
79	8	S9	0	.908	2658.039	233.547	2658.039	9.999+	9.999+
80	8	S10	0	7.107	2649.28	237.386	2649.28	9.999+	9.999+
81	9	S1	0	8.007	2662.607	996.752	2662.607	9.999+	2.671
82	9	S2	0	.32	2674.548	1079.987	2674.548	9.999+	2.476
83	9	S3	0	1.868	2665.542	1094.498	2665.542	9.999+	2.435
84	9	S4	0	.851	2667.001	1084.496	2667.001	9.999+	2.459
85	9	S5	0	1.131	2666.231	1070.569	2666.231	9.999+	2.49
86	9	S6	0	.34	2666.146	1084.867	2666.146	9.999+	2.458
87	9	S7	0	.666	2666.983	1075.342	2666.983	9.999+	2.48
88	9	S8	0	1.654	2665.456	1101.65	2665.456	9.999+	2.42
89	9	S9	0	.275	2674.221	1048.839	2674.221	9.999+	2.55
90	9	S10	0	6.324	2662.825	1077.9	2662.825	9.999+	2.47
91	10	S1	0	12.975	2724.508	843.342	2724.508	9.999+	3.231
92	10	S2	0	.552	2753.783	901.546	2753.783	9.999+	3.055
93	10	S3	0	1.09	2737.597	929.017	2737.597	9.999+	2.947
94	10	S4	0	1.826	2740.915	914.211	2740.915	9.999+	2.998
95	10	S5	0	3.262	2738.116	899.234	2738.116	9.999+	3.045
96	10	S6	0	2.7	2738.203	916.49	2738.203	9.999+	2.988
97	10	S7	0	1.568	2740.764	903.487	2740.764	9.999+	3.034
98	10	S8	0	1.017	2737.693	935.268	2737.693	9.999+	2.927



Slab Sliding Safety Factors (By Combination) (Continued)

	LC	Slab	Angle[deg]	Va-xx[lb]	Vr-xx[lb]	Va-zz[lb]	Vr-zz[lb]	SR-xx	SR-zz
99	10	S9	0	.791	2751.927	876.626	2751.927	9.999+	3.139
100	10	S10	0	13.85	2729.837	911.005	2729.837	9.999+	2.997
101	11	S1	0	64.329	3634.109	1181.301	3634.109	9.999+	3.076
102	11	S2	0	.351	3651.863	1283.075	3651.863	9.999+	2.846
103	11	S3	0	5.72	3682.079	1294.149	3682.079	9.999+	2.845
104	11	S4	0	.929	3685.629	1285.168	3685.629	9.999+	2.868
105	11	S5	0	5.584	3671.264	1268.545	3671.264	9.999+	2.894
106	11	S6	0	7.861	3674.117	1285.68	3674.117	9.999+	2.858
107	11	S7	0	3.115	3683.263	1274.21	3683.263	9.999+	2.891
108	11	S8	0	4.916	3687.536	1302.681	3687.536	9.999+	2.831
109	11	S9	0	6.218	3624.698	1246.05	3624.698	9.999+	2.909
110	11	S10	0	51.953	3708.183	1277.438	3708.183	9.999+	2.903
111	12	S1	0	50.064	3557.55	1022.667	3557.55	9.999+	3.479
112	12	S2	0	1.343	3589.926	1121.634	3589.926	9.999+	3.201
113	12	S3	0	5.053	3608.446	1117.721	3608.446	9.999+	3.228
114	12	S4	0	1.394	3613.366	1115.582	3613.366	9.999+	3.239
115	12	S5	0	6.875	3599.039	1104.031	3599.039	9.999+	3.26
116	12	S6	0	8.94	3601.647	1114.279	3601.647	9.999+	3.232
117	12	S7	0	3.92	3611.199	1108.743	3611.199	9.999+	3.257
118	12	S8	0	4.272	3613.315	1124.93	3613.315	9.999+	3.212
119	12	S9	0	6.158	3565.036	1088.307	3565.036	9.999+	3.276
120	12	S10	0	37.504	3626.164	1106.779	3626.164	9.999+	3.276
121	13	S1	0	41.49	3310.5	457.351	3310.5	9.999+	7.238
122	13	S2	0	4.12	3294.94	422.511	3294.94	9.999+	7.798
123	13	S3	0	1.608	3281.911	354.985	3281.911	9.999+	9.245
124	13	S4	0	2.474	3239.212	244.357	3239.212	9.999+	9.999+
125	13	S5	0	3.913	3198.337	169.042	3198.337	9.999+	9.999+
126	13	S6	0	4.418	3204.864	180.757	3204.864	9.999+	9.999+
127	13	S7	0	2.046	3208.935	178.262	3208.935	9.999+	9.999+
128	13	S8	0	1.907	3211.1	180.476	3211.1	9.999+	9.999+
129	13	S9	0	3.371	3179.103	176.714	3179.103	9.999+	9.999+
130	13	S10	0	28.227	3222.676	178.04	3222.676	9.999+	9.999+
131	14	S1	0	7.765	2902.464	459.935	2902.464	9.999+	6.311
132	14	S2	0	5.175	2960.519	392.38	2960.519	9.999+	7.545
133	14	S3	0	1.608	3007.496	293.523	3007.496	9.999+	9.999+
134	14	S4	0	3.594	3035.945	223.147	3035.945	9.999+	9.999+
135	14	S5	0	2.651	3052.929	171.579	3052.929	9.999+	9.999+
136	14	S6	0	3.202	3050.263	180.565	3050.263	9.999+	9.999+
137	14	S7	0	1.069	3055.226	178.276	3055.226	9.999+	9.999+
138	14	S8	0	.835	3055.296	183.264	3055.296	9.999+	9.999+
139	14	S9	0	2.023	3039.729	173.603	3039.729	9.999+	9.999+
140	14	S10	0	11.956	3059.44	178.04	3059.44	9.999+	9.999+
141	15	S1	0	40.331	1883.995	1329.003	1883.995	9.999+	1.418
142	15	S2	0	.427	1881.362	1442.158	1881.362	9.999+	1.305
143	15	S3	0	4.636	1858.608	1457.441	1858.608	9.999+	1.275
144	15	S4	0	.069	1857.766	1445.995	1857.766	9.999+	1.285
145	15	S5	0	2.757	1864.946	1427.425	1864.946	9.999+	1.307
146	15	S6	0	4.63	1863.344	1446.49	1863.344	9.999+	1.288
147	15	S7	0	1.197	1859.022	1433.789	1859.022	9.999+	1.297
148	15	S8	0	4.013	1855.649	1467.007	1855.649	9.999+	1.265
149	15	S9	0	3.973	1895.606	1400.529	1895.606	9.999+	1.353
150	15	S10	0	31.863	1844.643	1437.2	1844.643	9.999+	1.283
151	16	S1	0	46.954	1966.529	1124.455	1966.529	9.999+	1.749
152	16	S2	0	.736	1987.008	1204.236	1987.008	9.999+	1.65
153	16	S3	0	3.598	1954.68	1236.8	1954.68	9.999+	1.58
154	16	S4	0	1.232	1956.317	1218.949	1956.317	9.999+	1.605
155	16	S5	0	.084	1960.793	1198.978	1960.793	9.999+	1.635



Company : Vector Structural Engineering
Designer : STB
Job Number : U2716.115.191
Model Name : Ground Mount

Apr 6, 2021
12:52 PM
Checked By: JSP

Envelope Slab Soil Pressures (Continued)

	Label	UC	LC	Soil Pressure[psf]	Allowable Bearing[psf]	Point
9	S9	.504	5	755.862	1500	N338
10	S10	.528	5	791.89	1500	N345



Company:		Date:	5/14/2018
Engineer:		Page:	1/6
Project:			
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-14
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: F1554 Grade 36
Diameter (inch): 0.375
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: ICC-ES ESR-4057
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 7.25
 c_{ac} (inch): 10.99
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 30.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Ignore concrete breakout in tension: No
Ignore concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 150/110°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.75 x 4.75 x 0.31

Recommended Anchor

Anchor Name: SET-3G - SET-3G w/ 3/8"Ø F1554 Gr. 36
Code Report: ICC-ES ESR-4057





Company:		Date:	5/14/2018
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Address:			
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Load and Geometry

Load factor source: ACI 318 Section 5.3

Load combination: not set

Seismic design: No

Anchors subjected to sustained tension: No

Apply entire shear load at front row: No

Anchors only resisting wind and/or seismic loads: No

Strength level loads:

N_{ua} [lb]: 4690

V_{uax} [lb]: 175

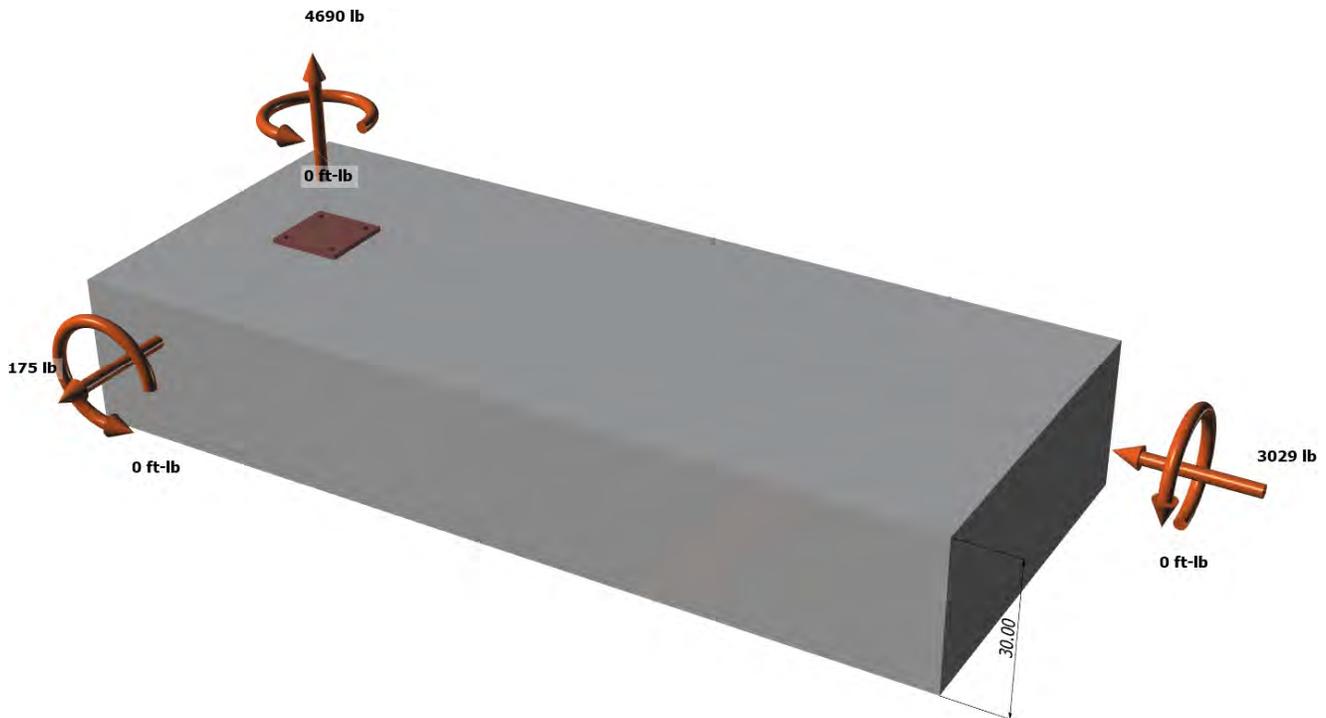
V_{uay} [lb]: -3029

M_{ux} [ft-lb]: 0

M_{uy} [ft-lb]: 0

M_{uz} [ft-lb]: 0

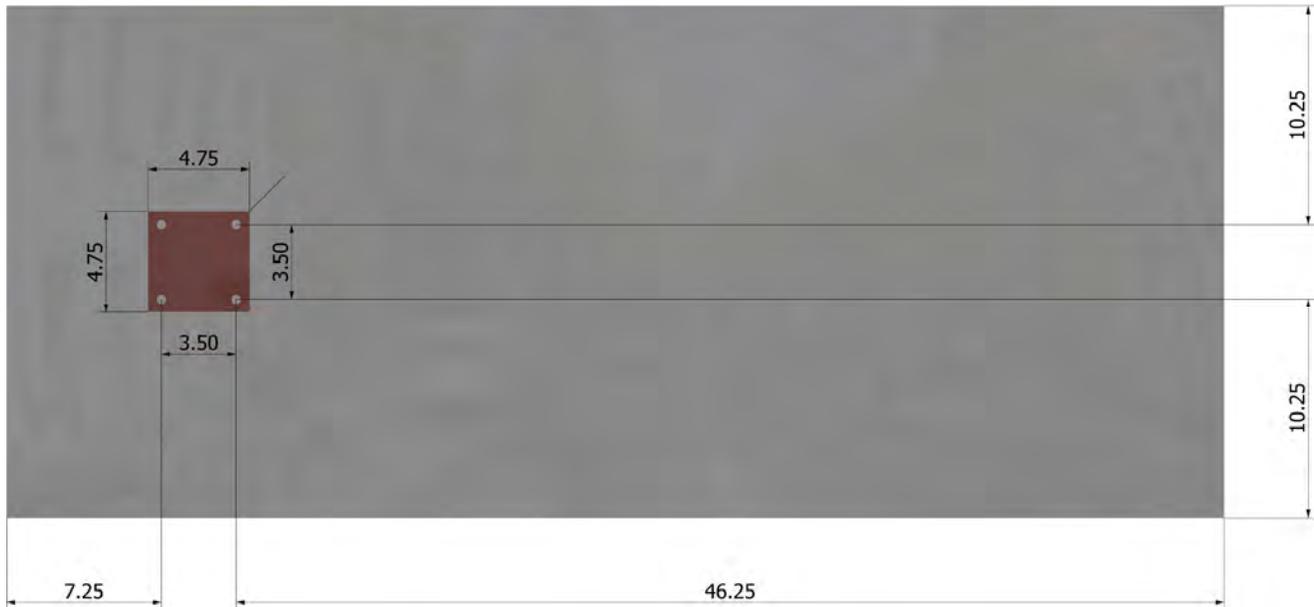
<Figure 1>





Company:		Date:	5/14/2018
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E-mail:			

