



Project Number: U2716-103-191

August 1, 2019

Sunmodo  
14800 NE 65<sup>th</sup> Street  
Vancouver, WA 98682

**REFERENCE: Florida Ground Mount D1  
Ground Mount PV Array Installation**

To Whom It May Concern:

Per request of Sunmodo, we have been asked to prepare the structural design of a ground-mounted PV solar array system with several foundation options as shown in the attached calculations. The adopted building code in this jurisdiction is the Florida Building Code, 2017 Edition (2015 IBC). Vector Structural Engineering requires that we review each site specific install, and we are not liable for installs at site specific locations we have not reviewed. The following design parameters are used in our analysis:

- Minimum Design Loads for Buildings and Other Structures (ASCE 7-10)
- Design wind speed for risk category I structures: 170 mph
- Wind exposure: C
- Ground snow load: 0 psf
- The ground screws and helical piers must be tested to 1.5 times uplift and 2.0 times lateral reactions found in the table below. A minimum of one ground screw or helical pier must be tested.

Load (ASD)	Value (lbs)	Factor of Safety	Test Value (lbs)
UPLIFT	2640	1.5	3960
LATERAL	1180	2	2360

Foundation concrete shall have a minimum compressive strength of 2500 psi at 28 days. Cement for all concrete shall be Type I or II with a water/cement ratio of 0.50. Maximum aggregate size shall be 3/4". No special inspection of concrete strength is required.

Footings are designed based on an allowable soil bearing pressure of 1500 psf an allowable skin friction of 250 psf, an allowable lateral bearing pressure of 150 pcf, and a coefficient of friction of 0.3. Vector Structural Engineering strongly recommends independent soils testing be performed by a licensed geotechnical engineer to verify the assumed soil parameters.

All ground mounts are to be installed per manufacturer’s recommendations. The use of solar panel support span tables provided by the manufacturer is allowed only where the site conditions and solar panel configuration match the description of the span tables. Electrical engineering is beyond our scope. All work performed must be in accordance with accepted industry-wide methods and applicable safety standards. Vector Structural Engineering assumes no responsibility for improper installation of the solar panels.

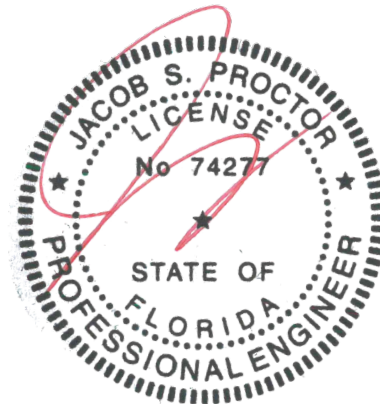
Very truly yours,

VECTOR STRUCTURAL ENGINEERING, LLC  
Firm License: COA 26626

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Jacob Proctor, P.E.  
License: 74277 - Expires: 02/28/2021  
Project Engineer

Enclosures

JSP/stb

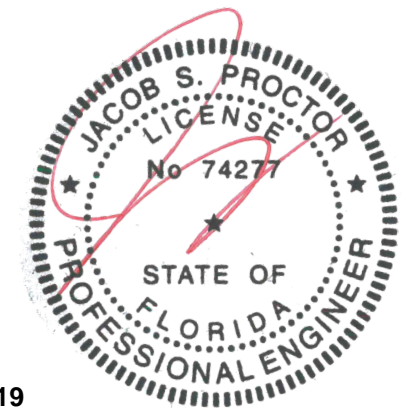
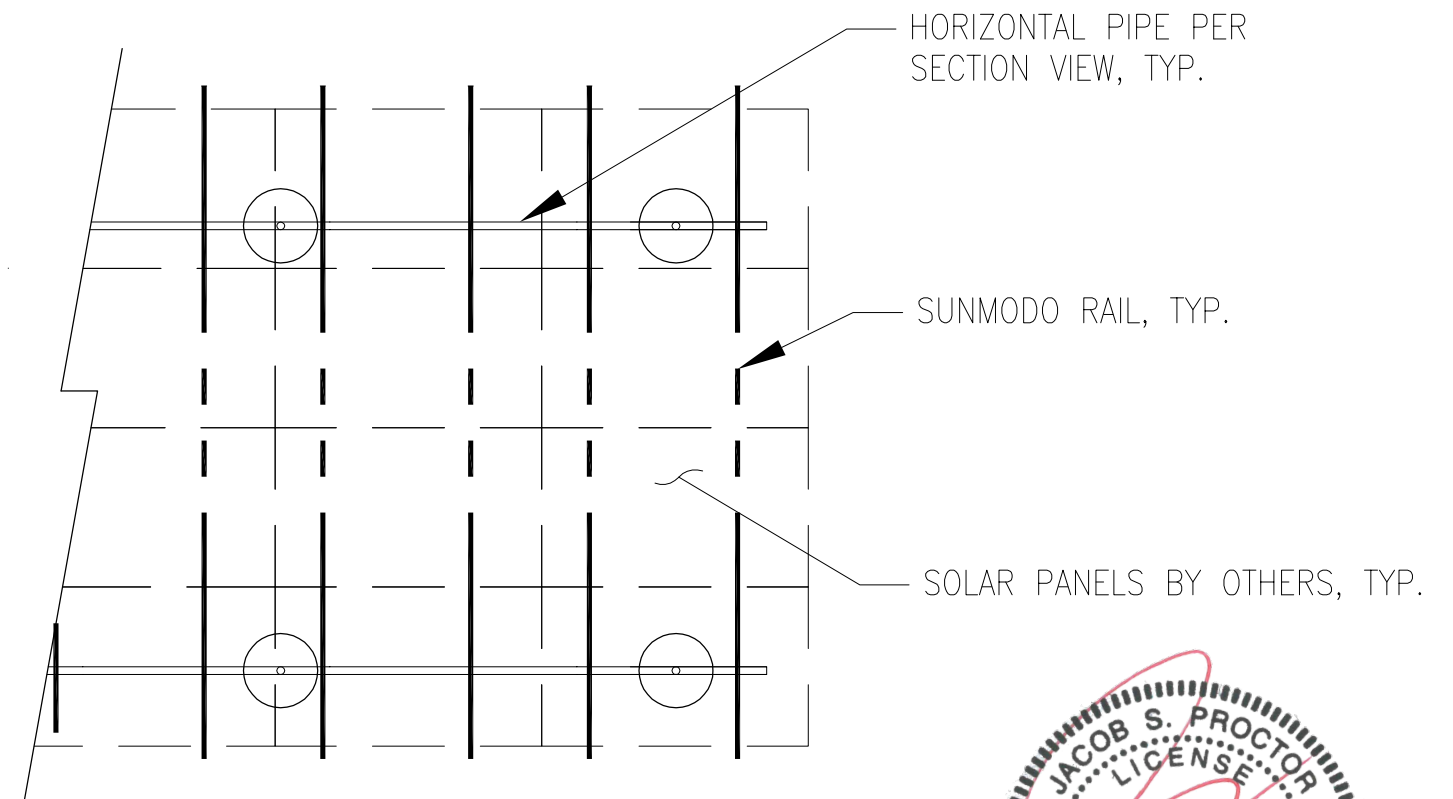
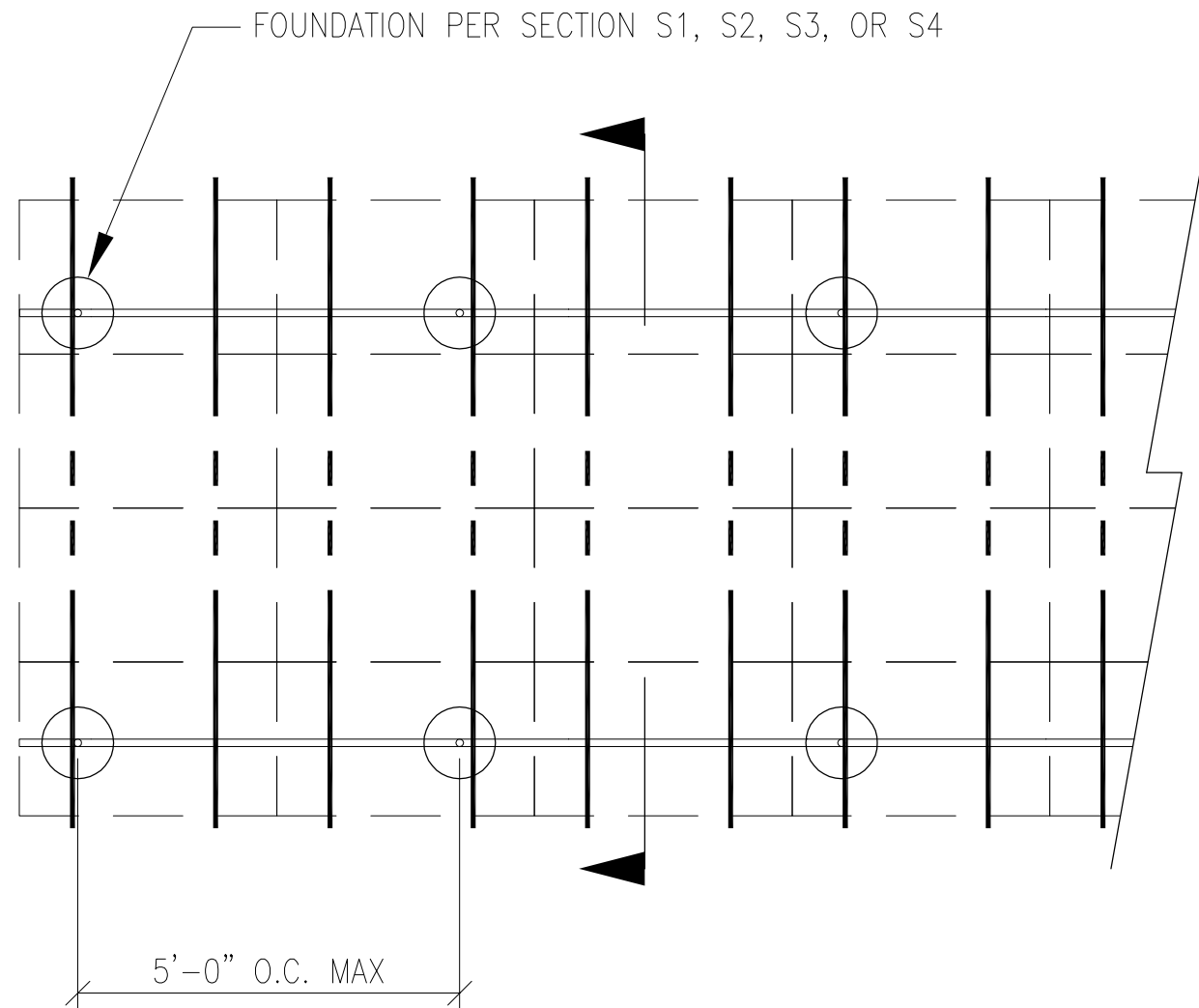


08/01/2019



JOB NO. U2716-103-191  
 PROJECT FLORIDA GROUND MOUNTS D1  
 SUBJECT ALL OPTIONS

651 W GALENA PARK BLVD. #101 (801) 990-1775  
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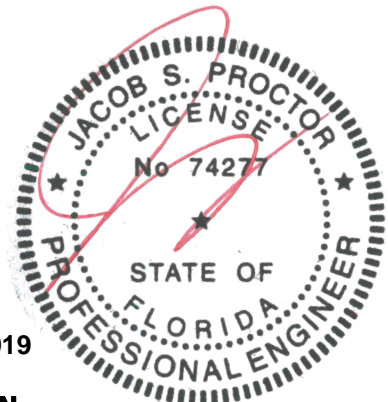
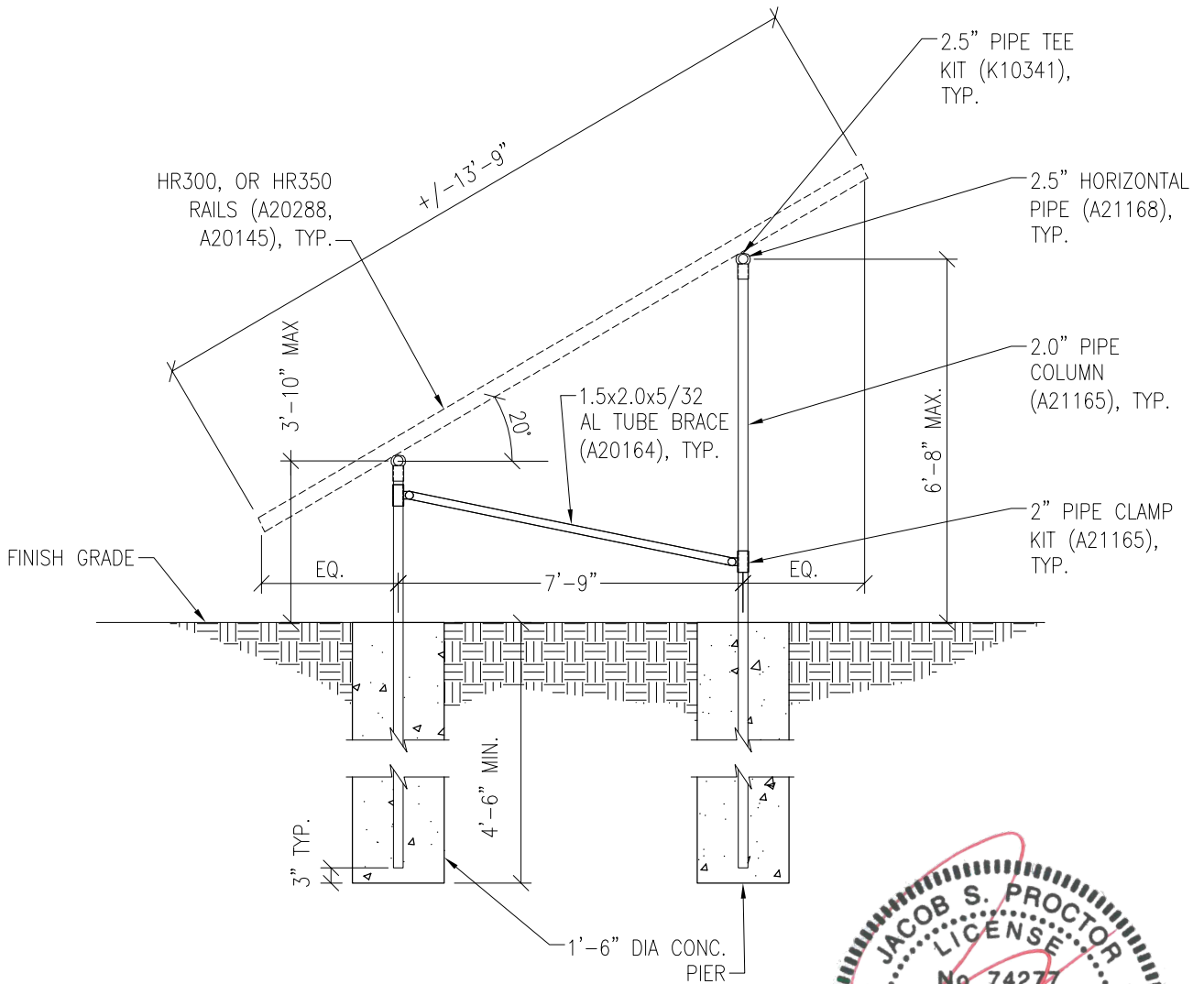
07/31/2019

**PV ARRAY PLAN**

N.T.S.