<Figure 2>





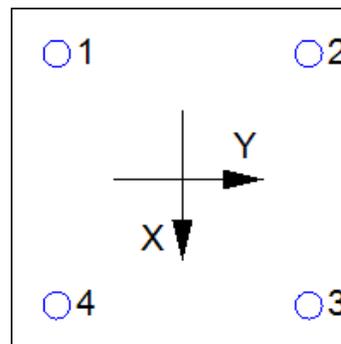
Company:		Date:	5/14/2018
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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1172.5	43.7	-757.3	758.5
2	1172.5	43.7	-757.3	758.5
3	1172.5	43.7	-757.3	758.5
4	1172.5	43.7	-757.3	758.5
Sum	4690.0	175.0	-3029.0	3034.1

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 4690
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. 17.4.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
4525	0.75	3394

5. Concrete Breakout Strength of Anchor in Tension (Sec. 17.4.2)

$$N_b = K_c \lambda_a \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. 17.4.2.2a)}$$

K _c	λ _a	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. 17.3.1 \& Eq. 17.4.2.1b)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	C _{a,min} (in)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
424.63	324.00	7.25	1.000	0.942	1.00	1.000	12492	0.65	10021

6. Adhesive Strength of Anchor in Tension (Sec. 17.4.5)

$$\tau_{k,cr} = \tau_{k,cr,short-term} K_{sat} (f_c / 2,500)^n$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	f _c (psi)	n	τ _{k,cr} (psi)
1346	1.00	1.00	2500	0.24	1346

$$N_{ba} = \lambda_a \tau_{cr} \pi d_a h_{ef} \text{ (Eq. 17.4.5.2)}$$

λ _a	τ _{cr} (psi)	d _a (in)	h _{ef} (in)	N _{ba} (lb)
1.00	1346	0.38	6.000	9514

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ec,Na} \psi_{ed,Na} \psi_{cp,Na} N_{ba} \text{ (Sec. 17.3.1 \& Eq. 17.4.5.1b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	C _{Na} (in)	C _{a,min} (in)	ψ _{ec,Na}	ψ _{ed,Na}	ψ _{cp,Na}	N _{ba} (lb)	φ	φN _{ag} (lb)
198.45	112.09	5.29	7.25	1.000	1.000	1.000	9514	0.55	9265



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8. Steel Strength of Anchor in Shear (Sec. 17.5.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
2715	1.0	0.65	1765

9. Concrete Breakout Strength of Anchor in Shear (Sec. 17.5.2)

Shear perpendicular to edge in x-direction:

$V_{bx} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$ (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

l_e (in)	d_a (in)	λ_a	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
3.00	0.375	1.00	2500	13.75	16564

$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \Psi_{ec,V} \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{bx}$ (Sec. 17.3.1 & Eq. 17.5.2.1b)

A_{Vc} (in ²)	A_{Vco} (in ²)	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
647.11	850.78	1.000	0.805	1.000	1.000	16564	0.70	7103

Shear perpendicular to edge in y-direction:

$V_{by} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$ (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

l_e (in)	d_a (in)	λ_a	f_c (psi)	c_{a1} (in)	V_{by} (lb)
3.00	0.375	1.00	2500	10.75	11450

$\phi V_{cbgy} = \phi (A_{Vc} / A_{Vco}) \Psi_{ec,V} \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{by}$ (Sec. 17.3.1 & Eq. 17.5.2.1b)

A_{Vc} (in ²)	A_{Vco} (in ²)	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgy} (lb)
387.00	520.03	1.000	0.891	1.000	1.000	11450	0.70	5313

Shear parallel to edge in x-direction:

$V_{by} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$ (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

l_e (in)	d_a (in)	λ_a	f_c (psi)	c_{a1} (in)	V_{by} (lb)
3.00	0.375	1.00	2500	7.25	6342

$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \Psi_{ec,V} \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{by}$ (Sec. 17.3.1, 17.5.2.1(c) & Eq. 17.5.2.1b)

A_{Vc} (in ²)	A_{Vco} (in ²)	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
261.00	236.53	1.000	1.000	1.000	1.000	6342	0.70	9797

Shear parallel to edge in y-direction:

$V_{bx} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$ (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

l_e (in)	d_a (in)	λ_a	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
3.00	0.375	1.00	2500	10.25	10661

$\phi V_{cbgy} = \phi (2)(A_{Vc} / A_{Vco}) \Psi_{ec,V} \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{bx}$ (Sec. 17.3.1, 17.5.2.1(c) & Eq. 17.5.2.1b)

A_{Vc} (in ²)	A_{Vco} (in ²)	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbgy} (lb)
401.67	472.78	1.000	1.000	1.000	1.000	10661	0.70	12680

10. Concrete Pryout Strength of Anchor in Shear (Sec. 17.5.3)

$\phi V_{cp} = \phi \min|k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min|k_{cp} (A_{Na} / A_{Na0}) \Psi_{ec,Na} \Psi_{ed,Na} \Psi_{cp,Na} N_{ba}; k_{cp} (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{cp,N} N_{b}|$ (Sec. 17.3.1 & Eq. 17.5.3.1b)

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\Psi_{ed,Na}$	$\Psi_{ec,Na}$	$\Psi_{cp,Na}$	N_{ba} (lb)	N_a (lb)
2.0	198.45	112.09	1.000	1.000	1.000	9514	16845

A_{Nc} (in ²)	A_{Nco} (in ²)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
424.63	324.00	1.000	0.942	1.000	1.000	12492	15417	0.70

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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ϕV_{cpq} (lb)
21584

11. Results

Interaction of Tensile and Shear Forces (Sec. R17.6)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	1173	3394	0.35	Pass	
Concrete breakout	4690	10021	0.47	Pass	
Adhesive	4690	9265	0.51	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	759	1765	0.43	Pass	
T Concrete breakout x+	175	7103	0.02	Pass	
T Concrete breakout y-	3029	5313	0.57	Pass	
Concrete breakout y-	87	9797	0.01	Pass	
Concrete breakout x-	1515	12680	0.12	Pass	
Concrete breakout, combined	-	-	0.57	Pass (Governs)	
Pryout	3034	21584	0.14	Pass	
Interaction check	$(N_{ua}/\phi N_{ua})^{5/3}$	$(V_{ua}/\phi V_{ua})^{5/3}$	Combined Ratio	Permissible	Status
Sec. R17.6	0.32	0.39	71.4%	1.0	Pass

SET-3G w/ 3/8"Ø F1554 Gr. 36 with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.