**P1**

Vector Engineers requires that we review each site specific install and is not liable for installs at site specific locations we have not reviewed.



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**PV ARRAY SECTION**

N.T.S.

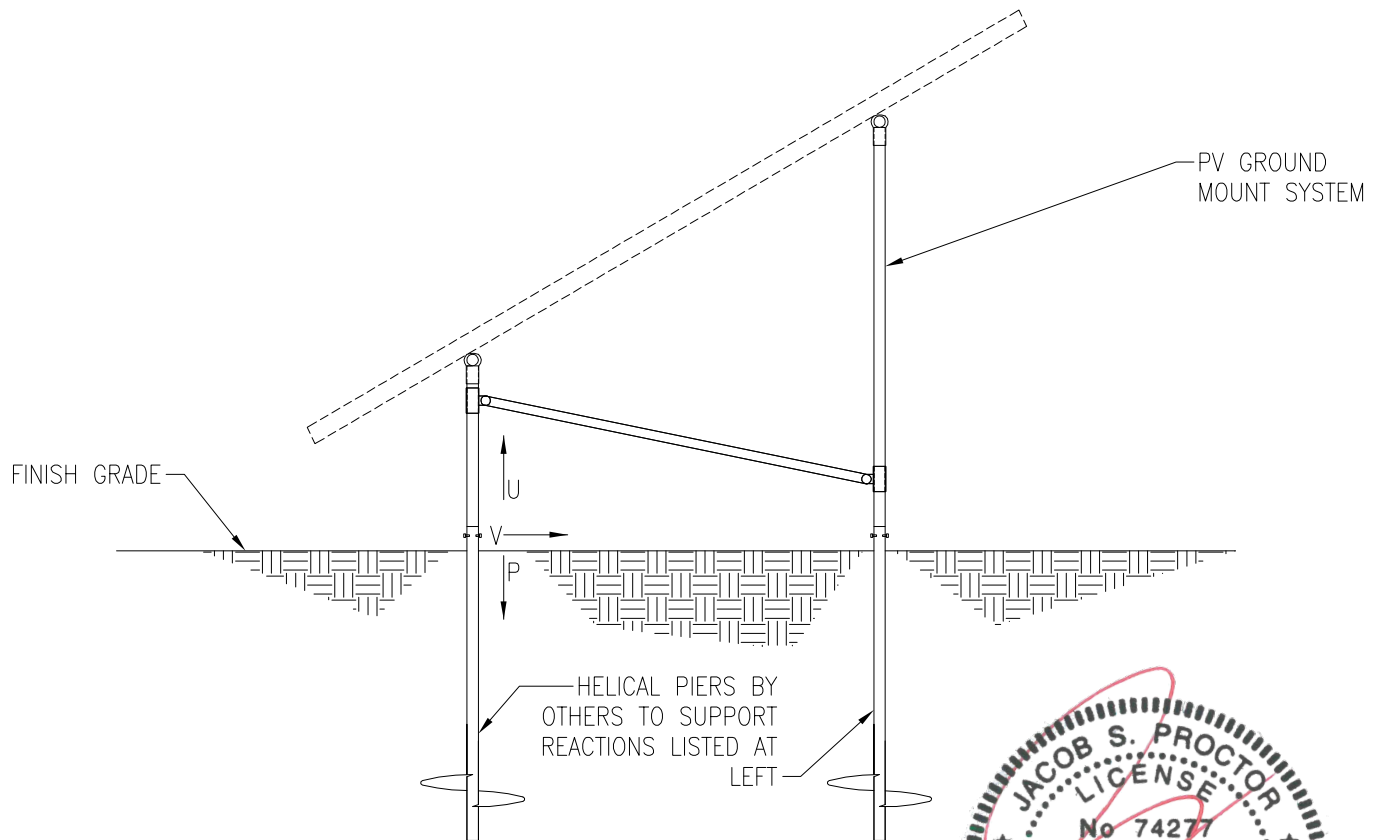
**S1**

PROJECT SUNMODO SUNTURF GROUND MOUNTS D1

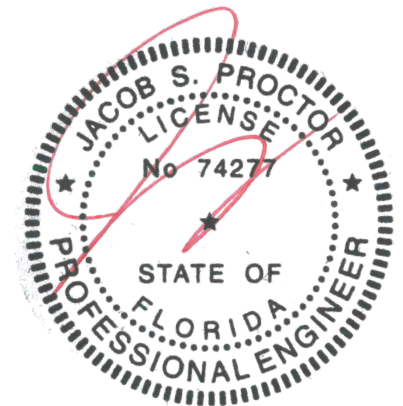
SUBJECT HELICAL PIER OPTION

**NOTES:**

1. For ground mount components see Section S1.
2. A minimum of (1) helical pier must be load-tested as follows:
  - 2.1. Safety factor for uplift = 1.5,
  - 2.2. Safety factor for lateral loads = 2.0
  - 2.3. Upward deflection limit = 1/2"
  - 2.4. Lateral deflection limit = 1"
  - 2.5. The load tests must be performed by an approved contractor.



MAXIMUM REACTIONS:	
U	= 2,640 LBS
P	= 2,760 LBS
V	= 1,180 LBS

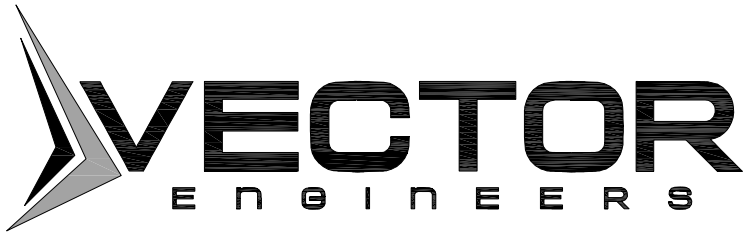


**PV ARRAY SECTION**

**03/26/2020**

N.T.S.

**S2**



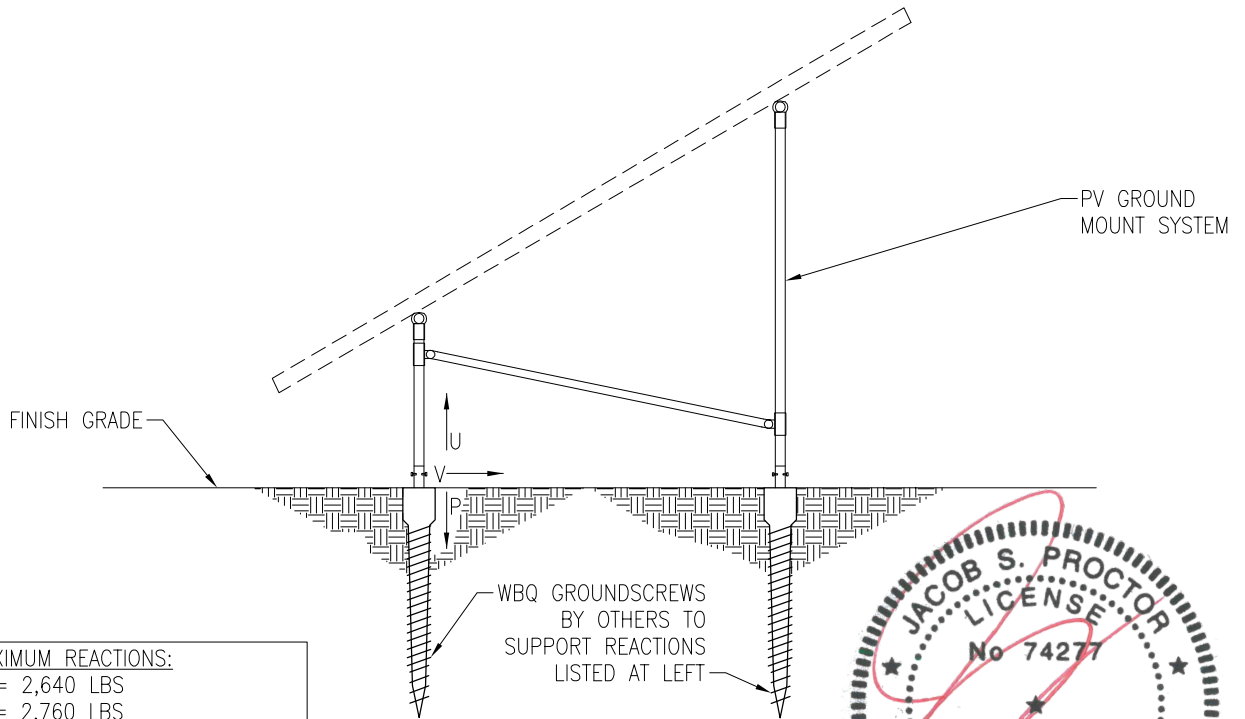
JOB NO. U2716-103-191

PROJECT SUNMODO SUNTURF GROUND MOUNTS D1

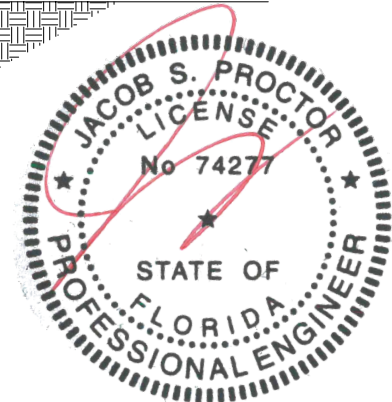
SUBJECT GROUND SCREW OPTION

NOTES:

- 1. For ground mount components see Section S1.
- 2. A minimum of (1) ground screw must be load-tested as follows:
  - 2.1. Safety factor for uplift = 1.5,
  - 2.2. Safety factor for lateral loads = 2.0
  - 2.3. Upward deflection limit = 1/2"
  - 2.4. Lateral deflection limit = 1"
  - 2.5. The load tests must be performed by an approved contractor.



MAXIMUM REACTIONS:  
 U = 2,640 LBS  
 P = 2,760 LBS  
 V = 1,180 LBS

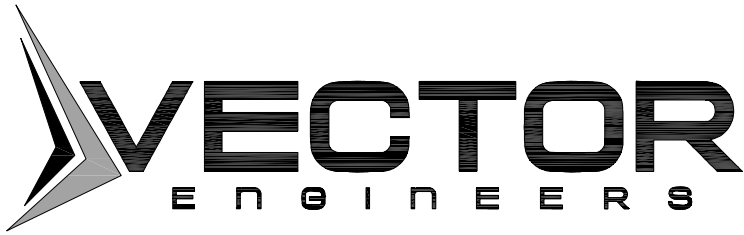


PV ARRAY SECTION

03/26/2020

S3

N.T.S.