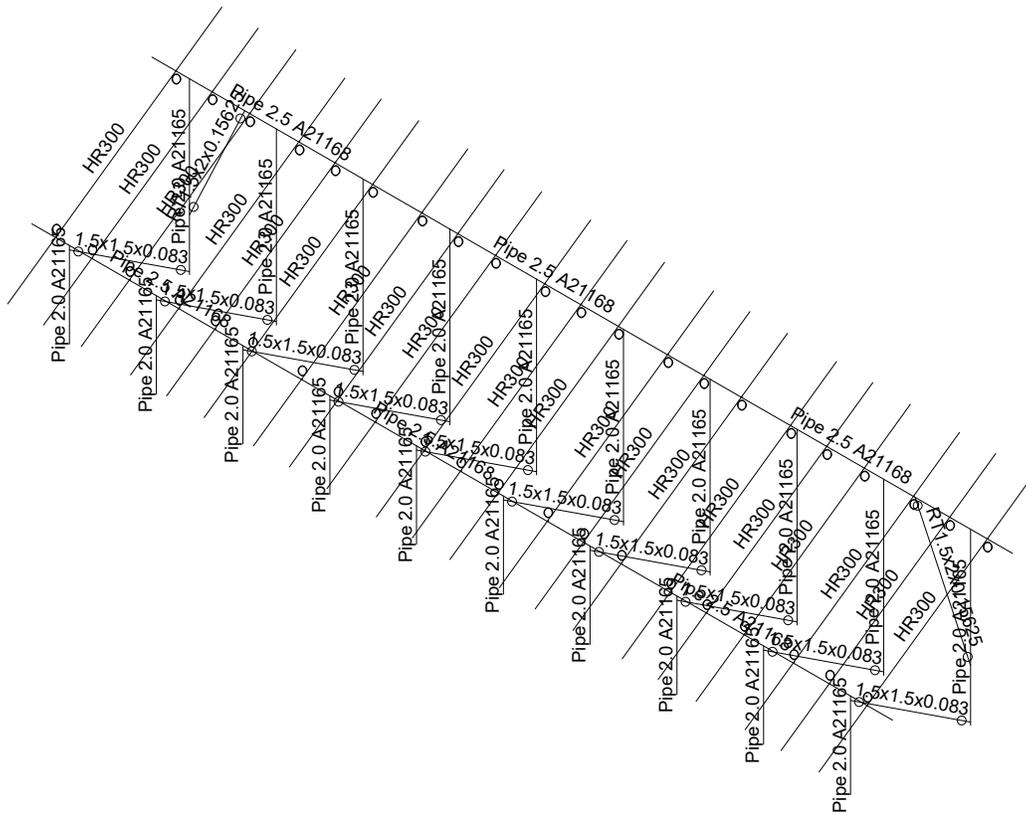


JOB NO.: U2716-070-181

DESIGNED: STB

PROJECT: Ground Mount Package for Ontario Canada

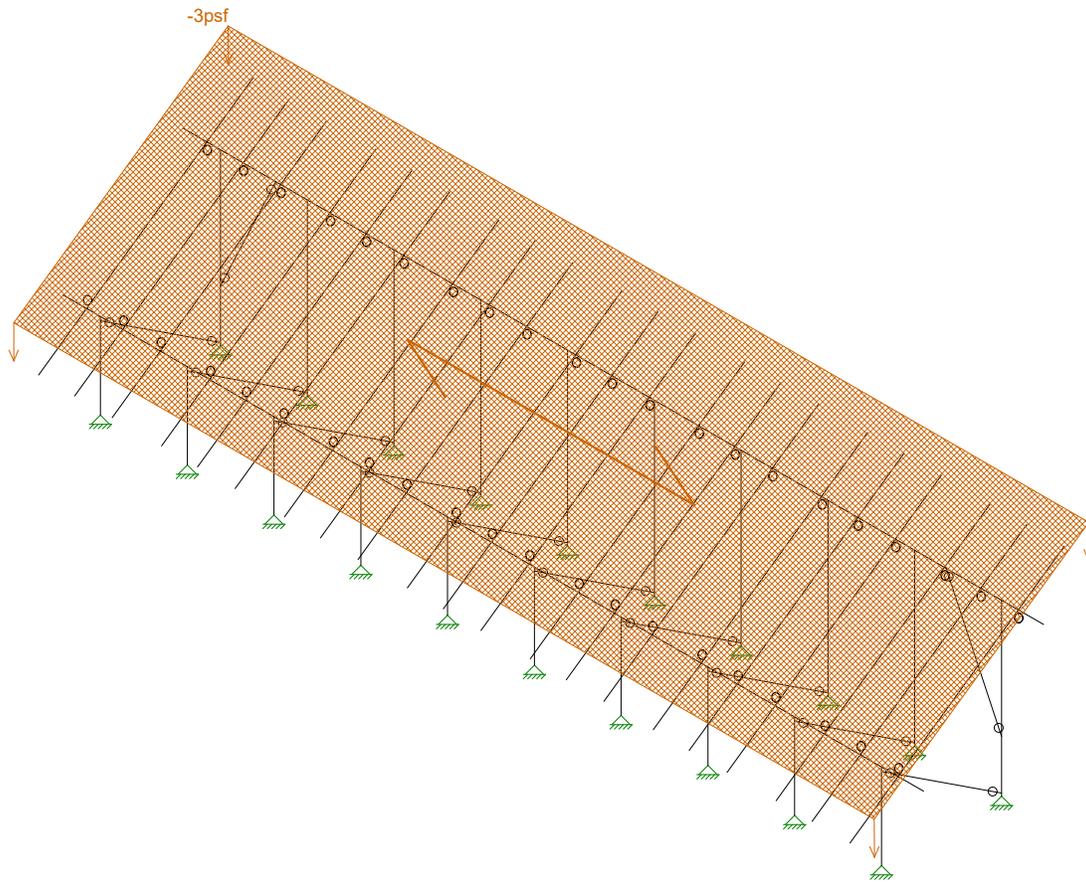
Framing Analysis



Vector Structural Engineeri...
STB
U2716.115.191

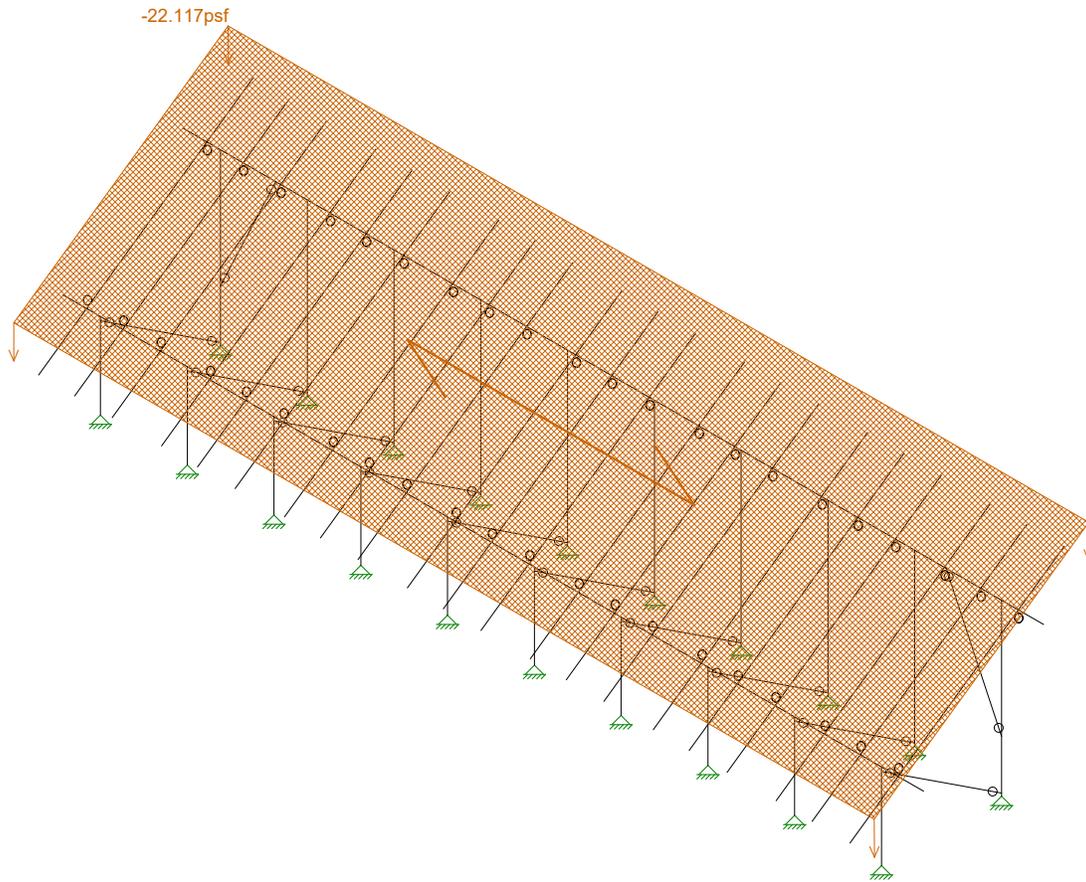
Ground Mount

SK - 5
Apr 6, 2021 at 12:46 PM
Sunmodo Sunturf A6 v3 85x45.r3d



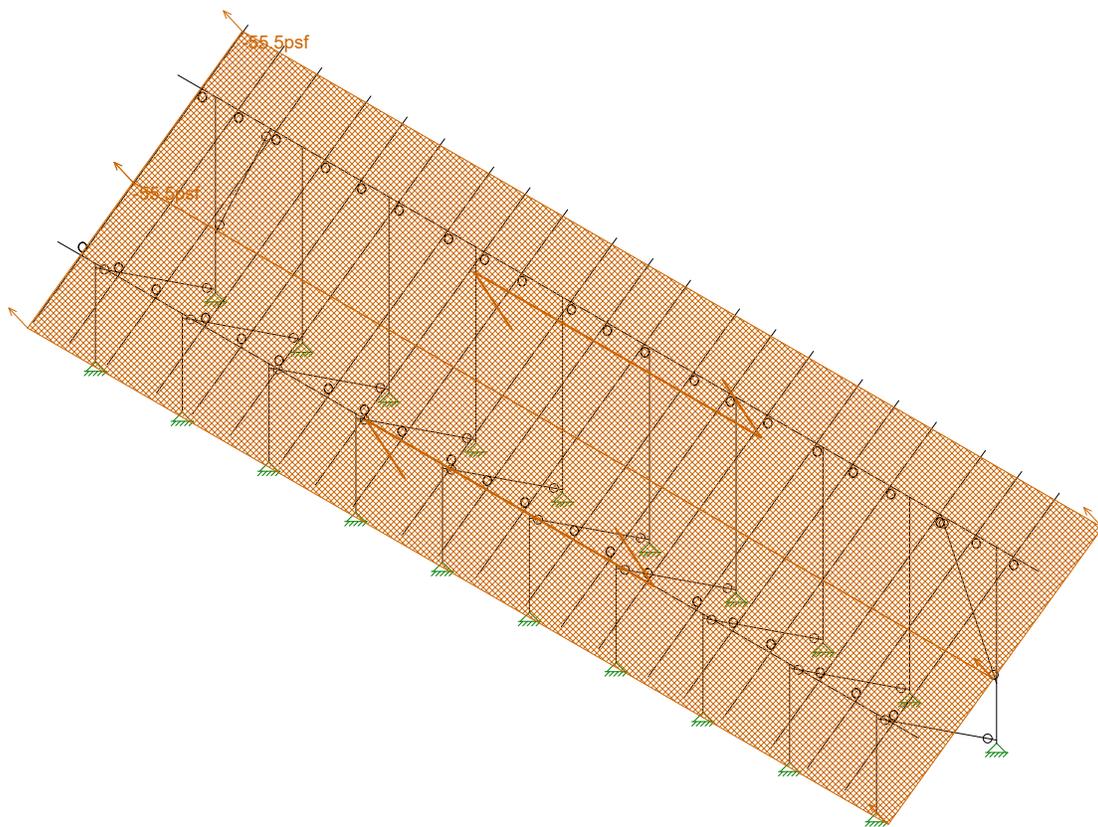
Loads: BLC 2, Solar Panel Weight

Vector Structural Engineeri...	Ground Mount	SK - 6
STB		Apr 6, 2021 at 12:46 PM
U2716.115.191		Sunmodo Sunturf A6 v3 85x45.r3d



Loads: BLC 3, Roof Live/Snow

Vector Structural Engineeri...	Ground Mount	SK - 7
STB		Apr 6, 2021 at 12:46 PM
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Loads: BLC 4, Wind A 0 deg

Vector Structural Engineeri..

STB

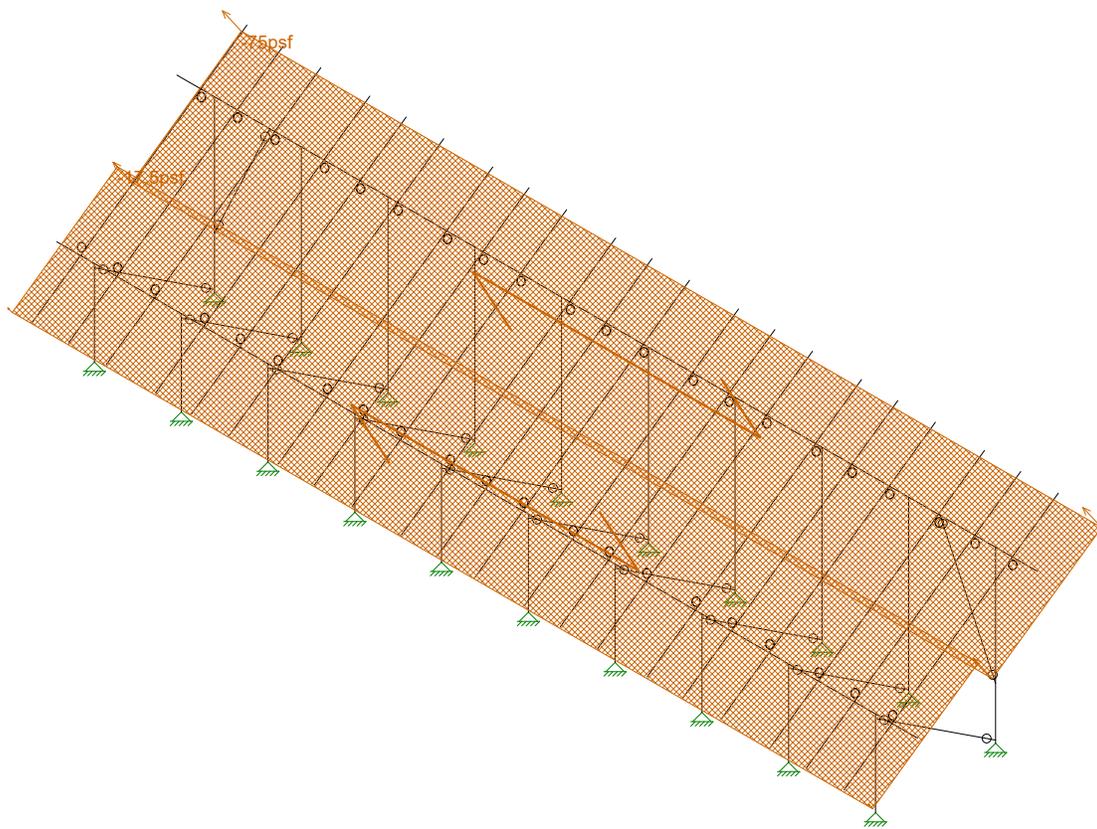
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Ground Mount

SK - 8

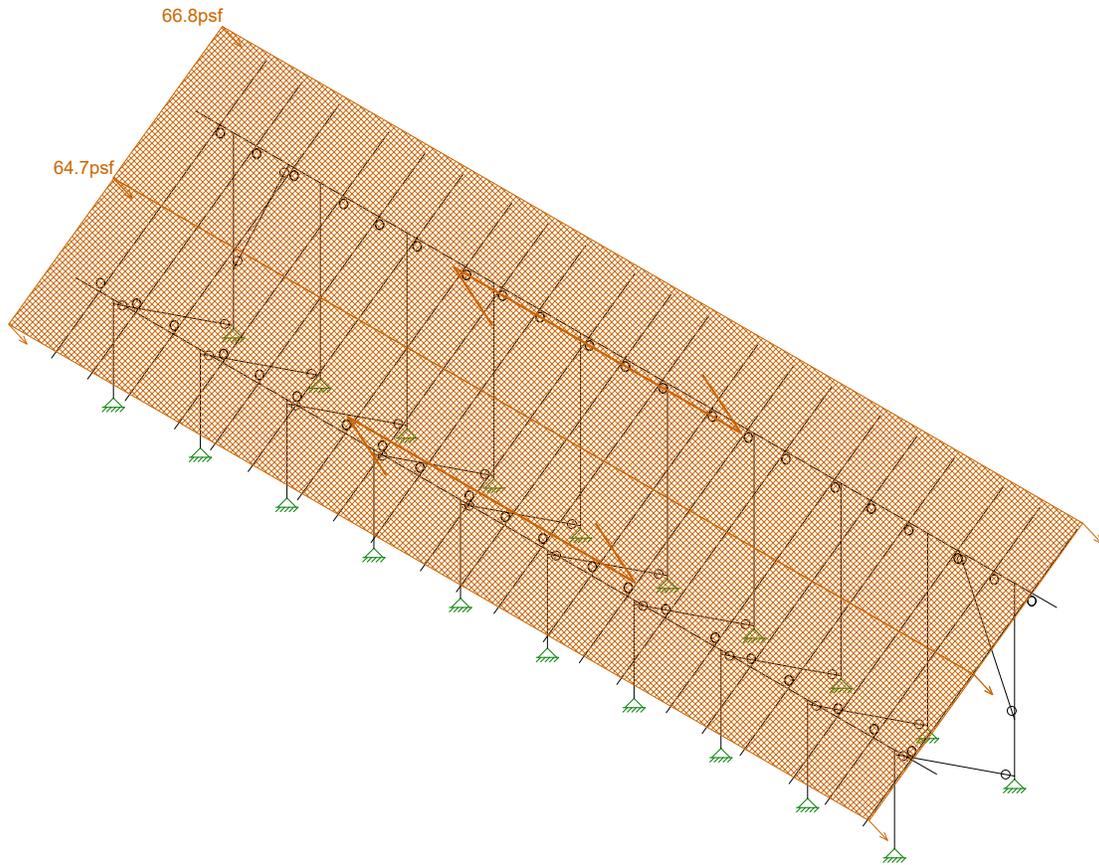
Apr 6, 2021 at 12:46 PM

Sunmodo Sunturf A6 v3 85x45.r3d



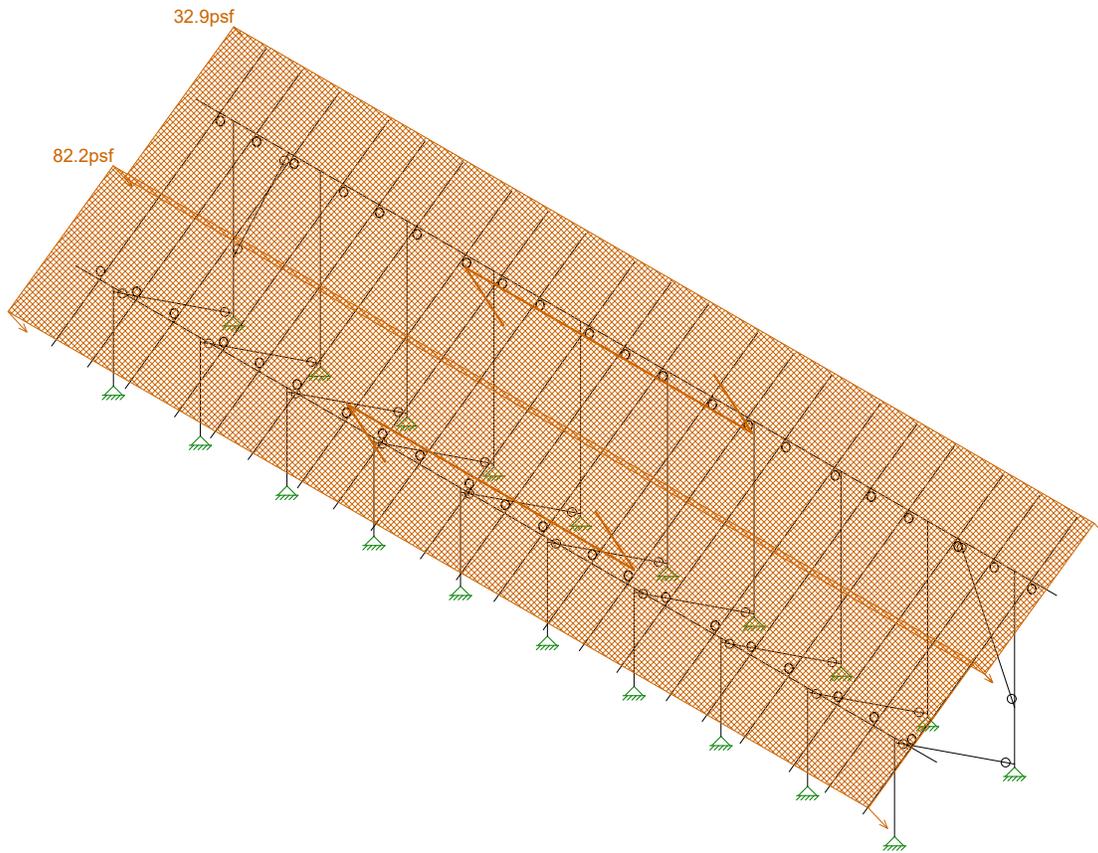
Loads: BLC 5, Wind B 0 deg

Vector Structural Engineeri..	Ground Mount	SK - 9
STB		Apr 6, 2021 at 12:46 PM
U2716.115.191		Sunmodo Sunturf A6 v3 85x45.r3d