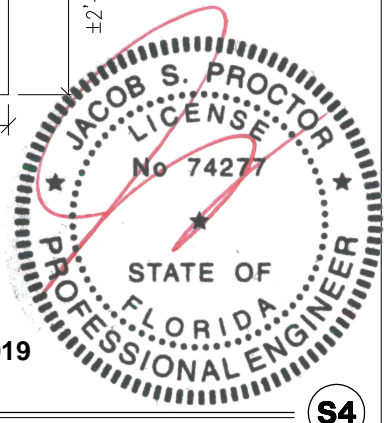
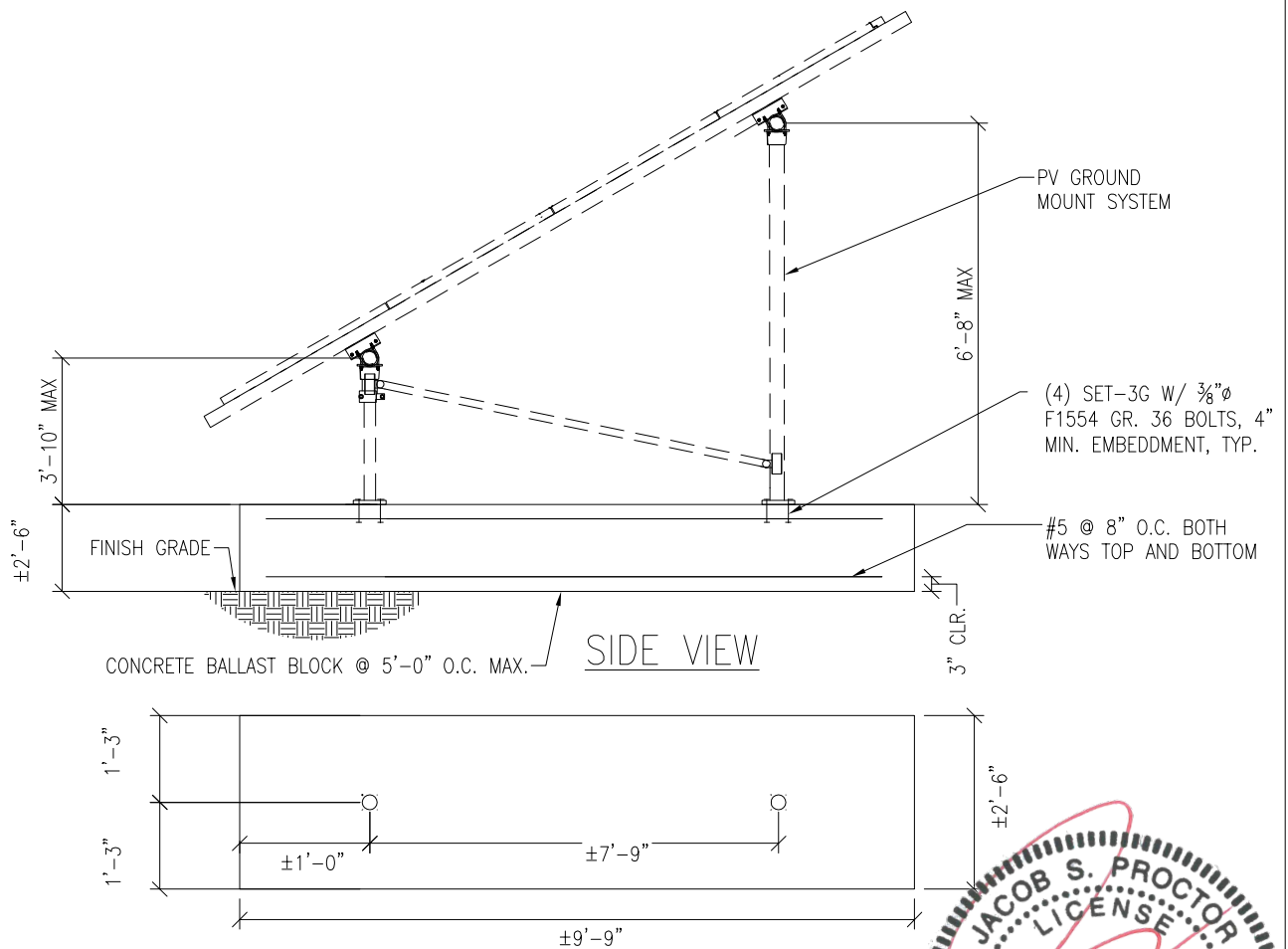
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PROJECT FLORIDA GROUND MOUNTS D1

SUBJECT BALLASTED BLOCK OPTION

NOTES:

1. For ground mount components see Section S1.



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PV ARRAY SECTION

N.T.S.

S4

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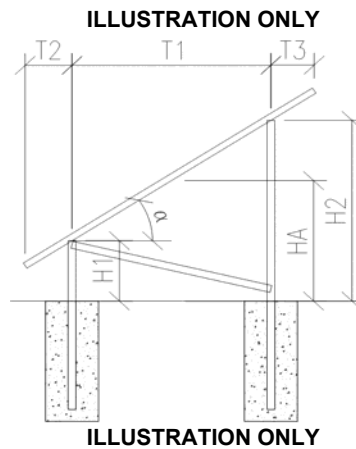
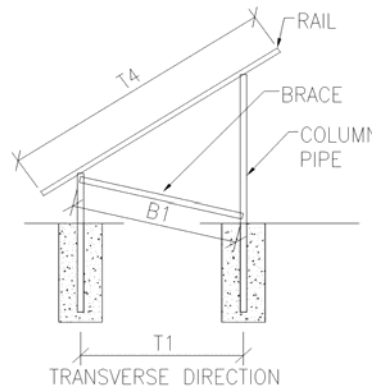
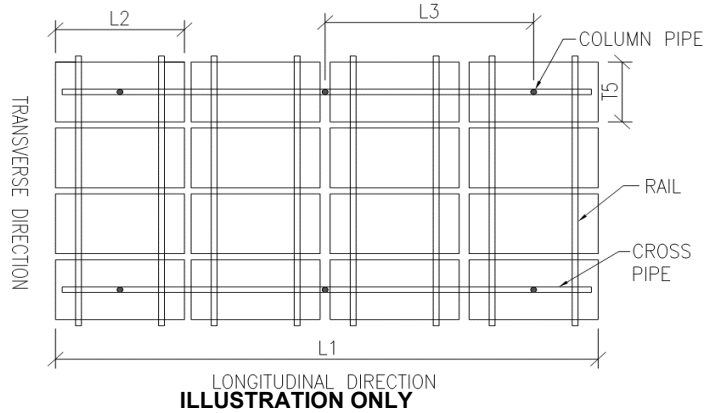
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PROJECT: D1 - Florida GM

SUBJECT: Dead Load

**Design Weight:**

Individual Panel Weight [lb]:	50.7
Panel Transverse Length (T5) [in]:	39.1
Panel Transverse Length (T5) [ft]:	3.3
Panel Longitudinal Length (L2) [in]:	77.0
Panel Longitudinal Length (L2) [ft]:	6.4
Individual Panel Area [ft <sup>2</sup> ]:	20.9
Individual Panel Weight [psf]:	2.4
# of Panels in Transverse Direction:	4
Approximate Transverse Length (T4) [ft]:	13.0
# of Panels in Longitudinal Direction:	12
Approximate Longitudinal Length (L1) [ft]:	77.0
Transverse Column Spacing (T1) [ft]:	7.8
Longitudinal Column Spacing (L3) [ft]:	5.0
# of Columns in Longitudinal Direction:	16
# of Columns in Transverse Direction:	2
Total Number of Columns:	32
Panel Slope from Horizontal (a) [°]:	20.0
Short Column Height (H1) [ft]:	4.0
Approximate Tall Column Height (H2) [ft]:	6.8
Transverse Brace between Columns :	Yes
Approximate Brace Length (B1) [ft]:	9.0
Weight of Columns [plf]:	3.7
Weight of Cross Pipe [plf]:	3.7
Weight of Brace [plf]:	3.7
Tributary Transverse Length per Column [ft]:	6.5
Tributary Longitudinal Length per Column [ft]:	5.0
Tributary Area per Column [ft <sup>2</sup> ]:	32.6
Rail Weight [plf]:	1.0
Transverse Rail Weight per Column [lb]:	13.0
Longitudinal Rail Weight per Column [lb]:	18.3
Tall Column Weight [lb]:	24.9
Panel Weight per Column [lb]:	79.0
Rail Weight per Column [lb]:	13.0
Cross Pipe Weight per Column [lb]:	18.3
Brace Weight per Column [lb]:	16.4
<b>Total Weight per Column (1.0 D) [lb]:</b>	<b>151.7</b>



**Assumptions:**

- T2 = T3



JOB NO.: U2716-103-191

DESIGNED: STB

DATE: 07/31/19

PROJECT: D1 - Florida GM

SUBJECT: Snow Load

**SNOW LOAD (S):**

ASCE 7 Standard:	10	
Panel Slope from Horizontal [°]:	20.0	
Snow Ground Load, $p_g$ [psf]:	0.0	(Section 7.2)
Terrain Category:	C	(Table 7-2)
Exposure of Roof:	Fully Exposed	(Table 7-2)
Exposure Factor, $C_e$ :	0.9	(Table 7-2)
Thermal Factor, $C_t$ :	1.2	(Table 7-3)
Risk Category:	I	(Table 1.5-1)
Importance Factor, $I_S$ :	0.8	(Table 1.5-2)
Flat Roof Snow Load, $p_f$ [psf]:	0	(Equation 7.3-1)
Minimum Roof Snow Load, $p_m$ [psf]:	0	(Section 7.3.4)
Unobstructed Slippery Surface?	Yes	(Section 7.4)
Slope Factor Figure:	Figure 7-2c	(Section 7.4)
Roof Slope Factor, $C_s$ :	0.909	(Figure 7-2)
Sloped Roof Snow Load, $p_s$ [psf]:	0	(Equation 7.4-1)
Design Snow Load, $S$ [psf]:	0	
Tributary Transverse Length [ft]:	6.1	
Tributary Longitudinal Length [ft]:	5	
Tributary Area per Column [ft <sup>2</sup> ]:	30.6	
<b>Snow Load per Column (1.0 S) [lb]:</b>	<b>0.0</b>	





PROJECT: D1 - Florida GM

SUBJECT: Wind Pressure

**Design Wind Load:**

ASCE 7 Standard:	10	
Basic Wind Speed, V [mph]:	170	
Risk Category:	I	
Exposure Category	C	(Section 26.7.3)
Velocity Pressure Exposure Coefficient, $K_h$ :	0.85	(Table 27.3-1)
Topographic Factor, $K_{ht}$ :	1.0	(Section 26.8.2)
Wind Directionality Factor, $K_d$ :	0.85	(Table 26.6-1)
Internal Pressure Coefficient, $GC_{pi}$ :	0.00	(Table 26.11-1)
Velocity Pressure, $q_h$ [psf]:	53.5	(Equation 27.3-1)
Gust Effect Factor, G:	0.85	(Section 26.9.1)
Panel Slope [degrees]:	20.0	

Net Pressure Coefficients ( $C_N$ ) per: (Figure 27.4-4)

Clear Wind Flow	$C_{NW}$	$C_{NL}$
Case 1 ( $\gamma = 0^\circ$ , Load Case A)	-1.30	-1.50
Case 2 ( $\gamma = 0^\circ$ , Load Case B)	-2.23	-0.20
Case 3 ( $\gamma = 180^\circ$ , Load Case A)	1.57	1.73
Case 4 ( $\gamma = 180^\circ$ , Load Case B)	2.07	0.67

Design Wind Pressures (p) [psf] per: (Equation 27.4-3)

Clear Wind Flow	$q_h GC_{NW}$	$q_h GC_{NL}$
Case 1 ( $\gamma = 0^\circ$ , Load Case A)	-59.1	-68.2
Case 2 ( $\gamma = 0^\circ$ , Load Case B)	-101.5	-9.1
Case 3 ( $\gamma = 180^\circ$ , Load Case A)	71.2	78.8
Case 4 ( $\gamma = 180^\circ$ , Load Case B)	93.9	30.3

Wind Pressure on Each Side of Panels [psf]

Clear Wind Flow	Short Col. Pressure	Long Col. Pressure
Case 1 ( $\gamma = 0^\circ$ , Load Case A)	-68.2	-59.1
Case 2 ( $\gamma = 0^\circ$ , Load Case B)	-9.1	-101.5
Case 3 ( $\gamma = 180^\circ$ , Load Case A)	71.2	78.8
Case 4 ( $\gamma = 180^\circ$ , Load Case B)	93.9	30.3



JOB NO.: U2716-103-191

PROJECT: D1 - Florida GM

SUBJECT: Open Building Wind Loads

### Design Wind Load Per ASCE 7-10

$$p = q_h G C_n$$

Velocity Pressure Exposure Coefficient, $K_{zt}$ :	0.85	(Table 27.3-1)
Topographic Factor, $K_{ht}$ :	1.0	(Section 26.8.2)
Wind Directionality Factor, $K_d$ :	0.85	(Table 26.6-1)
Ultimate Wind Speed, $V$ [mph]:	170	

Velocity Pressure, $q_h$ [psf]:	53.5	(Equation 27.3-1)
Gust Effect Factor, $G$ :	0.85	(Section 26.9.1)

$\gamma = 90^\circ$  or  $270^\circ$

Force Coefficient,  $C_N$ :

Horizontal Distance from Winward Edge	Roof angle		
	Load Case	Obstructed Wind Flow	
		CN	
$\leq h$	A	-0.8	
	B	0.8	
$> h, \leq 2h$	A	-0.6	
	B	0.5	
$> 2h$	A	-0.3	
	B	0.3	

Design Wind Pressure,  $p$  [psf]:

Horizontal Distance from Winward Edge	Roof angle		
	Load Case	Obstructed Wind Flow	
$\leq h$	A	-36.3	
	B	36.3	
$> h, \leq 2h$	A	-27.3	
	B	22.7	
$> 2h$	A	-13.6	
	B	13.6	



JOB NO.: U2716-103-191

DESIGNED: STB

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# Foundation Option 1: Drilled Concrete Pier



JOB NO.: U2716-103-191  
DATE: 07/31/19

DESIGNED: STB

**PROJECT: D1 - Florida GM**

**Drilled Pier Design**

**Design Loads:**

Max. Shear, V [k]:	1.2	Max. Down, P <sub>d</sub> [k]:	2.8
Max. Moment, M [k-ft]:	0.0	Max. Uplift, P <sub>u</sub> [k]:	2.6

**Pier Properties:**

Pier Diameter, b [ft]:	1.5	Volume of Concrete [ft <sup>3</sup> ]:	8
Min. Pier Diameter, b <sub>min</sub> (opt'l) [ft]:		Volume of Concrete [yd <sup>3</sup> ]:	0.3
Top of Pier Elevation [ft]:	0.00	Weight of Concrete [k]:	1.2
Pier Depth, d [ft]:	4.5		
Min. Pier Depth, d <sub>min</sub> (opt'l) [ft]:			
Max. Pier Depth, d <sub>max</sub> (opt'l) [ft]:			

**Soil Properties:**

Allow. Bearing Pressure [psf]:	1,500	<u>Optional Parameters for Uplift:</u>	
1/3 increase for short term loads?	No	Skin Friction [psf]:	250 (IBC 1810.3.3.1.4)
Lateral Bearing, S [pcf]:	150	Top Length to Ignore [ft]:	0.0
Max. Lateral Bearing (opt'l) [psf]:		1/3 increase for short term loads?	No
Top Depth to Ignore [ft]:	0.0	Combine w/ Bearing:	No
1/3 increase for short term loads?	No		
1/2" deflection at t/o pier allowed:	Yes		

**Check Bearing:**

Bearing Capacity [k]:	5.3	<b>Bearing capacity OK.</b>
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**Check Uplift:**

Uplift Capacity [k]:	6.4	<b>Uplift capacity OK.</b>
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**Check Lateral Bearing:**

Applied Lateral Force, P [lb]:	1,180	
Point of Application, h [ft]:	0.0	
S <sub>1_max</sub> [psf]:		
S <sub>1</sub> [psf]:	450	
A = 2.34*P/(S <sub>1</sub> b):	4.09	
Required Pier Depth, d <sub>reqd</sub> [ft]:	4.1	<b>Lateral bearing capacity OK.</b>

# Foundation Option 2: Helical Pier

The ground screws and helical piers must be tested to 1.5 times uplift and 2.0 times lateral reactions found in the table below.