Loads: BLC 6, Wind A 180 deg

Vector Structural Engineeri...	Ground Mount	SK - 10
STB		Apr 6, 2021 at 12:46 PM
U2716.115.191		Sunmodo Sunturf A6 v3 85x45.r3d



Loads: BLC 7, Wind B 180 deg

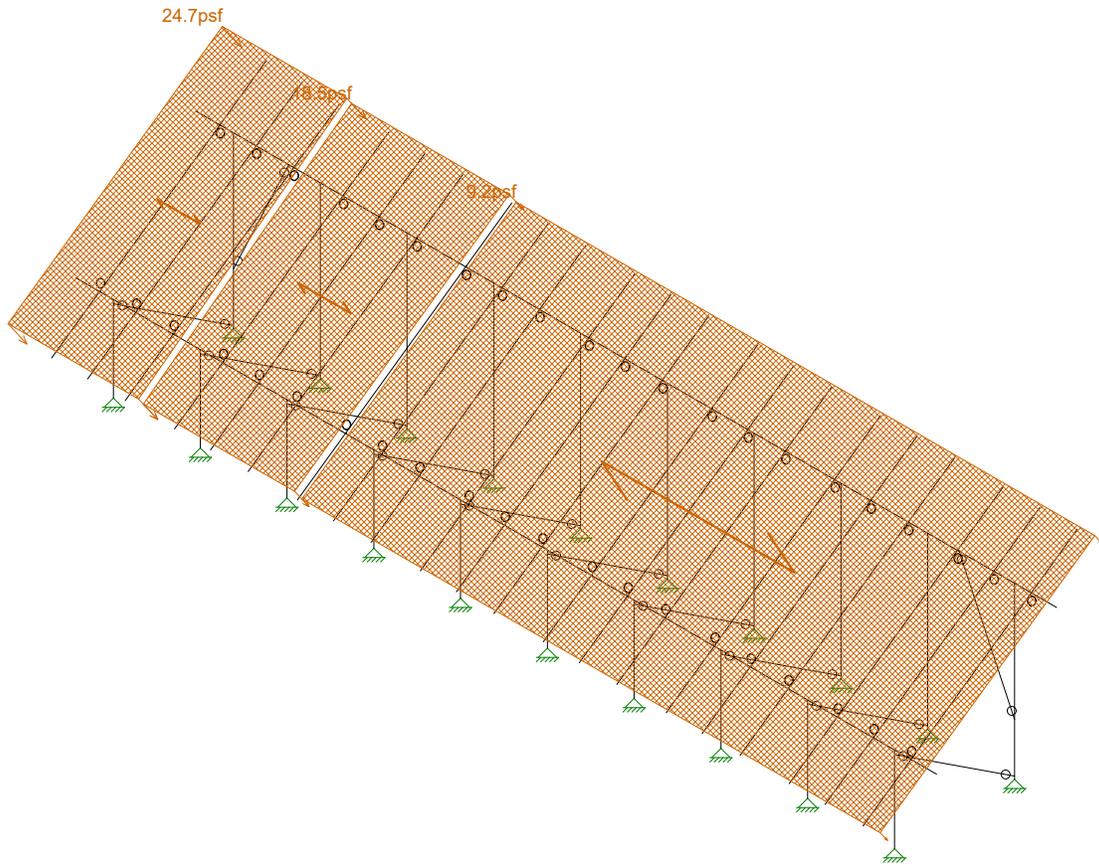
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Ground Mount

SK - 11

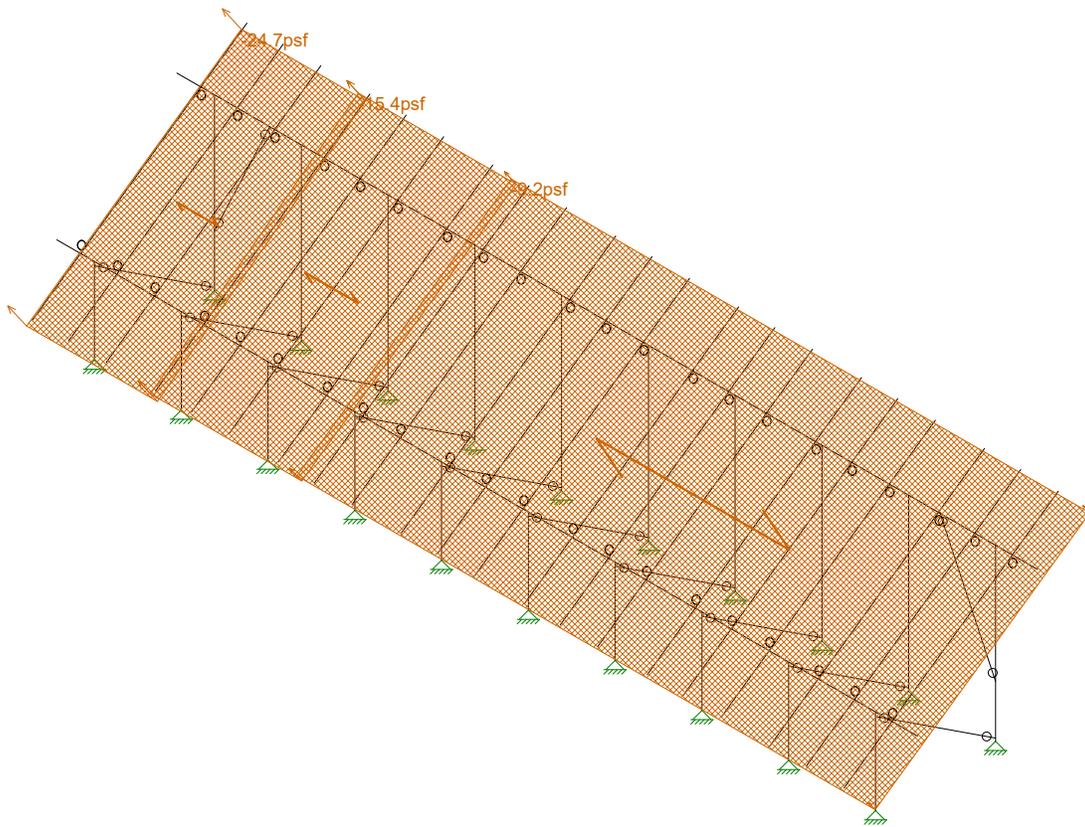
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Sunmodo Sunturf A6 v3 85x45.r3d



Loads: BLC 8, Wind A 90

Vector Structural Engineeri...	Ground Mount	SK - 12
STB		Apr 6, 2021 at 12:46 PM
U2716.115.191		Sunmodo Sunturf A6 v3 85x45.r3d

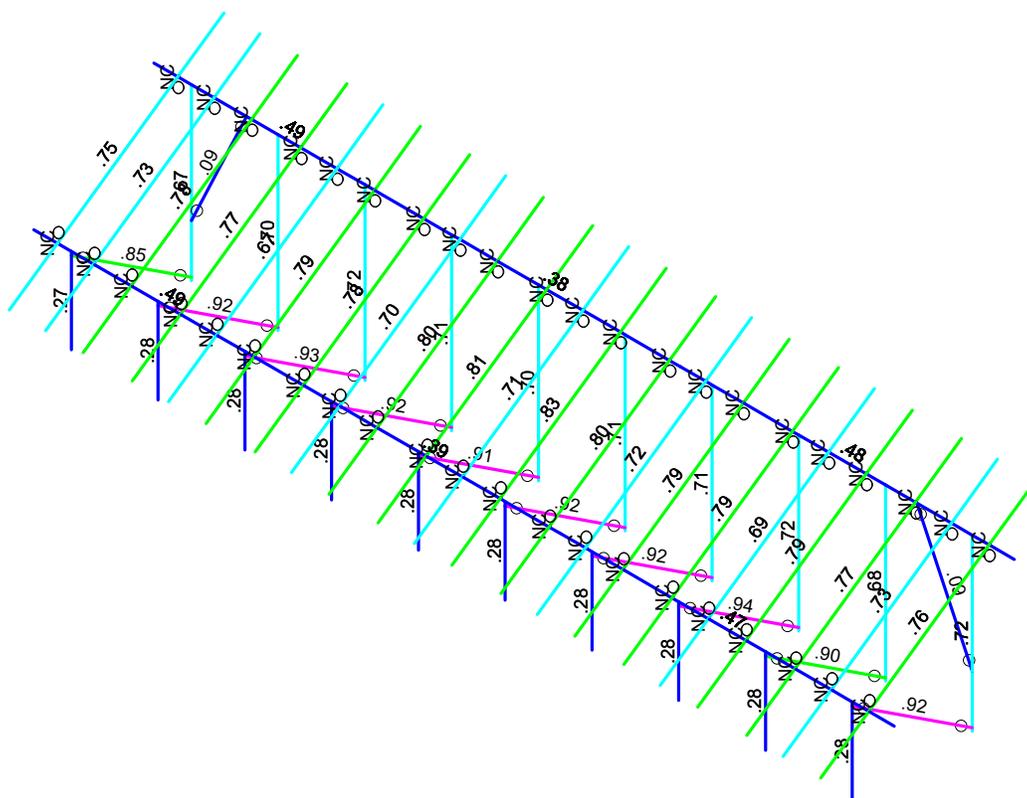


Loads: BLC 9, Wind B 90

Vector Structural Engineeri..	Ground Mount	SK - 13
STB		Apr 6, 2021 at 12:47 PM
U2716.115.191		Sunmodo Sunturf A6 v3 85x45.r3d

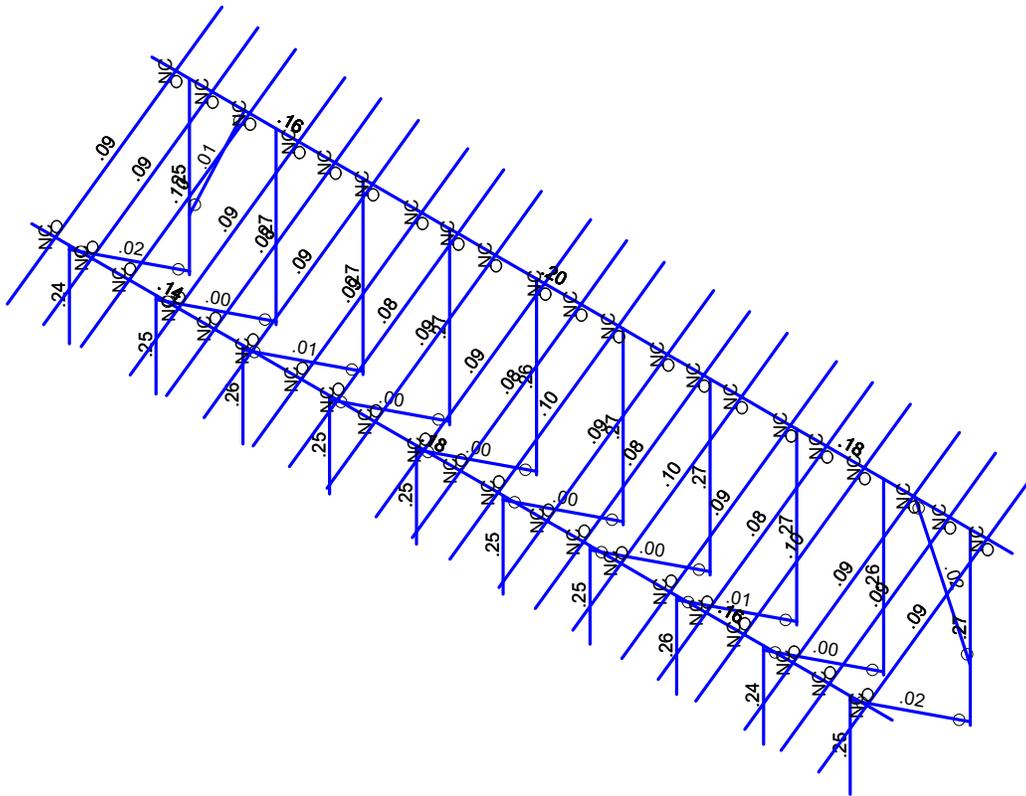
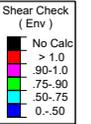