Load (ASD)	Value (lbs)	Factor of Safety	Test Value (lbs)
UPLIFT	2640	1.5	3960
LATERAL	1180	2	2360

# Foundation Option 3: Ground Screw

The ground screws and helical piers must be tested to 1.5 times uplift and 2.0 times lateral reactions found in the table below.

Load (ASD)	Value (lbs)	Factor of Safety	Test Value (lbs)
UPLIFT	2640	1.5	3960
LATERAL	1180	2	2360

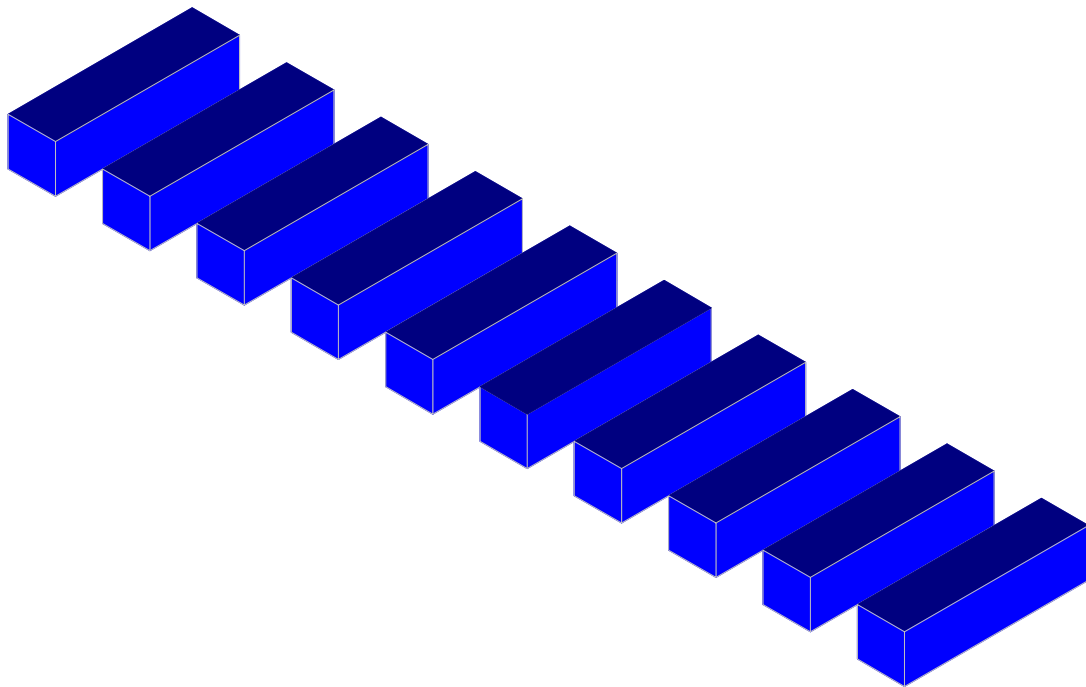


JOB NO.: U2716-103-191

DESIGNED: STB

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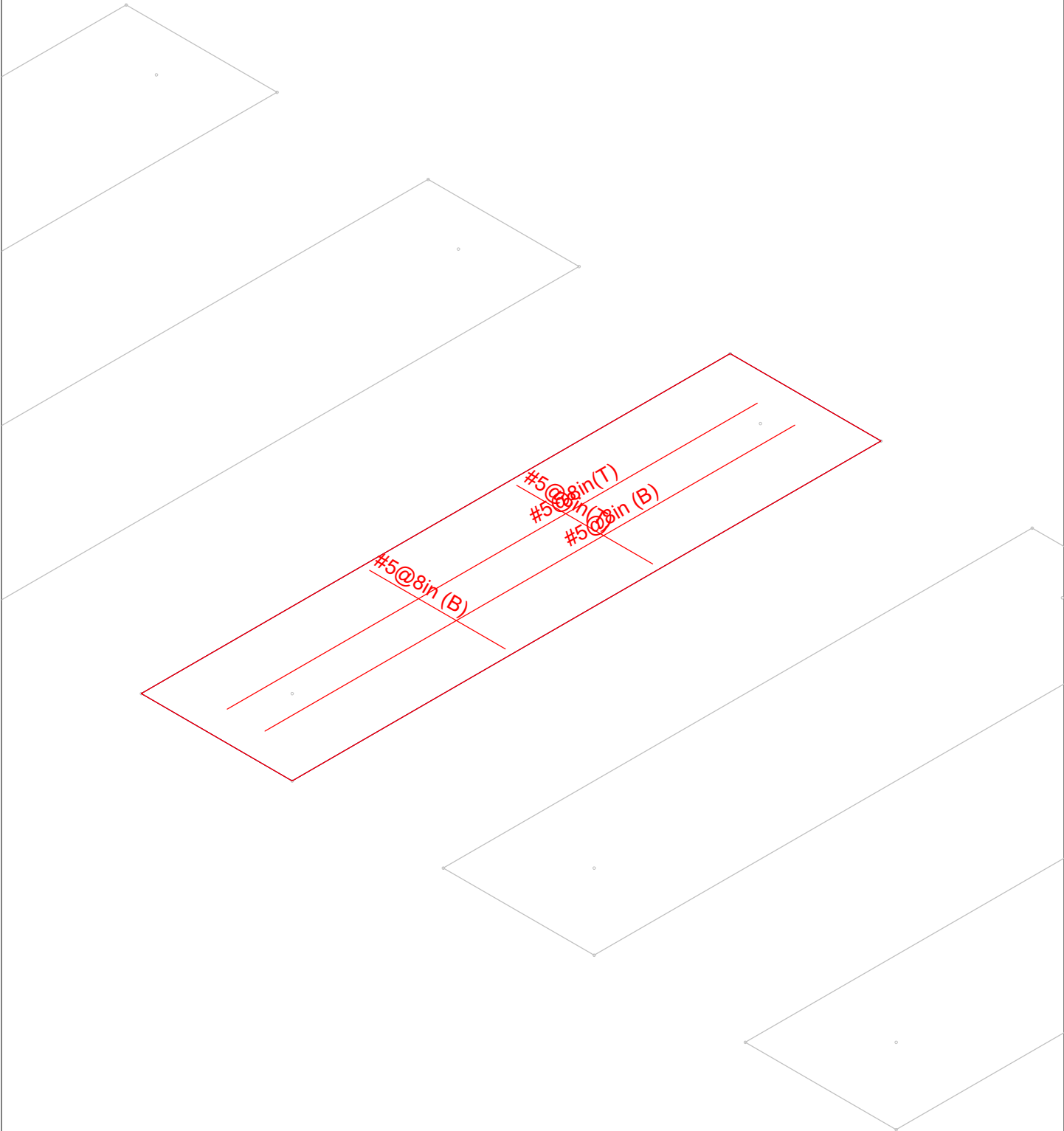
# Foundation Option 4: Ballasted Block



Results for LC 1, 1.0 D

Vector Structural Engineeri..	Ground Mount	SK - 13
STB		May 15, 2019 at 11:18 AM
U2716.103.191		Florida D1 GM 81 in panels.r3d





Loads: DL - Dead Load  
Results for LC 1, 1.0 D

Vector Structural Engineeri..	Ground Mount	SK - 12
STB		May 15, 2019 at 11:18 AM
U2716.103.191		Florida D1 GM 81 in panels.r3d

















**Point Loads and Moments (Cat 21 : OL6) (Continued)**

	Label	Direction	Magnitude [lb.-ft]
6	R3D N2 1	Z	-27.241
7	R3D N132 1	X	-4.651
8	R3D N132 1	Y	-614.422
9	R3D N132 1	Z	305.386
10	R3D N133 1	X	-1.621
11	R3D N133 1	Y	-199.834
12	R3D N133 1	Z	-11.726
13	R3D N109 1	X	-8.385
14	R3D N109 1	Y	-1162.324
15	R3D N109 1	Z	639.749
16	R3D N110A 1	X	-17.197
17	R3D N110A 1	Y	-379.793
18	R3D N110A 1	Z	-22.851
19	R3D N121 1	Y	-1213.821
20	R3D N121 1	Z	579.806
21	R3D N122 1	X	9.699
22	R3D N122 1	Y	-417.1
23	R3D N122 1	Z	-18.924
24	R3D N133B 1	X	-3.373
25	R3D N133B 1	Y	-716.165
26	R3D N133B 1	Z	368.712
27	R3D N134B 1	X	-1.769
28	R3D N134B 1	Y	-211.747
29	R3D N134B 1	Z	-14.135
30	R3D N151 2	X	-3.54
31	R3D N151 2	Y	-679.312
32	R3D N151 2	Z	332.841
33	R3D N152 2	Y	-225.888
34	R3D N152 2	Z	-11.495
35	R3D N157A	Y	-683.655
36	R3D N157A	Z	347.522
37	R3D N158A	X	6.417
38	R3D N158A	Y	-204.868
39	R3D N158A	Z	-11.552
40	R3D N155B	Y	-646.101
41	R3D N155B	Z	311.494
42	R3D N156B	X	5.129
43	R3D N156B	Y	-221.484
44	R3D N156B	Z	-11.02
45	R3D N167 1	X	-8.072
46	R3D N167 1	Y	-595.175
47	R3D N167 1	Z	323.384
48	R3D N168 1	X	-7.343
49	R3D N168 1	Y	-191.664
50	R3D N168 1	Z	-11.309
51	R3D N179 1	X	-27.492
52	R3D N179 1	Y	-769.572
53	R3D N179 1	Z	357.38
54	R3D N180 1	X	10.877
55	R3D N180 1	Y	-229.574
56	R3D N180 1	Z	-11.842

**Slabs**

	Label	Thickness [in]	Material	Local Axis Angle [deg]	Analysis Offset [in]
1	S1	30	Conc2500NW	0	0
2	S2	30	Conc2500NW	0	0























Company:		Date:	5/14/2018
Engineer:		Page:	1/6
Project:			
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-14  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: F1554 Grade 36  
Diameter (inch): 0.375  
Effective Embedment depth,  $h_{ef}$  (inch): 4.000  
Code report: ICC-ES ESR-4057  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 5.25  
 $c_{ac}$  (inch): 7.12  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 30.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Ignore concrete breakout in tension: No  
Ignore concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 150/110°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.75 x 4.75 x 0.31

#### Recommended Anchor

Anchor Name: SET-3G - SET-3G w/ 3/8"Ø F1554 Gr. 36  
Code Report: ICC-ES ESR-4057





Company:		Date:	5/14/2018
Engineer:		Page:	2/6
Project:			
Address:			
Phone:			
E-mail:			

**Load and Geometry**

Load factor source: ACI 318 Section 5.3

Load combination: not set

Seismic design: No

Anchors subjected to sustained tension: No

Apply entire shear load at front row: No

Anchors only resisting wind and/or seismic loads: No

Strength level loads:

$N_{ua}$  [lb]: 4400

$V_{uax}$  [lb]: 130

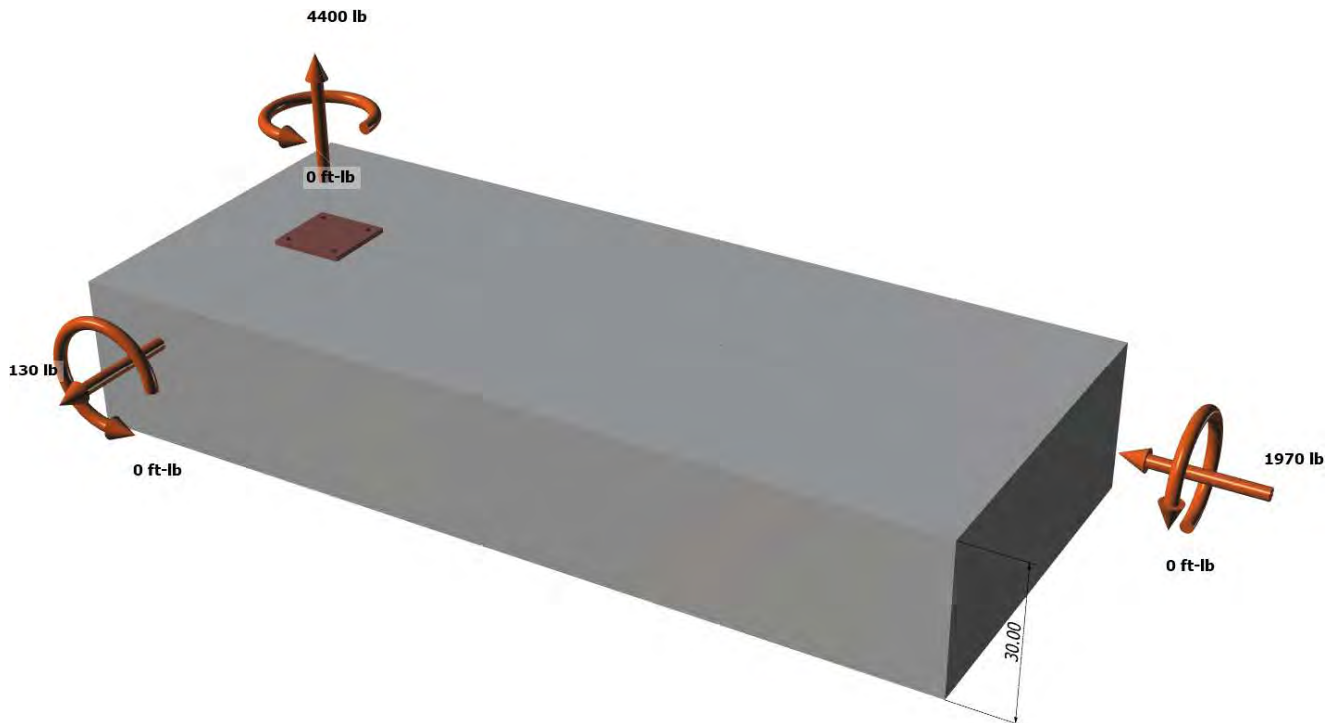
$V_{uay}$  [lb]: -1970

$M_{ux}$  [ft-lb]: 0

$M_{uy}$  [ft-lb]: 0

$M_{uz}$  [ft-lb]: 0

<Figure 1>







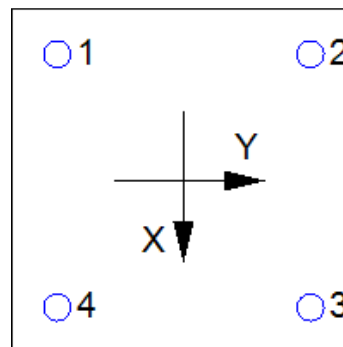
Company:		Date:	5/14/2018
Engineer:		Page:	4/6
Project:			
Address:			
Phone:			
E-mail:			

### 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1100.0	32.5	-492.5	493.6
2	1100.0	32.5	-492.5	493.6
3	1100.0	32.5	-492.5	493.6
4	1100.0	32.5	-492.5	493.6
Sum	4400.0	130.0	-1970.0	1974.3

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 4400  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis, e'<sub>Nx</sub> (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis, e'<sub>Ny</sub> (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis, e'<sub>Vx</sub> (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis, e'<sub>Vy</sub> (inch): 0.00

<Figure 3>



### 4. Steel Strength of Anchor in Tension (Sec. 17.4.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
4525	0.75	3394

### 5. Concrete Breakout Strength of Anchor in Tension (Sec. 17.4.2)

$$N_b = K_c \lambda_a \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. 17.4.2.2a)}$$

K <sub>c</sub>	λ <sub>a</sub>	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	4.000	6800

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. 17.3.1 \& Eq. 17.4.2.1b)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	c <sub>a,min</sub> (in)	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
240.25	144.00	7.25	1.000	1.000	1.00	1.000	6800	0.65	7374

### 6. Adhesive Strength of Anchor in Tension (Sec. 17.4.5)

$$\tau_{k,cr} = \tau_{k,cr,short-term} K_{sat} (f_c / 2,500)^n$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	f <sub>c</sub> (psi)	n	τ <sub>k,cr</sub> (psi)
1346	1.00	1.00	2500	0.24	1346

$$N_{ba} = \lambda_a \tau_{cr} \pi d_a h_{ef} \text{ (Eq. 17.4.5.2)}$$

λ <sub>a</sub>	τ <sub>cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>ba</sub> (lb)
1.00	1346	0.38	4.000	6343

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ec,Na} \psi_{ed,Na} \psi_{cp,Na} N_{ba} \text{ (Sec. 17.3.1 \& Eq. 17.4.5.1b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	c <sub>Na</sub> (in)	c <sub>a,min</sub> (in)	ψ <sub>ec,Na</sub>	ψ <sub>ed,Na</sub>	ψ <sub>cp,Na</sub>	N <sub>ba</sub> (lb)	φ	φN <sub>ag</sub> (lb)
198.45	112.09	5.29	7.25	1.000	1.000	1.000	6343	0.55	6176



Company:		Date:	5/14/2018
Engineer:		Page:	5/6
Project:			
Address:			
Phone:			
E-mail:			

**8. Steel Strength of Anchor in Shear (Sec. 17.5.1)**

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
2715	1.0	0.65	1765

**9. Concrete Breakout Strength of Anchor in Shear (Sec. 17.5.2)**

**Shear perpendicular to edge in x-direction:**

$V_{bx} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$  (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

$l_e$ (in)	$d_a$ (in)	$\lambda_a$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
3.00	0.375	1.00	2500	13.75	16564

$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx}$  (Sec. 17.3.1 & Eq. 17.5.2.1b)

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
647.11	850.78	1.000	0.805	1.000	1.000	16564	0.70	7103

**Shear perpendicular to edge in y-direction:**

$V_{by} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$  (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

$l_e$ (in)	$d_a$ (in)	$\lambda_a$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
3.00	0.375	1.00	2500	10.75	11450

$\phi V_{cbgy} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by}$  (Sec. 17.3.1 & Eq. 17.5.2.1b)

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgy}$ (lb)
387.00	520.03	1.000	0.891	1.000	1.000	11450	0.70	5313

**Shear parallel to edge in x-direction:**

$V_{by} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$  (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

$l_e$ (in)	$d_a$ (in)	$\lambda_a$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
3.00	0.375	1.00	2500	7.25	6342

$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by}$  (Sec. 17.3.1, 17.5.2.1(c) & Eq. 17.5.2.1b)

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
261.00	236.53	1.000	1.000	1.000	1.000	6342	0.70	9797

**Shear parallel to edge in y-direction:**

$V_{bx} = \min|7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}|$  (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

$l_e$ (in)	$d_a$ (in)	$\lambda_a$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
3.00	0.375	1.00	2500	10.25	10661

$\phi V_{cbgy} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx}$  (Sec. 17.3.1, 17.5.2.1(c) & Eq. 17.5.2.1b)

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgy}$ (lb)
401.67	472.78	1.000	1.000	1.000	1.000	10661	0.70	12680

**10. Concrete Pryout Strength of Anchor in Shear (Sec. 17.5.3)**

$\phi V_{cp} = \phi \min|k_{cp} N_{ba}; k_{cp} N_{cbg}| = \phi \min|k_{cp} (A_{Na} / A_{Na0}) \psi_{ec,Na} \psi_{ed,Na} \psi_{cp,Na} N_{ba}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b|$  (Sec. 17.3.1 & Eq. 17.5.3.1b)

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{ec,Na}$	$\psi_{cp,Na}$	$N_{ba}$ (lb)	$N_a$ (lb)
2.0	198.45	112.09	1.000	1.000	1.000	6343	11230

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
240.25	144.00	1.000	1.000	1.000	1.000	6800	11345	0.70

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Company:		Date:	5/14/2018
Engineer:		Page:	6/6
Project:			
Address:			
Phone:			
E-mail:			

$$\frac{\phi V_{cpg} \text{ (lb)}}{15722}$$

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. R17.6)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1100	3394	0.32	Pass	
Concrete breakout	4400	7374	0.60	Pass	
<b>Adhesive</b>	<b>4400</b>	<b>6176</b>	<b>0.71</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	494	1765	0.28	Pass	
T Concrete breakout x+	130	7103	0.02	Pass	
T Concrete breakout y-	1970	5313	0.37	Pass	
Concrete breakout y-	65	9797	0.01	Pass	
Concrete breakout x-	985	12680	0.08	Pass	
<b>Concrete breakout, combined</b>	-	-	<b>0.37</b>	<b>Pass (Governs)</b>	
Pryout	1974	15722	0.13	Pass	
Interaction check	$(N_{ua}/\phi N_{ua})^{5/3}$	$(V_{ua}/\phi V_{ua})^{5/3}$	Combined Ratio	Permissible	Status
Sec. R17.6	0.57	0.19	76.0%	1.0	Pass

**SET-3G w/ 3/8"Ø F1554 Gr. 36 with hef = 4.000 inch meets the selected design criteria.**

## 12. Warnings

- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



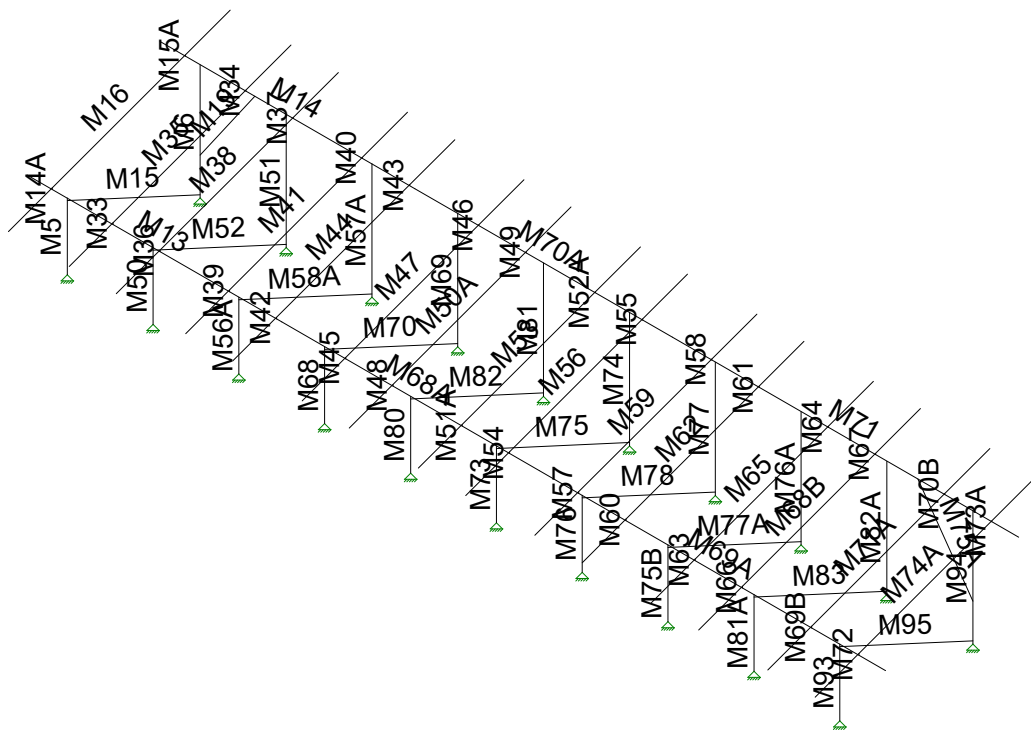


JOB NO.: U2716-103-191

DESIGNED: STB

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# Framing Analysis



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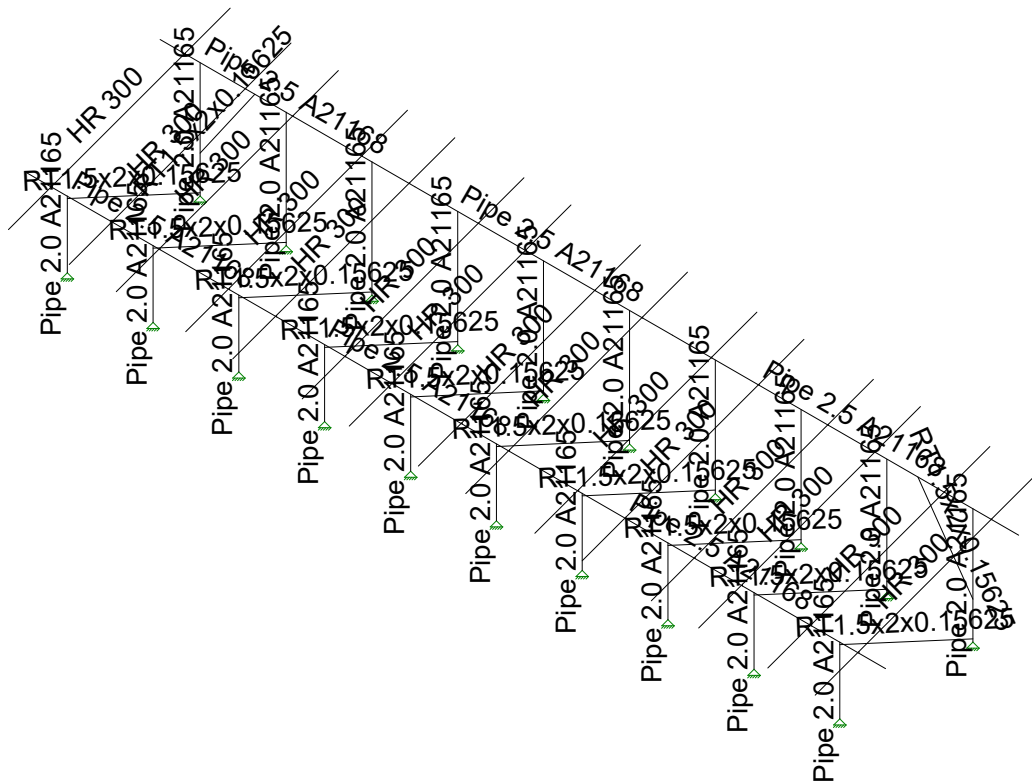
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Ground Mount