Code Check (Env)	
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Red	> 1.0
Magenta	.90-1.0
Green	.75-.90
Cyan	.50-.75
Blue	0-.50



Member Code Checks Displayed (Enveloped)
Envelope Only Solution

Vector Structural Engineeri...	Ground Mount	SK - 3
STB		Apr 6, 2021 at 12:45 PM
U2716.115.191		Sunmodo Sunturf A6 v3 85x45.r3d



Member Shear Checks Displayed (Enveloped)
Envelope Only Solution

Vector Structural Engineeri...	Ground Mount	SK - 2
STB		Apr 6, 2021 at 12:44 PM
U2716.115.191		Sunmodo Sunturf A6 v3 85x45.r3d

(Global) Model Settings, Continued

Seismic Code	ASCE 7-10
Seismic Base Elevation (in)	15600
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
T X (sec)	Not Entered
T Z (sec)	Not Entered
R X	3
R Z	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	I or II
Drift Cat	Other
Om Z	1
Om X	1
Cd Z	4
Cd X	4
Rho Z	1
Rho X	1

Hot Rolled Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (/1E5 F)	Density[lb/ft^3]	Yield[psi]	Ry	Fu[psi]	Rt
1	A992	29000	11154	.3	.65	490	50000	1.1	65000	1.1
2	A36 Gr.36	29000	11154	.3	.65	490	36000	1.5	58000	1.2
3	A572 Gr.50	29000	11154	.3	.65	490	50000	1.1	65000	1.1
4	A500 Gr.B R...	29000	11154	.3	.65	527	42000	1.4	58000	1.3
5	A500 Gr.B Re...	29000	11154	.3	.65	527	46000	1.4	58000	1.3
6	A53 Gr.B	29000	11154	.3	.65	490	35000	1.6	60000	1.2
7	A1085	29000	11154	.3	.65	490	50000	1.4	65000	1.3

Aluminum Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (...Density[... Table B.4	kt	Ftu[psi]	Fty[psi]	Fcy[psi]	Fsu[psi]	Ct
1	3003-H14	10100	3787.5	.33	1.3 172.8 Table B.4-1	1	19000	16000	13000	12000	141
2	6061-T6	10100	3787.5	.33	1.3 172.8 Table B.4-2	1	38000	35000	35000	24000	141
3	6063-T5	10100	3787.5	.33	1.3 172.8 Table B.4-2	1	22000	16000	16000	13000	141
4	6063-T6	10100	3787.5	.33	1.3 172.8 Table B.4-2	1	30000	25000	25000	19000	141
5	5052-H34	10200	3787.5	.33	1.3 172.8 Table B.4-1	1	34000	26000	24000	20000	141
6	6061-T6 W	10100	3787.5	.33	1.3 172.8 Table B.4-1	1	24000	15000	15000	15000	141
7	6005-T5	10100	3787.5	.33	1.3 172.8 Table B.4-1	1	38000	35000	35000	24000	141

Hot Rolled Steel Section Sets

	Label	Shape	Type	Design List	Material	Design R...	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	Post	Pipe 2.0 A21165	Column	Pipe	A572 Gr.50	Typical	.776	.499	.499	.998
2	Cross Beam	Pipe 2.5 A21168	Beam	Wide Flange	A572 Gr.50	Typical	.947	.907	.907	1.814
3	Diagonal Brace	1.5x1.5x0.083	HBrace	SquareTube	A572 Gr.50	Typical	.47	.158	.158	.236

Aluminum Section Sets

	Label	Shape	Type	Design List	Material	Design R...	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	AL Posts	2.375ODX0.188	Column	Pipe	6005-T5	Typical	1.29	.778	.778	1.54
2	AL Brace	RT1.5x2x0.15625	VBrace	Rectangular Tubes	6005-T5	Typical	.996	.327	.524	.602
3	AL Rails	HR300	Beam	Rectangular Tubes	6005-T5	Typical	.74	.253	.727	.578
4	AL Cross Beam	Cross Rail	Beam	Rectangular Tubes	6005-T5	Typical	1.909	1.97	4.366	4.017

Member Area Loads (BLC 2 : Solar Panel Weight)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N199	N196	Y	A-B	-3

Member Area Loads (BLC 3 : Roof Live/Snow)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N199	N196	PY	A-B	-27

Member Area Loads (BLC 4 : Wind A 0 deg)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	-55.5
2	N198	N201	N199	N196	Perp	A-B	-55.5

Member Area Loads (BLC 5 : Wind B 0 deg)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	-75
2	N198	N201	N199	N196	Perp	A-B	-17.5

Member Area Loads (BLC 6 : Wind A 180 deg)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	66.8
2	N198	N201	N199	N196	Perp	A-B	64.7

Member Area Loads (BLC 7 : Wind B 180 deg)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	32.9
2	N198	N201	N199	N196	Perp	A-B	82.2

Member Area Loads (BLC 8 : Wind A 90)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N203	N202	N196	Perp	A-B	24.7
2	N203	N209	N208	N202	Perp	A-B	18.5
3	N209	N200	N199	N208	Perp	A-B	9.2

Member Area Loads (BLC 9 : Wind B 90)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N203	N202	N196	Perp	A-B	-24.7
2	N203	N209	N208	N202	Perp	A-B	-15.4
3	N209	N200	N199	N208	Perp	A-B	-9.2

Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed	Area(M...Surface...
1	Self Weight	DL		-1.05					
2	Solar Panel Weight	DL							1



Company : Vector Structural Engineering
 Designer : STB
 Job Number : U2716.115.191
 Model Name : Ground Mount

Apr 6, 2021
 12:47 PM
 Checked By: _____

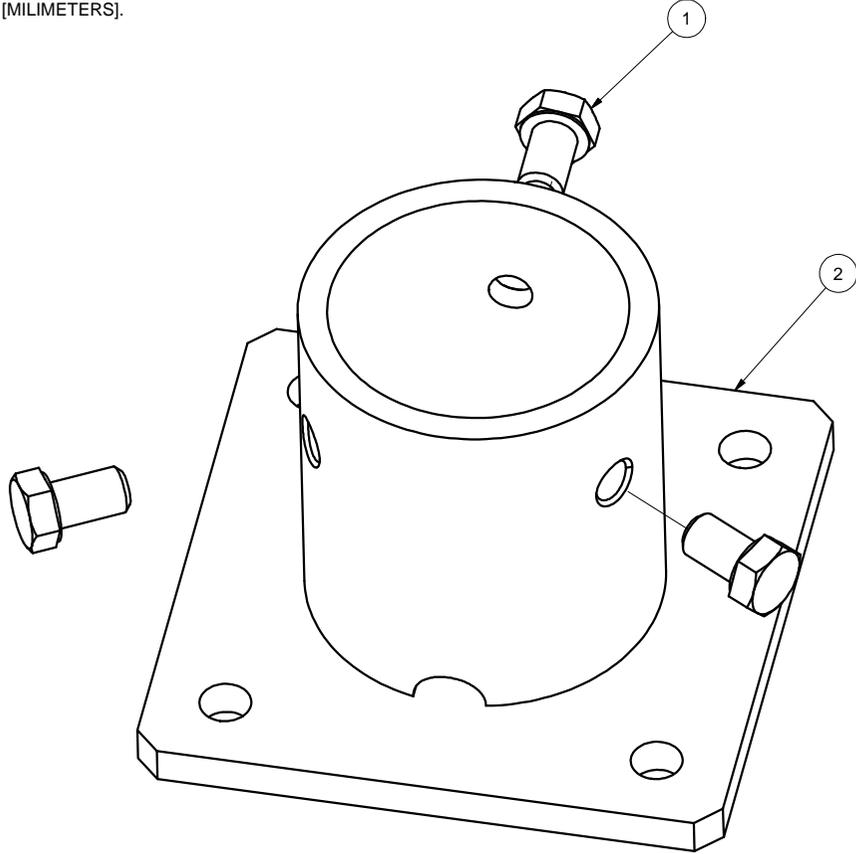
Envelope Joint Reactions (Continued)

Joint	X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [lb-ft]	LC MY [lb-ft]	LC	MZ [lb-ft]	LC			
14	min	-686	2	-2782.706	16	-1524...	3	0	1	0	1	0	1	
15	N122	max	3.582	15	1670.11	10	66.815	3	0	20	0	20	0	20
16		min	-5.68	11	-519.382	17	-78.576	5	0	1	0	1	0	1
17	N133B	max	1.785	11	3220.582	5	1792.2...	5	0	20	0	20	0	20
18		min	-1.077	16	-2747.849	16	-1512...	3	0	1	0	1	0	1
19	N134B	max	2.971	14	1654.738	10	66.758	3	0	20	0	20	0	20
20		min	-3.231	19	-485.752	17	-78.601	5	0	1	0	1	0	1
21	N151	max	2.618	16	3209.397	5	1792.6...	5	0	20	0	20	0	20
22		min	-2.238	11	-2738.929	16	-1513...	3	0	1	0	1	0	1
23	N152	max	2.166	15	1646.327	10	66.734	3	0	20	0	20	0	20
24		min	-8.033	12	-503.545	17	-78.503	5	0	1	0	1	0	1
25	N143A	max	1.683	16	3197.212	5	1777.6...	5	0	20	0	20	0	20
26		min	-1.059	5	-2724.362	16	-1500...	3	0	1	0	1	0	1
27	N144A	max	.28	15	1638.147	10	66.628	3	0	20	0	20	0	20
28		min	-3.887	12	-468.741	17	-78.502	5	0	1	0	1	0	1
29	N149A	max	1.684	11	3262.942	5	1815.7...	17	0	20	0	20	0	20
30		min	-.727	16	-2800.564	16	-1534...	3	0	1	0	1	0	1
31	N150A	max	4.316	12	1679.526	10	67.003	3	0	20	0	20	0	20
32		min	-3.253	15	-523.483	17	-78.77	5	0	1	0	1	0	1
33	N155B	max	52.671	16	3282.153	5	1780.0...	5	0	20	0	20	0	20
34		min	-43.679	11	-2824.397	16	-1503...	3	0	1	0	1	0	1
35	N156B	max	1.583	17	1634.667	10	66.27	3	0	20	0	20	0	20
36		min	-3.262	16	-487.586	17	-77.742	5	0	1	0	1	0	1
37	N161A	max	2.817	16	3036.427	5	1739.4...	5	0	20	0	20	0	20
38		min	-1.993	11	-2533.067	16	-1465...	3	0	1	0	1	0	1
39	N162A	max	1.663	15	1596.473	10	65.692	3	0	20	0	20	0	20
40		min	-4.993	12	-459.083	17	-77.695	5	0	1	0	1	0	1
41	Totals:	max	.009	20	33982.943	11	16930...	5						
42		min	-.01	13	-18339.354	15	-1428...	3						

Envelope AISC 14th(360-10): ASD Steel Code Checks

Member	Shape	Code Check	Loc[in]	LC	Shear	...	Loc[in]	Dir	LC	Pnc/om	[...]	Pnt/om	[lb]	Mnyy/om	Mnzz/om	Cb	Eqn
1	M5	Pipe 2.0 A2...	.268	56.574	5	.236	56.574	5	5	15634.075	23232.186	1397.505	1397.505	1...	H1-1b		
2	M6	Pipe 2.0 A2...	.671	1.223	5	.247	0	5	5	5439	23232.186	1397.505	1397.505	1...	H1-1a		
3	M13	Pipe 2.5 A2...	.488	86.25	12	.143	147....	5	11641.036	28358.413	2081.747	2081.747	1...	H1-1b			
4	M14	Pipe 2.5 A2...	.491	86.25	11	.160	147....	11	11641.036	28358.413	2081.747	2081.747	1...	H1-1b			
5	M15	1.5x1.5x0.083	.853	51.711	5	.023	99.285	y	11	2408.78	14085.15	624.421	624.421	1...	H1-1a		
6	M80	Pipe 2.0 A2...	.280	56.574	5	.249	56.574	5	5	15634.075	23232.186	1397.505	1397.505	1...	H1-1b		
7	M81	Pipe 2.0 A2...	.703	1.223	5	.264	0	5	5	5439	23232.186	1397.505	1397.505	1...	H1-1a		
8	M82	1.5x1.5x0.083	.913	51.711	5	.004	99.285	y	10	2408.78	14085.15	624.421	624.421	1...	H1-1a		
9	M50	Pipe 2.0 A2...	.278	56.574	5	.252	56.574	5	5	15634.075	23232.186	1397.505	1397.505	1...	H1-1b		
10	M51	Pipe 2.0 A2...	.696	1.223	5	.267	0	5	5	5439	23232.186	1397.505	1397.505	1...	H1-1a		
11	M52	1.5x1.5x0.083	.922	51.711	5	.005	0	y	8	2408.78	14085.15	624.421	624.421	1...	H1-1a		
12	M56A	Pipe 2.0 A2...	.282	56.574	5	.255	56.574	5	5	15634.075	23232.186	1397.505	1397.505	1...	H1-1b		
13	M57A	Pipe 2.0 A2...	.718	1.223	5	.269	0	5	5	5439	23232.186	1397.505	1397.505	1...	H1-1a		
14	M58A	1.5x1.5x0.083	.931	51.711	5	.006	99.285	y	5	2408.78	14085.15	624.421	624.421	1...	H1-1a		
15	M68	Pipe 2.0 A2...	.282	56.574	5	.253	56.574	5	5	15634.075	23232.186	1397.505	1397.505	1...	H1-1b		
16	M69	Pipe 2.0 A2...	.713	1.223	5	.267	0	5	5	5439	23232.186	1397.505	1397.505	1...	H1-1a		
17	M70	1.5x1.5x0.083	.925	51.711	5	.004	99.285	y	7	2408.78	14085.15	624.421	624.421	1...	H1-1a		
18	M68A	Pipe 2.5 A2...	.391	82.5	12	.176	140....	5	11641.036	28358.413	2081.747	2081.747	2...	H1-1b			
19	M69A	Pipe 2.5 A2...	.474	141....	12	.158	81.813	5	11641.036	28358.413	2081.747	2081.747	2...	H1-1b			
20	M70A	Pipe 2.5 A2...	.382	82.5	11	.196	140....	11	11641.036	28358.413	2081.747	2081.747	2...	H1-1b			
21	M71	Pipe 2.5 A2...	.480	202....	11	.176	21.656	11	11641.036	28358.413	2081.747	2081.747	2...	H1-1b			
22	M73	Pipe 2.0 A2...	.281	56.574	5	.253	56.574	5	5	15634.075	23232.186	1397.505	1397.505	1...	H1-1b		
23	M74	Pipe 2.0 A2...	.711	1.223	5	.267	0	5	5	5439	23232.186	1397.505	1397.505	1...	H1-1a		

NOTES: UNLESS OTHERWISE SPECIFIED
 1. DIMENSIONS SHOWN ARE INCHES [MILLIMETERS].