SK - 3

July 31, 2019 at 3:05 PM

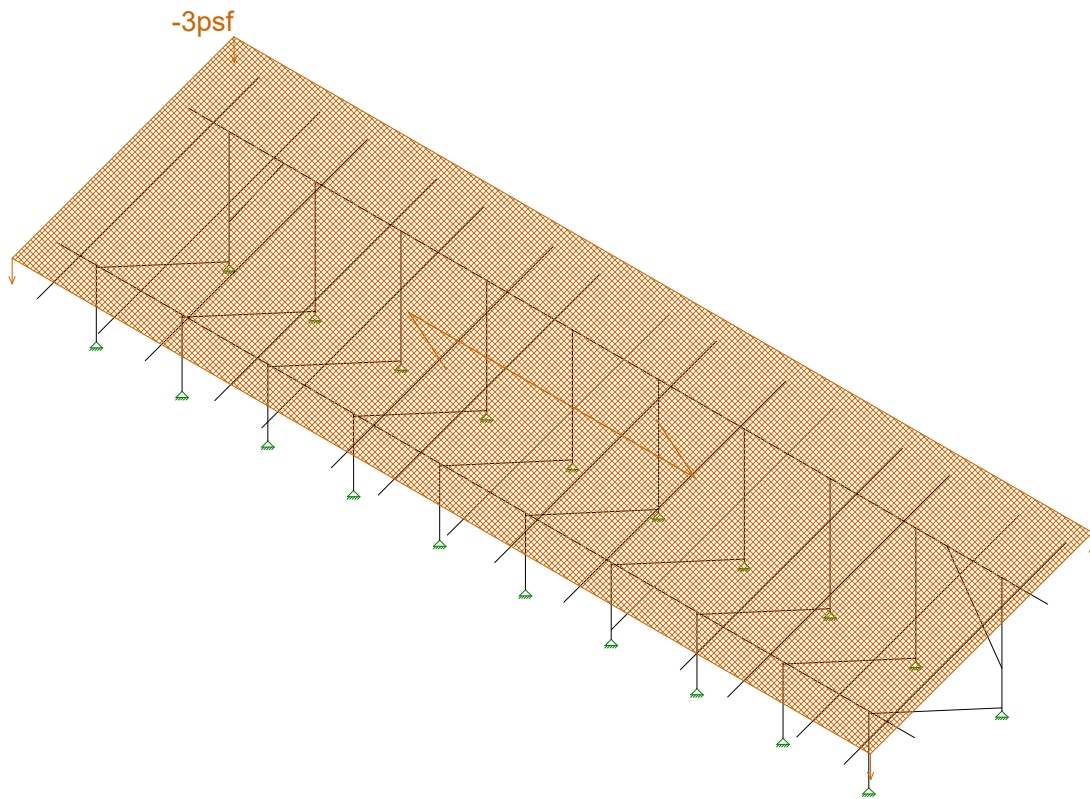
Florida D1 GM v3.r3d



Vector Structural Engineeri...  
STB  
U2716.103.191

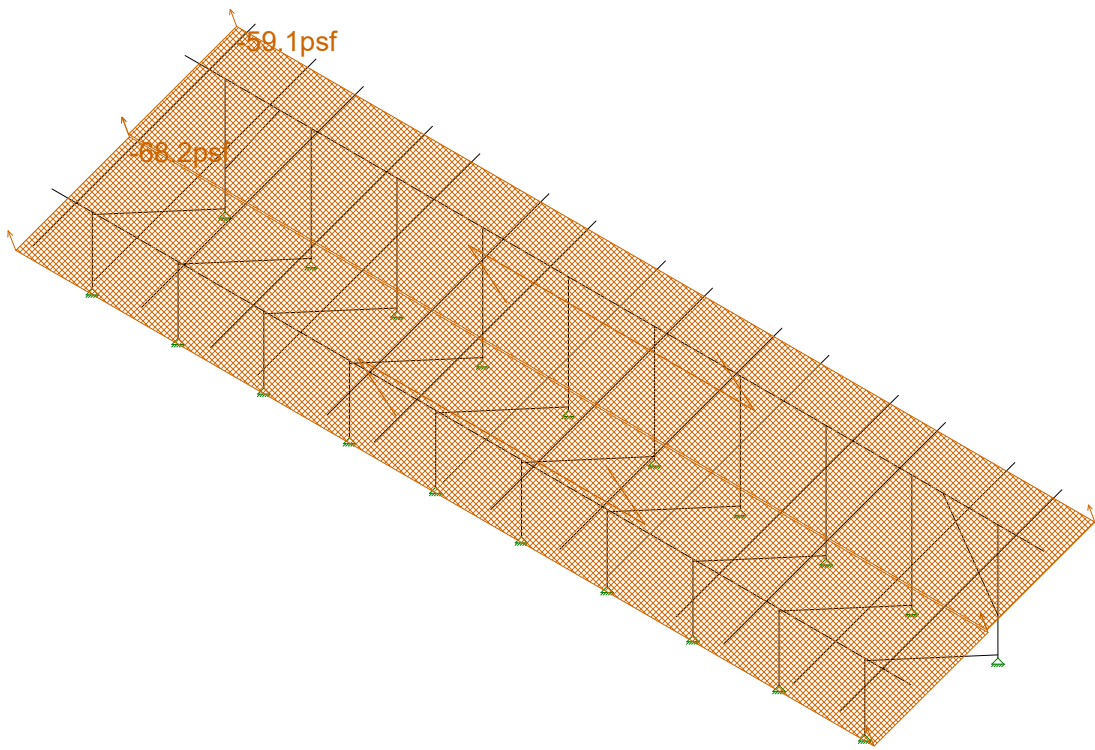
Ground Mount

SK - 4  
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Florida D1 GM v3.r3d



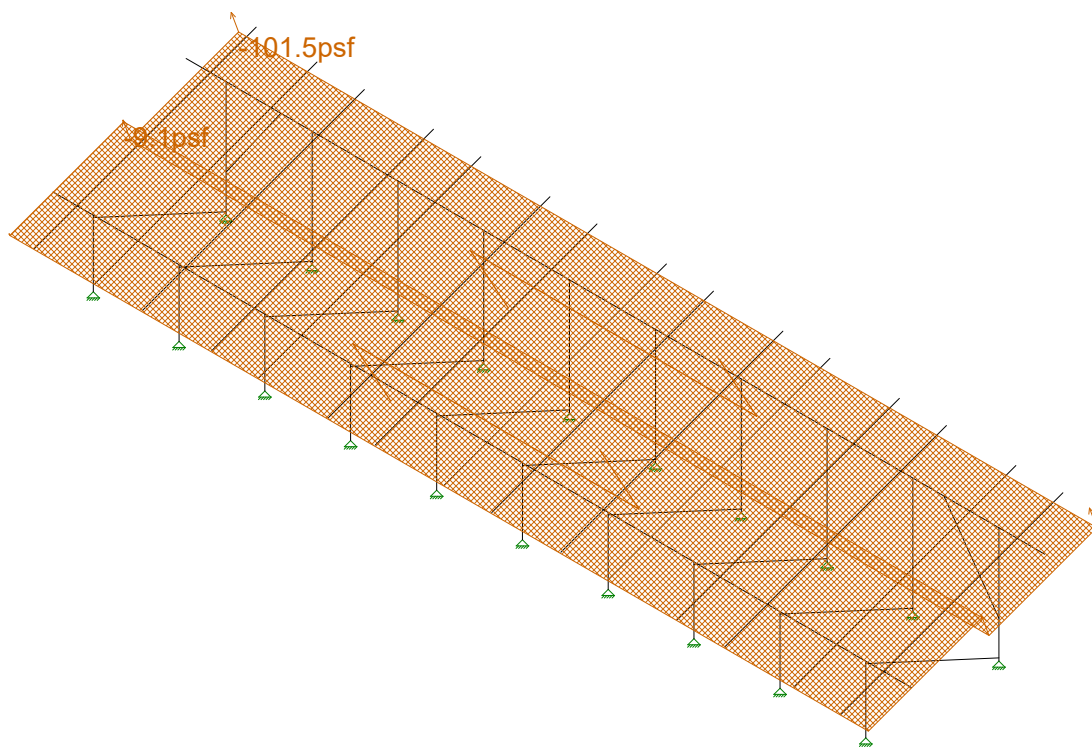
Loads: BLC 2, Solar Panel Weight

Vector Structural Engineeri..	Ground Mount	SK - 5
STB		July 31, 2019 at 3:05 PM
U2716.103.191		Florida D1 GM v3.r3d



Loads: BLC 4, Wind A 0 deg

Vector Structural Engineeri..	Ground Mount	SK - 6
STB		July 31, 2019 at 3:05 PM
U2716.103.191		Florida D1 GM v3.r3d



Loads: BLC 5, Wind B 0 deg

Vector Structural Engineeri..

STB

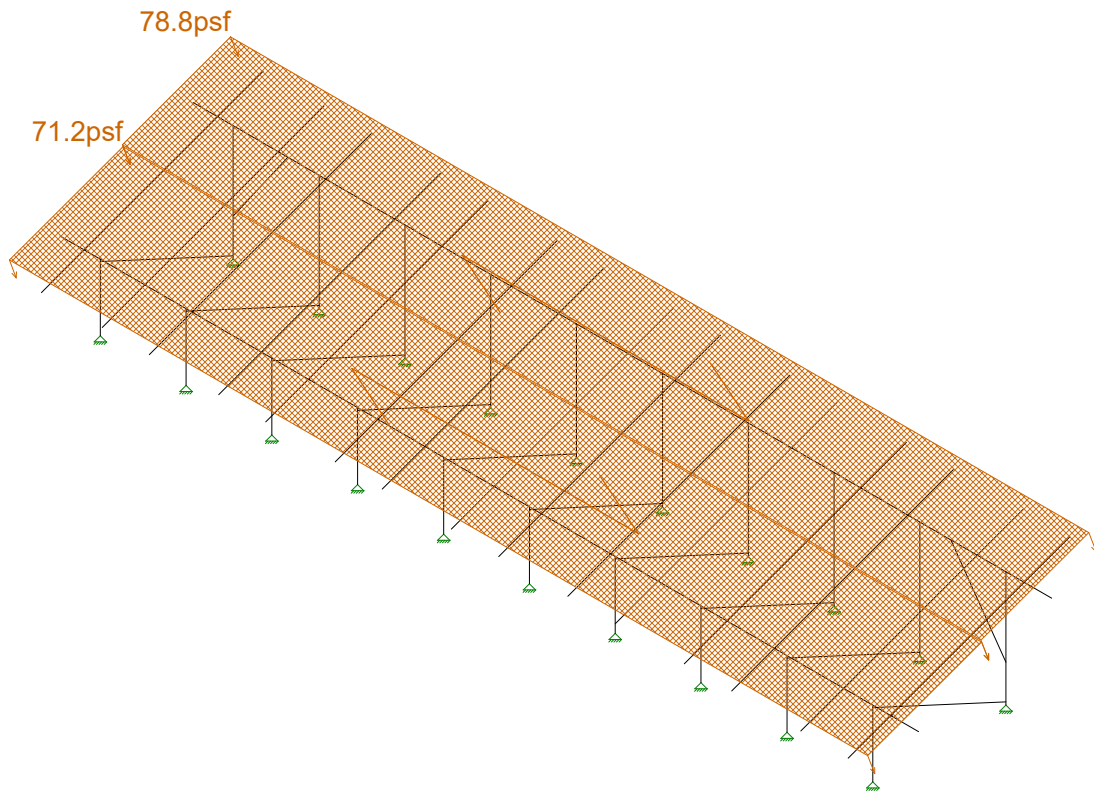
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Ground Mount

SK - 7

July 31, 2019 at 3:06 PM

Florida D1 GM v3.r3d



Loads: BLC 6, Wind A 180 deg

Vector Structural Engineeri..

STB

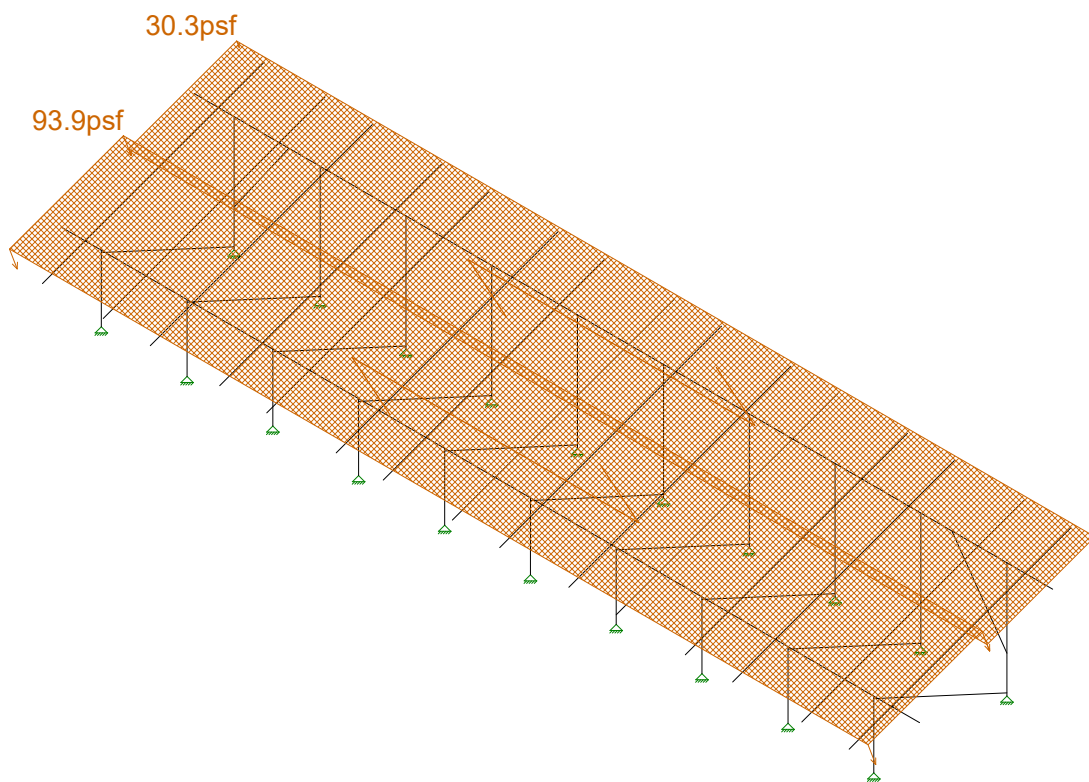
U2716.103.191

Ground Mount

SK - 8

July 31, 2019 at 3:06 PM

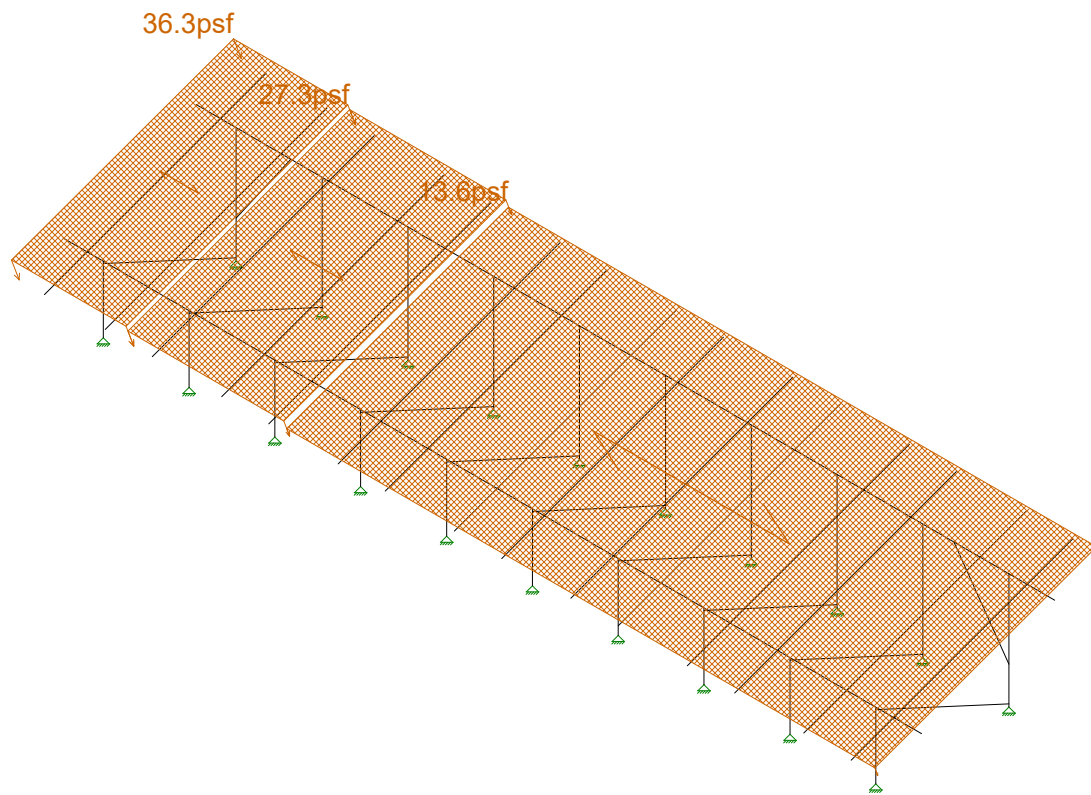
Florida D1 GM v3.r3d



Loads: BLC 7, Wind B 180 deg

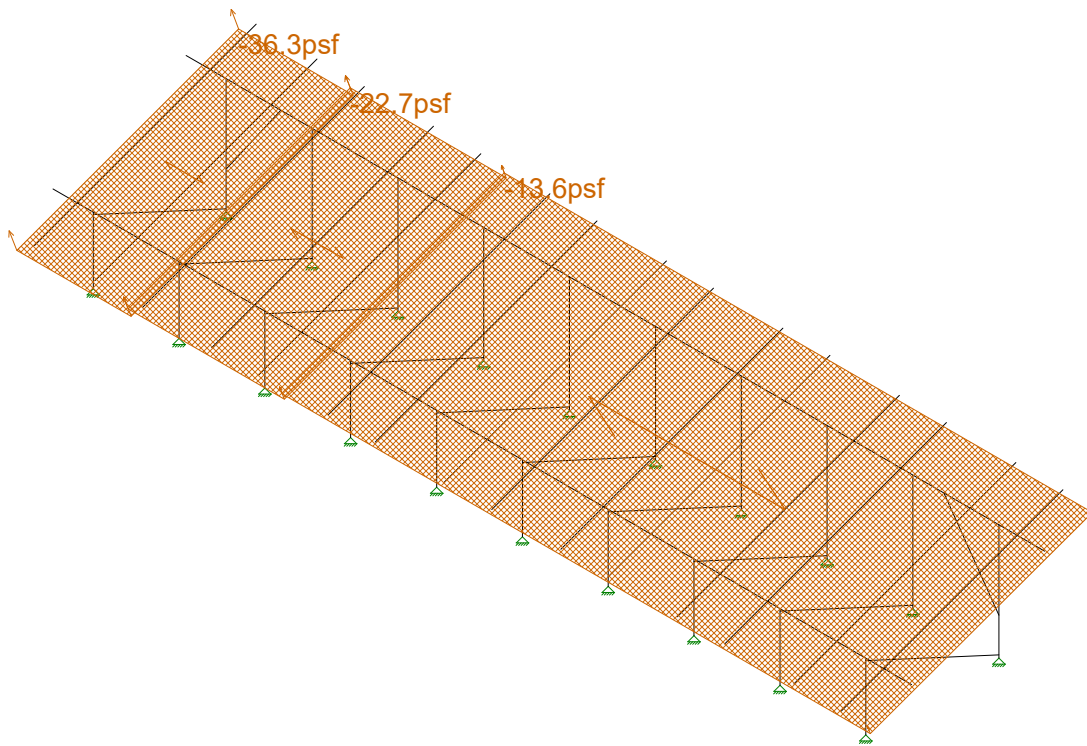
Vector Structural Engineeri..	Ground Mount	SK - 9
STB		July 31, 2019 at 3:06 PM
U2716.103.191		Florida D1 GM v3.r3d





Loads: BLC 8, Wind A 90

Vector Structural Engineeri..	Ground Mount	SK - 10
STB		July 31, 2019 at 3:06 PM
U2716.103.191		Florida D1 GM v3.r3d

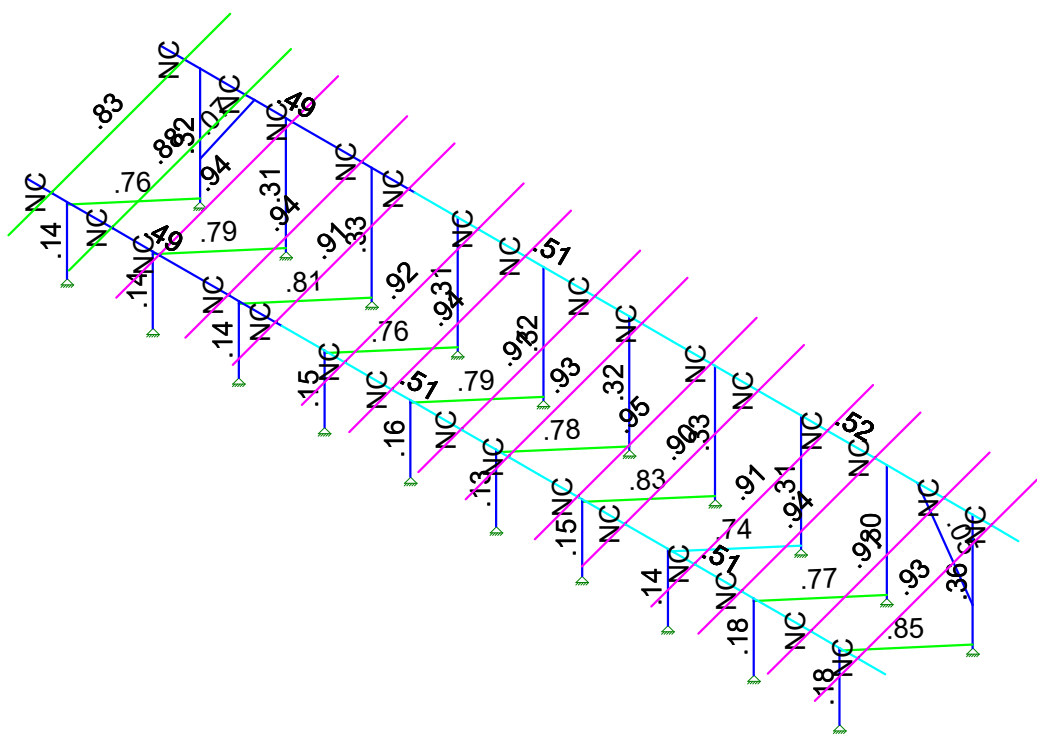


Loads: BLC 9, Wind B 90

Vector Structural Engineeri..	Ground Mount	SK - 11
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U2716.103.191		Florida D1 GM v3.r3d

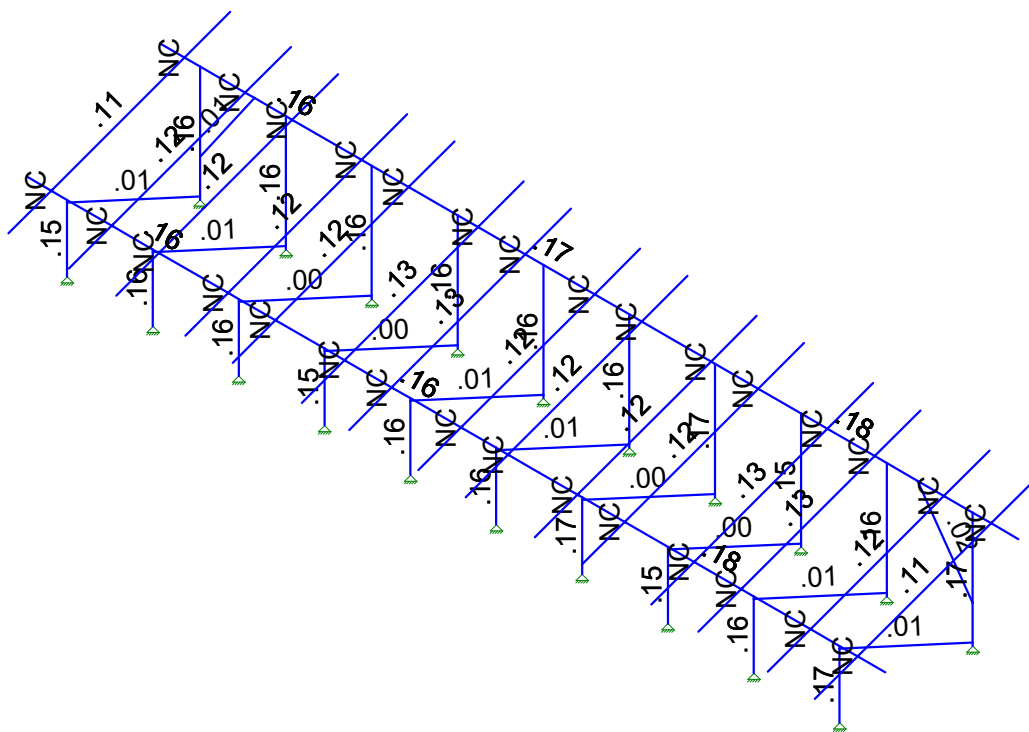


Code Check  
(Enr)  
No Calc  
> 1.0  
40-1.0  
75-90  
50-75  
0-.50



Member Code Checks Displayed (Enveloped)  
Envelope Only Solution

Vector Structural Engineeri...	Ground Mount	SK - 1
STB		July 31, 2019 at 3:04 PM
U2716.103.191		Florida D1 GM v3.r3d



Member Shear Checks Displayed (Enveloped)  
Envelope Only Solution

Vector Structural Engineeri...	Ground Mount	SK - 2
STB		July 31, 2019 at 3:04 PM
U2716.103.191		Florida D1 GM v3.r3d







Company : Vector Structural Engineering  
 Designer : STB  
 Job Number : U2716.103.191  
 Model Name : Ground Mount

July 31, 2019  
 3:07 PM  
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**Aluminum Section Sets**

	Label	Shape	Type	Design List	Material	Design R...	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	AL Posts	2.375ODX0.188	Column	Pipe	6005-T5	Typical	1.29	.778	.778	1.54
2	AL Brace	RT1.5x2x0.15625	VBrace	Rectangular Tubes	6005-T5	Typical	.996	.327	.524	.602
3	AL Rails	HR 300	Beam	Rectangular Tubes	6005-T5	Typical	.736	.214	.727	.614
4	AL Cross Beam	Cross Rail	Beam	Rectangular Tubes	6005-T5	Typical	1.909	1.97	4.366	4.017

**Member Area Loads (BLC 2 : Solar Panel Weight)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N199	N196	Y	A-B	-3

**Member Area Loads (BLC 3 : Roof Live/Snow)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N199	N196	PY	A-B	0

**Member Area Loads (BLC 4 : Wind A 0 deg)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	-59.1
2	N198	N201	N199	N196	Perp	A-B	-68.2

**Member Area Loads (BLC 5 : Wind B 0 deg)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	-101.5
2	N198	N201	N199	N196	Perp	A-B	-9.1

**Member Area Loads (BLC 6 : Wind A 180 deg)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	78.8
2	N198	N201	N199	N196	Perp	A-B	71.2

**Member Area Loads (BLC 7 : Wind B 180 deg)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	30.3
2	N198	N201	N199	N196	Perp	A-B	93.9

**Member Area Loads (BLC 8 : Wind A 90)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N203	N202	N196	Perp	A-B	36.3
2	N203	N209	N208	N202	Perp	A-B	27.3
3	N209	N200	N199	N208	Perp	A-B	13.6

**Member Area Loads (BLC 9 : Wind B 90)**

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N203	N202	N196	Perp	A-B	-36.3
2	N203	N209	N208	N202	Perp	A-B	-22.7
3	N209	N200	N199	N208	Perp	A-B	-13.6

**Basic Load Cases**

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed	Area(M...Surface...
1	Self Weight	DL		-1.05					
2	Solar Panel Weight	DL							1









Company : Vector Structural Engineering  
 Designer : STB  
 Job Number : U2716.103.191  
 Model Name : Ground Mount