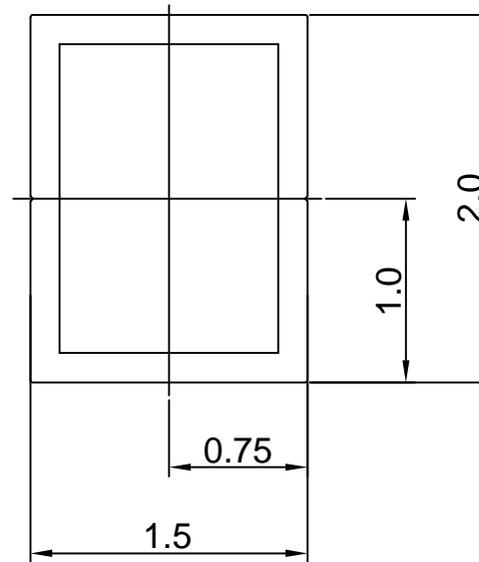
REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	10/20/2016

2	A21120-001	2" PIPE BASE	1
1	B15018-011	HEX CAP SCREW 3/8-16 X 5/8	3
ITEM	PART NUMBER	DESCRIPTION	QTY
MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		Sunmodo Corp. 1905 E 5TH STREET, STE A, VANCOUVER, WA 98661	
Tolerances		TITLE	
X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] X.X ±0.039 [1.0mm] Unless otherwise spec'd		2" PIPE BASE KIT	
DRAWN BY		DATE	
LWF		10/20/2016	
CHECKED BY		DRAWING NUMBER	
		B K10268-001	
APPROVALS		SCALE: NONE SHEET 1 of 1	

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NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERAIL: ALUMINUM 6005-T5.
FINISH: CLEAR ANODIZED 10 μ m THICK.
3. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.



Section properties:

Weight: 1.156 lbs/ft

Area: 0.992 in²

Perimeter: 12.601 in

Bounding Box: X: -1.000,1.000

Y: -0.750, 0.750

Centroid:(0.000,0.000)

Moments of Inertia(in⁴): I_x=0.506,I_y=0.322

Section modulus in bending(in³): W_x=0.675,W_y=0.322

Radii of Gyration: X: 0.714, Y: 0.570

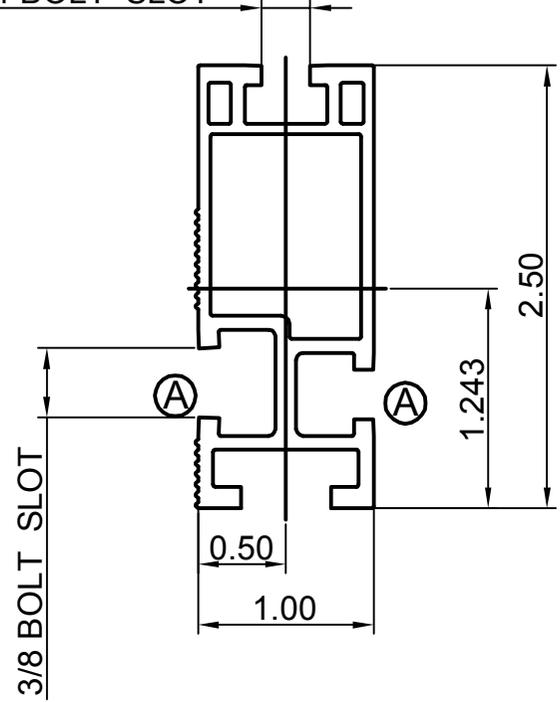
MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS			
All Dimensions in inches [millimeters]			
Tolerances			
X.XXX ± 0.01 [0.25mm]	Break all sharp edges		
X.XX ± 0.02 [0.50mm]	.010-.020 unless		
X.X ± 0.039 [1.0mm]	otherwise specified.		
DRAWN BY		DATE	
zcg		03/12/2014	
CHECKED BY			
APPROVALS			
		1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661	
		TITLE	
		1.5X2 AL TUBE BRACE EXTRUSION	
		DRAWING NUMBER	
		A20164	
		SCALE: NONE	
		SHEET 1 of 1	

REVISIONS			
REV	DESCRIPTION	BY	DATE
A	ADDED BOTTOM CHANNEL & CHANGED ONE 3/8 CHANNEL TO 1/4	zcg	02/21/2013

NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERAIL: ALUMINUM 6005-T5.
FINISH: CLEAR ANODIZED 15 μ m THICK.
3. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.

2X 1/4 BOLT SLOT



Section properties:

Weight: 0.850 lbs/ft
 Area: 0.723 in²
 Perimeter: 17.325 in
 Bounding Box: X: -0.500,0.500
 Y: -1.243,1.257

Centroid:(0.000,0.000)
 Moments of Inertia(in⁴): I_x=0.486,I_y=0.095
 Section modulus in bending(in³): W_x=0.387,W_y=0.190
 Radii of Gyration: X: 0.820, Y: 0.363

MATERIAL SEE NOTES	
Third Angle Projection:	
GENERAL SPECIFICATIONS All Dimensions in inches [millimeters]	
Tolerances	
X,XXX ± 0.01 [0.25mm]	Break all sharp edges
X,XX ± 0.02 [0.50mm]	.010-.020 unless
X,X ± 0.039 [1.0mm]	otherwise specified.
Unless otherwise spec'd	
DRAWN BY zcg	DATE 02/21/2013
CHECKED BY	
APPROVALS	

Sunmodo Corp.	
1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661	
TITLE HELIO STANDARD RAIL	
DRAWING NUMBER B A20144	
SCALE: NONE	SHEET 1 of 1

4

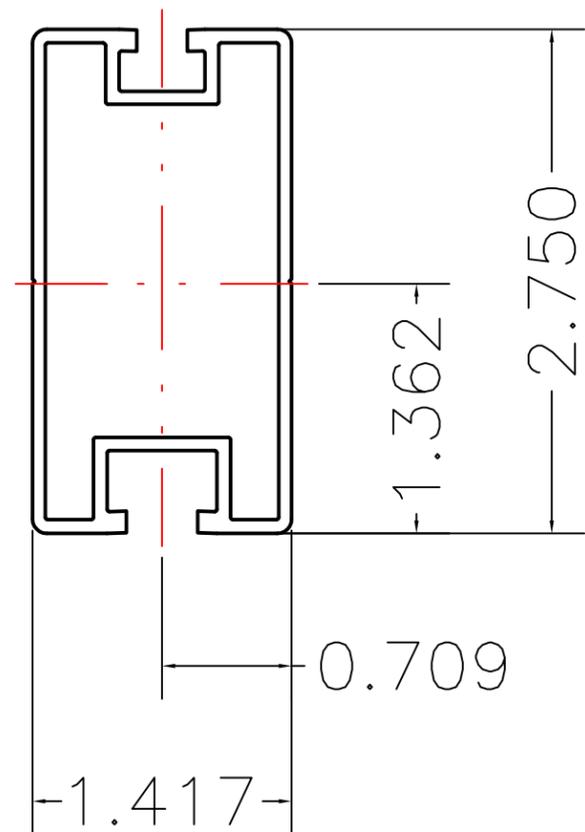
3

2

1

NOTES: UNLESS OTHERWISE SPECIFIED

- 1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
- 2. MATERIAL: 6005-T5.
FINISH: CLEAR ANODIZED 10um THICK.
- 3. THE UNSPECIFIED RADII ARE .02" MAX.
- 4. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.



Section properties:

Weight: 0.862 lbs/ft
 Area: 0.736 in²
 Perimeter: 19.824 in
 Bounding Box: X: -0.709,0.709
 Y: -1.362,1.388
 Centroid:(0.000,0.000)
 Moments of Inertia(in⁴): I_x=0.727,I_y=0.214
 Section modulus in bending(in³): W_x=0.524,W_y=0.302
 Radii of Gyration: X: 0.994, Y: 0.539

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MATERIAL SEE NOTES		SunModo Corp. 14800 NE 65TH STREET, VANCOUVER WA 98682	
Third Angle Projection:			
GENERAL SPECIFICATIONS All Dimensions in inches [millimeters]		TITLE RAIL, HR300 (SUNRAY), EXTRUSION	
Tolerances X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] Break all sharp edges X.X ±0.039 [1.0mm] .010-.020 unless otherwise specified. Unless otherwise spec'd			
DRAWN BY KYY	DATE 01/18/2018	B	DRAWING NUMBER A20288
CHECKED BY			
APPROVALS		SCALE: NONE	SHEET 1 of 1

4

3

2

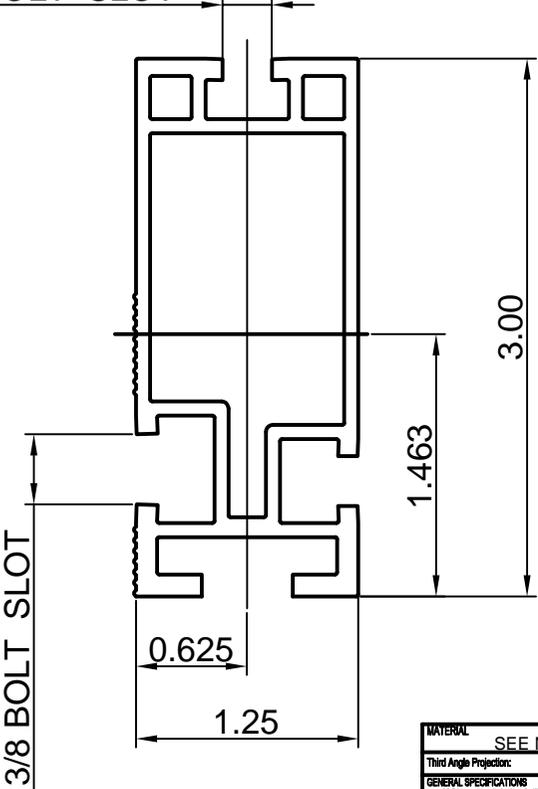
1

REV	DESCRIPTON	BY	DATE
A	0.44 WAS 0.41, 0.44 WAS 0.33	LWF	11/30/2015

NOTES: UNLESS OTHERWISE SPECIFIED

- 1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
- 2. MATERIAL: ALUMINUM 6005-T5.
FINISH: CLEAR ANODIZED 15 μm THICK.
- 3. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.

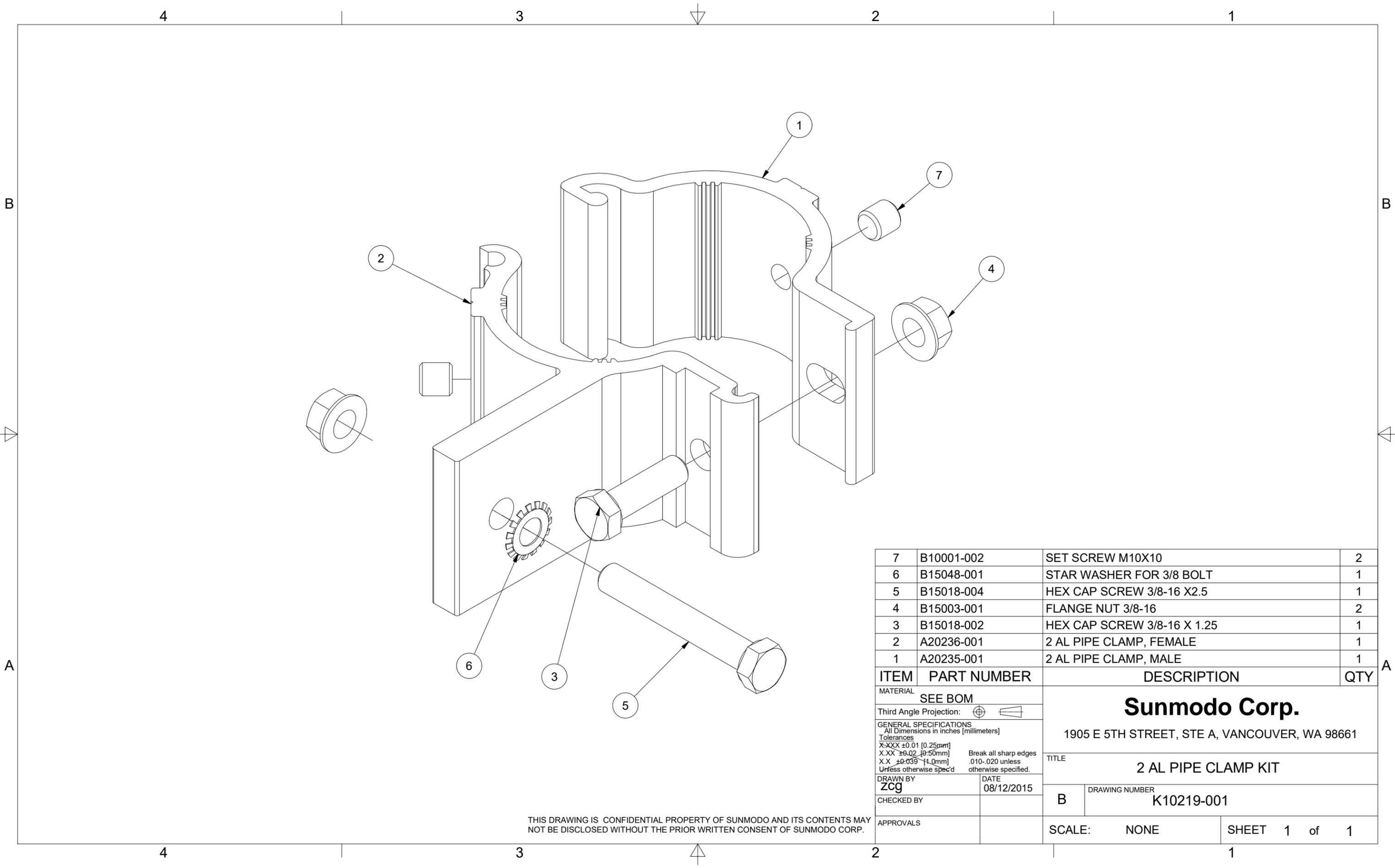
2X 1/4 BOLT SLOT



Section properties:

Weight: 1.151 lbs/ft
 Area: 0.980 in²
 Perimeter: 22.104 in
 Bounding Box: X: -0.625,0.625
 Y: -1.463,1.537
 Centroid:(0.000,0.000)
 Moments of Inertia(in⁴): Ix=1.047,Iy=0.207
 Section modulus in bending(in³): Wx=0.681,Wy=0.331
 Radii of Gyration: X: 1.034, Y: 0.460

MATERIAL		SEE NOTES	
Third Angle Projection			
GENERAL SPECIFICATIONS			
All Dimensions in Inches [millimeters]			
Tolerances			
XXX ±0.01 (0.25mm)			
XX ±0.02 (0.50mm)			
X ±0.03 (1.0mm)			
Unless otherwise specified.			
DRAWN BY		DATE	
ZCJ		02/21/2013	
CHECKED BY		DATE	
APPROVALS		DATE	
Sunmodo Corp.		1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661	
TITLE		HELIO HEAVY RAIL	
DRAWING NUMBER		A20145	
SCALE:		NONE	
SHEET		1 of 1	



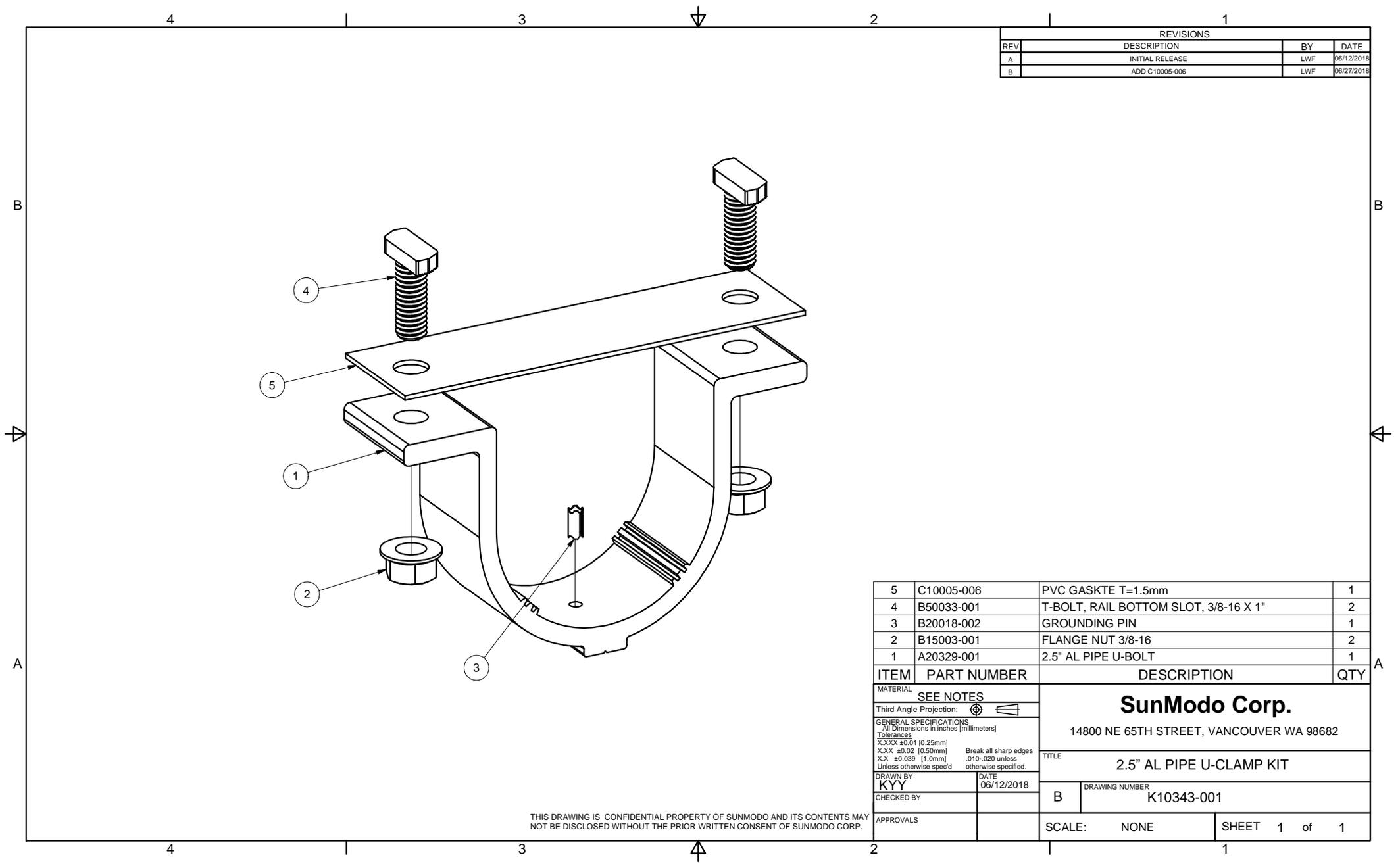
7	B10001-002	SET SCREW M10X10	2
6	B15048-001	STAR WASHER FOR 3/8 BOLT	1
5	B15018-004	HEX CAP SCREW 3/8-16 X2.5	1
4	B15003-001	FLANGE NUT 3/8-16	2
3	B15018-002	HEX CAP SCREW 3/8-16 X 1.25	1
2	A20236-001	2 AL PIPE CLAMP, FEMALE	1
1	A20235-001	2 AL PIPE CLAMP, MALE	1

ITEM	PART NUMBER	DESCRIPTION	QTY
MATERIAL		SEE BOM	
Third Angle Projection:			
GENERAL SPECIFICATIONS All Dimensions in inches (millimeters)			
Tolerances			
X.XXX ±0.01 [0.25mm]		Break all sharp edges	
X.XX ±0.02 [0.50mm]		.010-.020 unless	
X.X ±0.039 [1.0mm]		otherwise specified.	
DRAWN BY		DATE	
zcg		08/12/2015	
CHECKED BY		B	
APPROVALS		DRAWING NUMBER	
		K10219-001	
SCALE:		NONE	SHEET 1 of 1

Sunmodo Corp.
1905 E 5TH STREET, STE A, VANCOUVER, WA 98661

TITLE
2 AL PIPE CLAMP KIT

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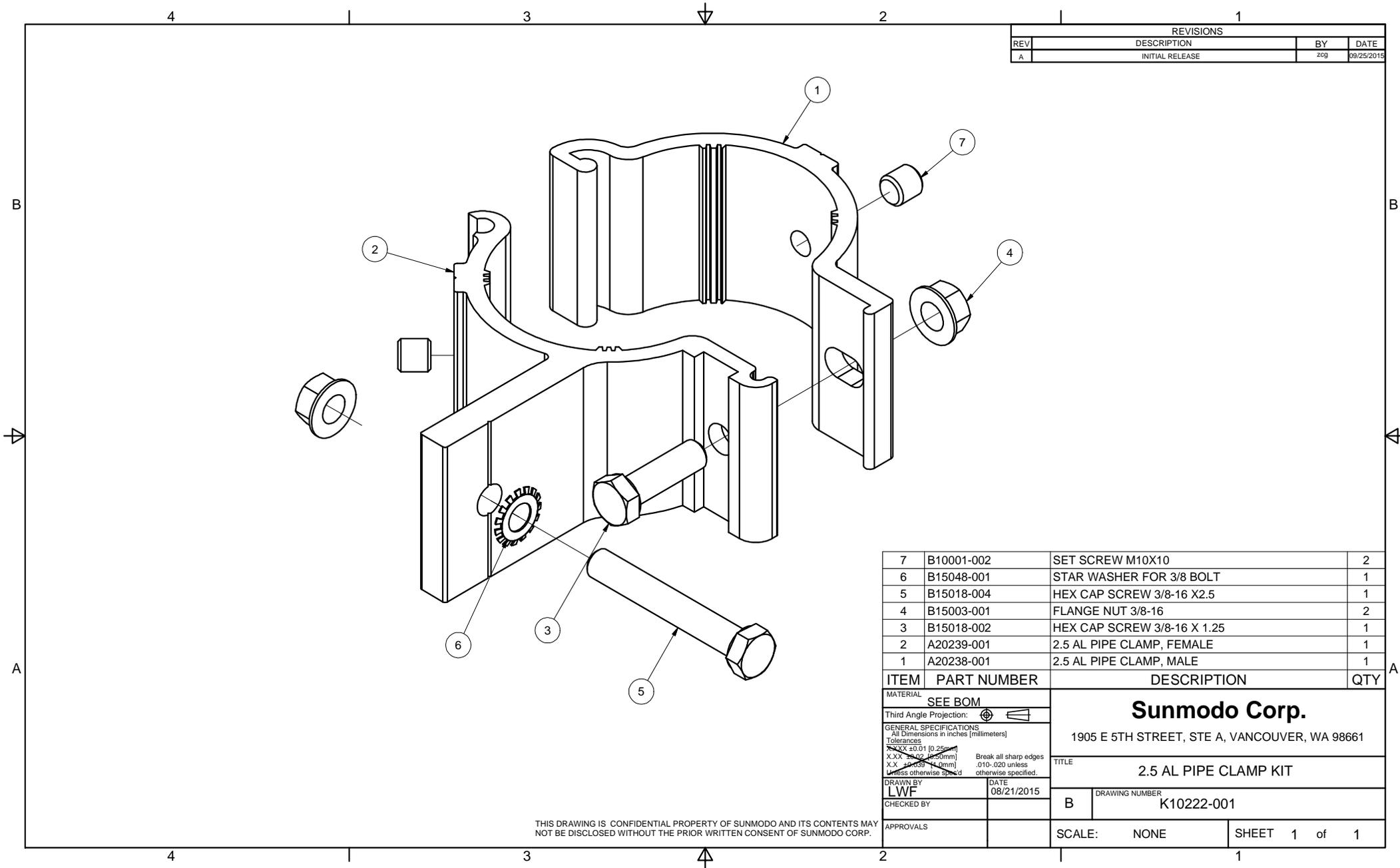


REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	06/12/2018
B	ADD C10005-006	LWF	06/27/2018

ITEM	PART NUMBER	DESCRIPTION	QTY
5	C10005-006	PVC GASKTE T=1.5mm	1
4	B50033-001	T-BOLT, RAIL BOTTOM SLOT, 3/8-16 X 1"	2
3	B20018-002	GROUNDING PIN	1
2	B15003-001	FLANGE NUT 3/8-16	2
1	A20329-001	2.5" AL PIPE U-BOLT	1

MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		<p style="text-align: center;">SunModo Corp.</p> <p style="text-align: center;">14800 NE 65TH STREET, VANCOUVER WA 98682</p>	
Tolerances		TITLE	
X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] X.X ±0.039 [1.0mm] Unless otherwise spec'd		2.5" AL PIPE U-CLAMP KIT	
DRAWN BY	DATE	DRAWING NUMBER	
KYY	06/12/2018	B K10343-001	
CHECKED BY		SCALE: NONE	
APPROVALS		SHEET 1 of 1	

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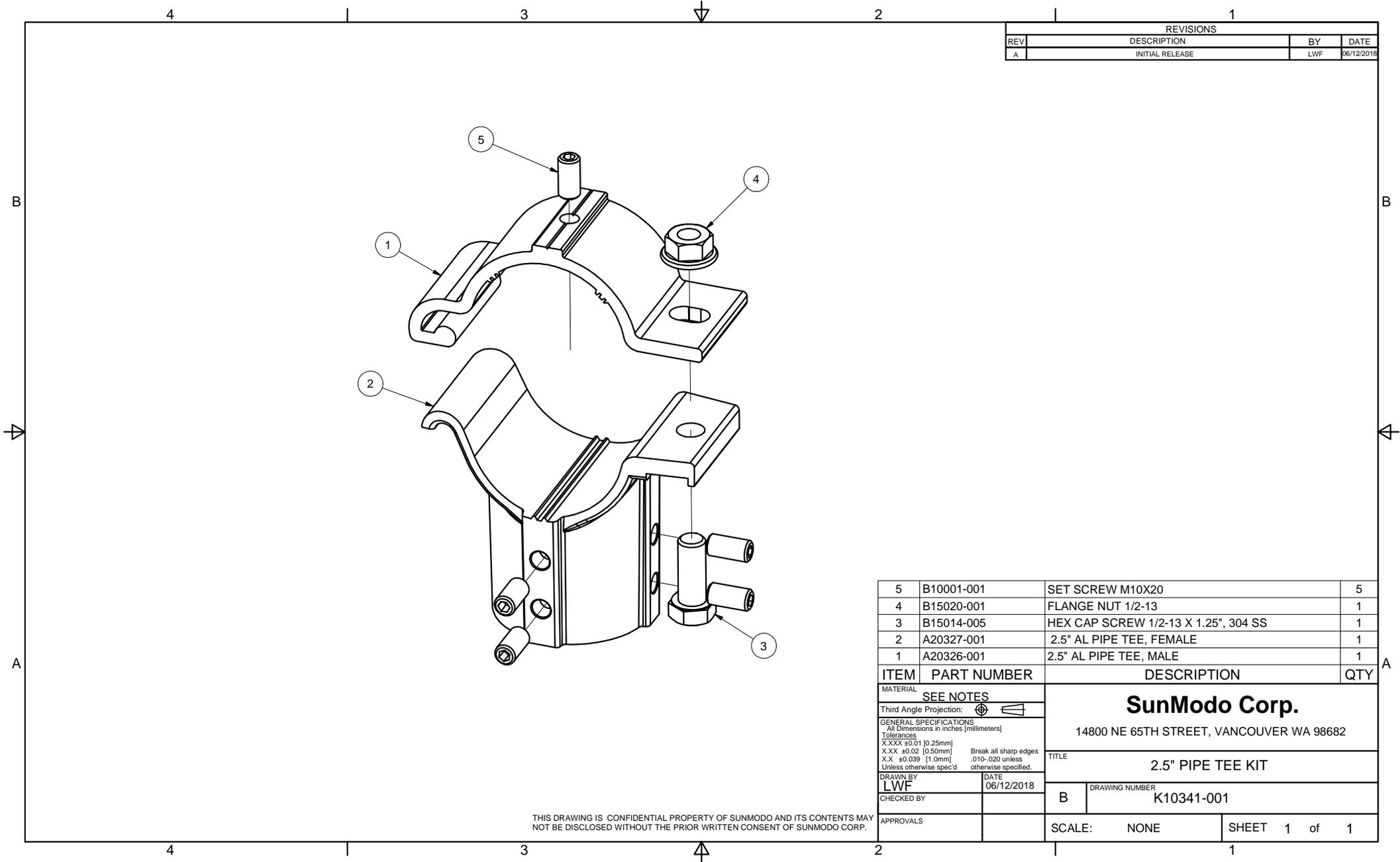


REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	zcg	09/25/2015

ITEM	PART NUMBER	DESCRIPTION	QTY
7	B10001-002	SET SCREW M10X10	2
6	B15048-001	STAR WASHER FOR 3/8 BOLT	1
5	B15018-004	HEX CAP SCREW 3/8-16 X2.5	1
4	B15003-001	FLANGE NUT 3/8-16	2
3	B15018-002	HEX CAP SCREW 3/8-16 X 1.25	1
2	A20239-001	2.5 AL PIPE CLAMP, FEMALE	1
1	A20238-001	2.5 AL PIPE CLAMP, MALE	1

MATERIAL		SEE BOM	
Third Angle Projection:			
GENERAL SPECIFICATIONS All Dimensions in inches [millimeters] Tolerances X.XX ±0.01 (0.25mm) X.X ±0.02 (0.5mm) X.X ±0.03 (0.75mm) Unless otherwise specified, Break all sharp edges .010-.020 unless otherwise specified.			
DRAWN BY	DATE	TITLE	
LWF	08/21/2015	2.5 AL PIPE CLAMP KIT	
CHECKED BY		DRAWING NUMBER	
		B	K10222-001
APPROVALS		SCALE:	NONE
		SHEET	1 of 1

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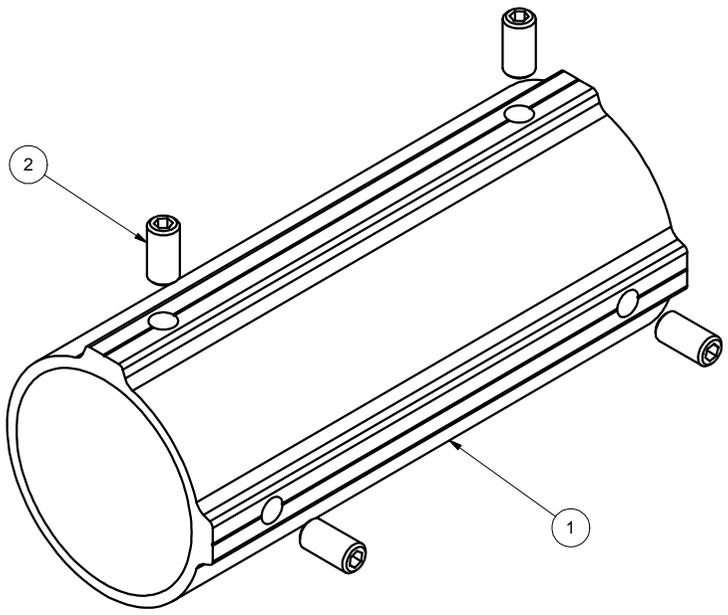
REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	06/12/2018

ITEM	PART NUMBER	DESCRIPTION	QTY
5	B10001-001	SET SCREW M10X20	5
4	B15020-001	FLANGE NUT 1/2-13	1
3	B15014-005	HEX CAP SCREW 1/2-13 X 1.25", 304 SS	1
2	A20327-001	2.5" AL PIPE TEE, FEMALE	1
1	A20326-001	2.5" AL PIPE TEE, MALE	1

MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		SunModo Corp. 14800 NE 65TH STREET, VANCOUVER WA 98682	
Tolerances		TITLE	
X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] X.X ±0.039 [1.0mm] Unless otherwise spec'd		2.5" PIPE TEE KIT	
DRAWN BY		DATE	
LWF		06/12/2018	
CHECKED BY		DRAWING NUMBER	
		B K10341-001	
APPROVALS		SCALE: NONE SHEET 1 of 1	

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REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	06/12/2018



2	B10001-001	SET SCREW M10X20	4
1	A20328-001	2.5" PIPE SPLICE	1
ITEM	PART NUMBER	DESCRIPTION	QTY
MATERIAL		<p align="center">SunModo Corp.</p> <p align="center">14800 NE 65TH STREET, VANCOUVER WA 98682</p>	
SEE NOTES			
Third Angle Projection:		TITLE	
GENERAL SPECIFICATIONS All Dimensions in inches [millimeters] Tolerances X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] X.X ±0.039 [1.0mm] Unless otherwise spec'd		2.5" PIPE SPLICE KIT	
DRAWN BY	DATE	DRAWING NUMBER	
LWF	06/12/2018	B K10342-001	
CHECKED BY		SCALE: NONE SHEET 1 of 1	
APPROVALS			

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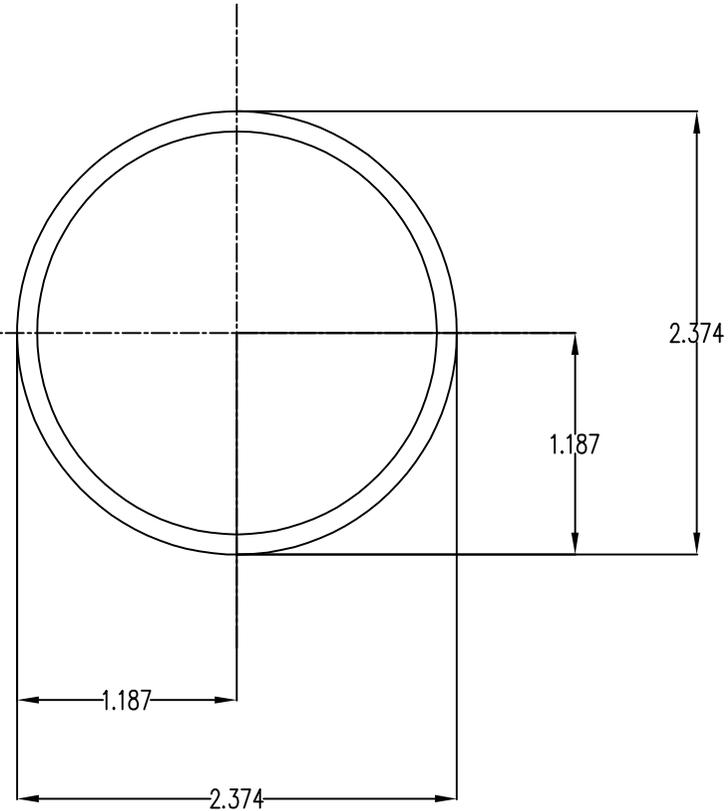
NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERIAL:HIGH STRENGTH STEEL PIPE OR TUBE.
3. FINISH: HOT DIP GALVANIZE PER ASTM A123 / A123M - 02.

MINIMUM 50 KSI YIELD STRESS.

4. BREAK ALL BURRS AND SHARP EDGES.

5. ALL WELDING MUST BE IN COMPLIANCE WITH AWS CODE D1;1.



Section properties:

Weight: 2.641 lbs/ft

Area: 0.776 in²

Perimeter: 14.238 in

Bounding Box: X: -1.187,1.187

Y: -1.187,1.187

Centroid:(0.000,0.000)

Moments of Inertia(in⁴): I_x=0.499,I_y=0.499

Section modulus in bending(in³): W_x=0.420,W_y=0.420

Radii of Gyration: X: 0.802, Y: 0.802

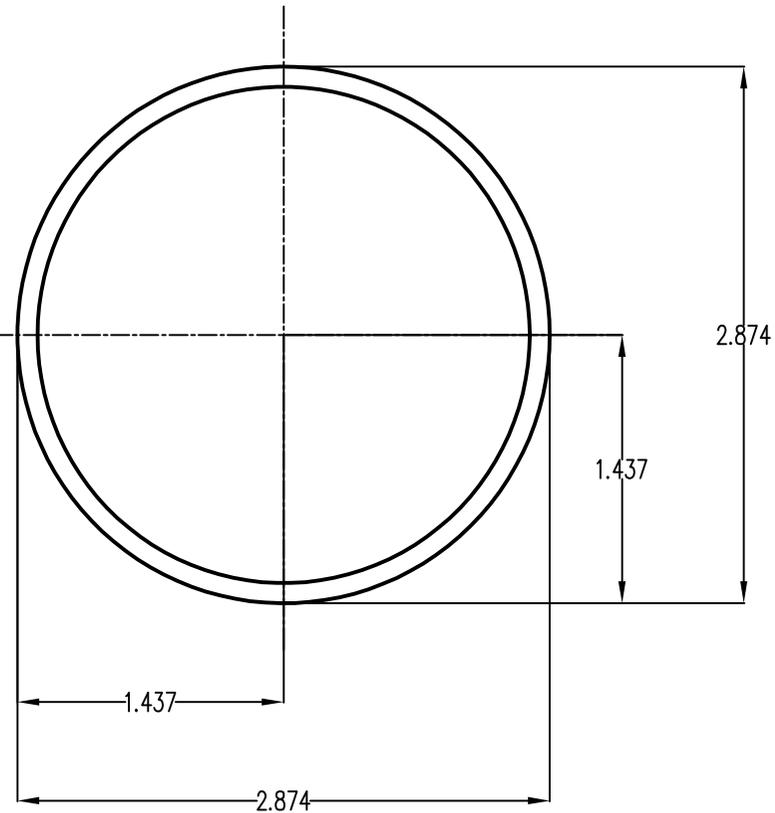
MATERIAL		SEE NOTES		Sunmodo Corp. 14800 NE 85TH STREET, VANCOUVER WA 98682	
Third Angle Projection:					
GENERAL SPECIFICATIONS				TITLE	
All Dimensions in Inches (millimeters)				PIPE, HSS, 2.375" OD X 12 GAUGE,L=XXX	
Tolerances				Break all sharp edges .010-.020 unless otherwise specified.	
XXX ±0.01 (0.25mm)					
XX ±0.02 (0.50mm)					
X ±0.05 (1.25mm)					
Unless otherwise specified					
DRAWN BY		DATE		DRAWING NUMBER	
LWF		04/03/2019		A21165	
CHECKED BY				B	
APPROVALS				SCALE: NONE	
				SHEET 1 of 1	

NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERAIL:HIGH STRENGTH STEEL PIPE OR TUBE.
3. FINISH: HOT DIP GALVANIZE PER ASTM A123 / A123M - 02.

MINIMUM 50 KSI YIELD STRESS.

4. BREAK ALL BURRS AND SHARP EDGES.
5. ALL WELDING MUST BE IN COMPLIANCE WITH AWS CODE D1;1.



Section properties:

Weight: 3.201 lbs/ft

Area: 0.941 in²

Perimeter: 17.378 in

Bounding Box: X: -1.437,1.437

Y: -1.437,1.437

Centroid:(0.000,0.000)

Moments of Inertia(in⁴): Ix=0.901,Iy=0.901

Section modulus in bending(in³): Wx=0.627,Wy=0.627

Radii of Gyration: X: 0.979, Y: 0.979

MATERIAL		SEE NOTES		Sunmodo Corp. 14800 NE 85TH STREET, VANCOUVER WA 98682	
Third Angle Projection:					
GENERAL SPECIFICATIONS All Dimensions in Inches (millimeters)				TITLE	
Tolerances XXX ±0.01 (0.25mm) XX ±0.02 (0.50mm) X ±0.030 (1.0mm) Unless otherwise specified				PIPE, HSS, 2.875" OD X 12 GAUGE,L=XXX	
DRAWN BY LWF		DATE 04/03/2019		DRAWING NUMBER A21168	
CHECKED BY				B	
APPROVALS				SCALE: NONE	
				SHEET 1 of 1	