July 31, 2019  
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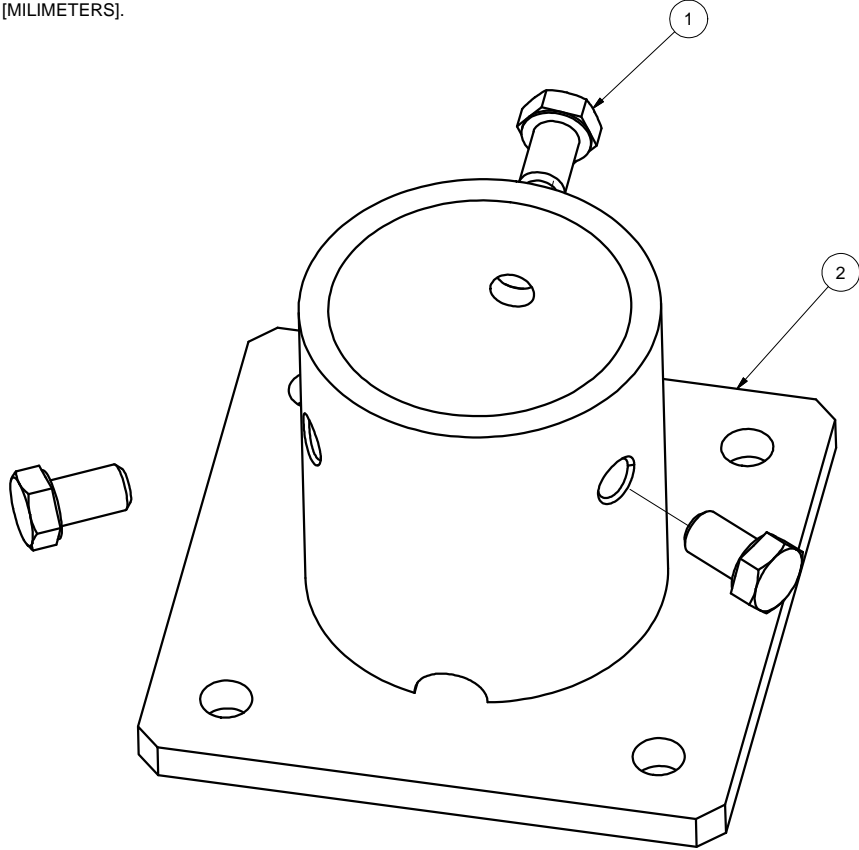
**Envelope AISC 14th(360-10): ASD Steel Code Checks (Continued)**

Member	Shape	Code Check	Loc[in]	LC Shear	Loc[in]	Dir	LC Pnc/om	Pnt/om	Mny/om	Mnz/om	Cb	Eqn	
24	M82A	Pipe 2.0 A2...	.302	1.682	5	.158	0	5	11077.698	23232.186	1397.505	1397.505	1... H1-1a
25	M93	Pipe 2.0 A2...	.176	45.051	6	.170	45.051	5	18169.568	23232.186	1397.505	1397.505	1... H1-1b
26	M94	Pipe 2.0 A2...	.356	1.682	5	.174	0	5	11077.698	23232.186	1397.505	1397.505	2... H1-1a

**Envelope AA ADM1-15: ASD - Building Aluminum Code Checks**

Member	Shape	Code C...	Loc[in]	LC Shear	Loc[in]	Dir	LC Pnc/O...	Pnt/Om...	Mny/O...	Mnz/O...	Vny/O...	Vnz/O...	Cb	Eqn
1	M15	RT1.5x2x...	.757	54.421	5	.008	0	z	16	1600.225	19411....	770.742	927.083	6090.1994101.5631... H.1-1
2	M19	RT1.5x2x...	.067	37.496	5	.008	0	z	5	3779.203	19411....	770.742	927.083	6090.1994101.5631... H.1-1
3	M16	HR 300	.829	84.234	5	.112	32.662	y	6	3322.606	14342....	494.953	934.619	6236.3642843.273 1 H.1-1
4	M82	RT1.5x2x...	.789	54.421	5	.007	102.44	y	5	1600.225	19411....	770.742	927.083	6090.1994101.5631... H.1-1
5	M52	RT1.5x2x...	.789	54.421	5	.006	102.44	y	5	1600.225	19411....	770.742	927.083	6090.1994101.5631... H.1-1
6	M58A	RT1.5x2x...	.806	54.421	5	.002	0	y	6	1600.225	19411....	770.742	927.083	6090.1994101.5631... H.1-1
7	M70	RT1.5x2x...	.765	54.421	5	.003	0	y	6	1600.225	19411....	770.742	927.083	6090.1994101.5631... H.1-1
8	M75	RT1.5x2x...	.781	54.421	5	.005	102.44	y	5	1600.225	19411....	770.742	927.083	6090.1994101.5631... H.1-1
9	M75A	RT1.5x2x...	.052	37.577	5	.016	0	z	5	3763.007	19411....	770.742	927.083	6090.1994101.5631... H.1-1
10	M78	RT1.5x2x...	.828	54.421	5	.003	0	y	6	1600.225	19411....	770.742	927.083	6090.1994101.5631... H.1-1
11	M35	HR 300	.880	85.953	5	.115	32.662	y	6	3322.606	14342....	494.953	934.619	6236.3642843.273 1 H.1-1
12	M38	HR 300	.939	130.6...	16	.122	32.662	y	6	3322.606	14342....	494.953	934.619	6236.3642843.273 1 H.1-1
13	M41	HR 300	.937	130.6...	16	.122	32.662	y	6	3322.606	14342....	494.953	934.619	6236.3642843.273 1 H.1-1
14	M44	HR 300	.907	130.6...	16	.122	32.662	y	6	3322.606	14342....	494.953	934.619	6236.3642843.273 1 H.1-1
15	M47	HR 300	.925	130.6...	16	.126	32.662	y	6	3322.606	14342....	494.953	934.619	6236.3642843.273 1 H.1-1
16	M50A	HR 300	.938	130.6...	16	.126	32.662	y	6	3322.606	14342....	494.953	934.619	6236.3642843.273 1 H.1-1
17	M53	HR 300	.909	130.6...	16	.123	32.662	y	6	3322.606	14342....	494.953	934.619	6236.3642843.273 1 H.1-1
18	M56	HR 300	.926	130.6...	16	.122	32.662	y	6	3322.606	14342....	494.953	934.619	6236.3642843.273 1 H.1-1
19	M59	HR 300	.951	130.6...	16	.123	32.662	y	6	3322.606	14342....	494.953	934.619	6236.3642843.273 1 H.1-1
20	M62	HR 300	.902	130.6...	16	.122	32.662	y	6	3322.606	14342....	494.953	934.619	6236.3642843.273 1 H.1-1
21	M65	HR 300	.913	130.6...	16	.125	32.662	y	6	3322.606	14342....	494.953	934.619	6236.3642843.273 1 H.1-1
22	M68B	HR 300	.940	130.6...	16	.126	32.662	y	6	3322.606	14342....	494.953	934.619	6236.3642843.273 1 H.1-1
23	M71A	HR 300	.908	85.953	5	.120	32.662	y	6	3322.606	14342....	494.953	934.619	6236.3642843.273 1 H.1-1
24	M74A	HR 300	.926	130.6...	16	.111	130.6...	y	16	3322.606	14342....	494.953	934.619	6236.3642843.273 1 H.1-1
25	M77A	RT1.5x2x...	.738	54.421	5	.002	102.44	y	6	1600.225	19411....	770.742	927.083	6090.1994101.5631... H.1-1
26	M83	RT1.5x2x...	.775	54.421	5	.010	0	z	5	1600.225	19411....	770.742	927.083	6090.1994101.5631... H.1-1
27	M95	RT1.5x2x...	.845	54.421	5	.005	102.44	y	4	1600.225	19411....	770.742	927.083	6090.1994101.5631... H.1-1

NOTES: UNLESS OTHERWISE SPECIFIED  
 1. DIMENSIONS SHOWN ARE INCHES [MILLIMETERS].



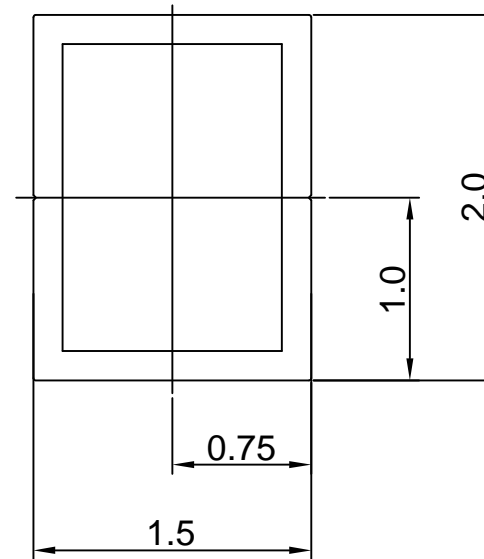
REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	10/20/2016

2	A21120-001	2" PIPE BASE	1
1	B15018-011	HEX CAP SCREW 3/8-16 X 5/8	3
ITEM	PART NUMBER	DESCRIPTION	QTY
MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		<b>Sunmodo Corp.</b> 1905 E 5TH STREET, STE A, VANCOUVER, WA 98661	
Tolerances		TITLE	
X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] X.X ±0.039 [1.0mm] Unless otherwise spec'd		2" PIPE BASE KIT	
DRAWN BY		DATE	
LWF		10/20/2016	
CHECKED BY		DRAWING NUMBER	
		B K10268-001	
APPROVALS		SCALE: NONE SHEET 1 of 1	

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NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERAIL: ALUMINUM 6005-T5.  
FINISH: CLEAR ANODIZED 10  $\mu$ m THICK.
3. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.



## Section properties:

Weight: 1.156 lbs/ft

Area: 0.992 in<sup>2</sup>

Perimeter: 12.601 in

Bounding Box: X: -1.000,1.000

Y: -0.750, 0.750

Centroid:(0.000,0.000)

Moments of Inertia(in<sup>4</sup>): I<sub>x</sub>=0.506,I<sub>y</sub>=0.322

Section modulus in bending(in<sup>3</sup>): W<sub>x</sub>=0.675,W<sub>y</sub>=0.322

Radii of Gyration: X: 0.714, Y: 0.570

MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS			
All Dimensions in inches [millimeters]			
Tolerances			
X.XXX ± 0.01 [0.25mm]		Break all sharp edges	
X.XX ± 0.02 [0.50mm]		.010-.020 unless	
X.X ± 0.039 [1.0mm]		otherwise specified.	
Unless otherwise specd			
DRAWN BY	DATE	TITLE	
zcg	03/12/2014	1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661	
CHECKED BY		B	DRAWING NUMBER
			A20164
APPROVALS		SCALE:	SHEET 1 of 1
		NONE	

**Sunmodo Corp.**

1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661

TITLE  
1.5X2 AL TUBE BRACE EXTRUSION

DRAWING NUMBER  
A20164

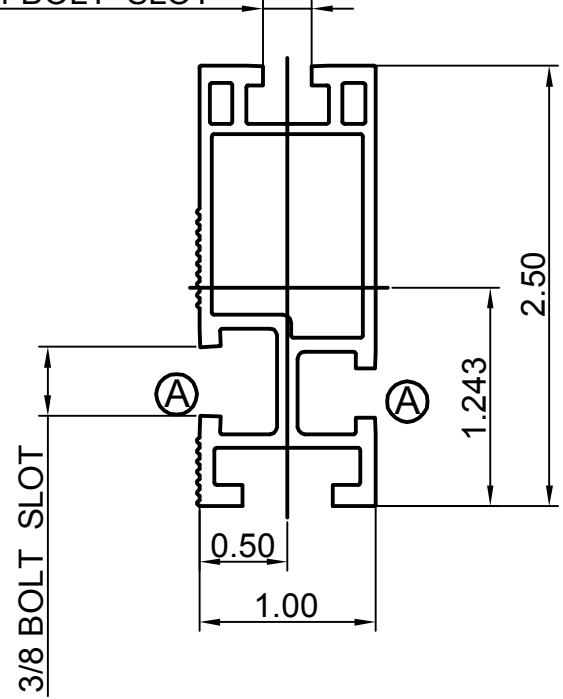
SCALE: NONE SHEET 1 of 1

REVISIONS			
REV	DESCRIPTION	BY	DATE
A	ADDED BOTTOM CHANNEL & CHANGED ONE 3/8 CHANNEL TO 1/4	zcg	02/21/2013

NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERAIL: ALUMINUM 6005-T5.  
FINISH: CLEAR ANODIZED 15  $\mu$ m THICK.
3. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.

2X 1/4 BOLT SLOT



### Section properties:

Weight: 0.850 lbs/ft  
 Area: 0.723 in<sup>2</sup>  
 Perimeter: 17.325 in  
 Bounding Box: X: -0.500,0.500  
 Y: -1.243,1.257

Centroid:(0.000,0.000)  
 Moments of Inertia(in<sup>4</sup>): I<sub>x</sub>=0.486,I<sub>y</sub>=0.095  
 Section modulus in bending(in<sup>3</sup>): W<sub>x</sub>=0.387,W<sub>y</sub>=0.190  
 Radii of Gyration: X: 0.820, Y: 0.363

MATERIAL SEE NOTES	
Third Angle Projection:	
GENERAL SPECIFICATIONS All Dimensions in inches [millimeters]	
Tolerances	
X.XXX ± 0.01 [0.25mm]	Break all sharp edges
X.XX ± 0.02 [0.50mm]	.010-.020 unless
X.X ± 0.039 [1.0mm]	otherwise specified.
Unless otherwise spec'd	
DRAWN BY zcg	DATE 02/21/2013
CHECKED BY	
APPROVALS	

<b>Sunmodo Corp.</b>	
1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661	
TITLE HELIO STANDARD RAIL	
DRAWING NUMBER B A20144	
SCALE: NONE	SHEET 1 of 1

4

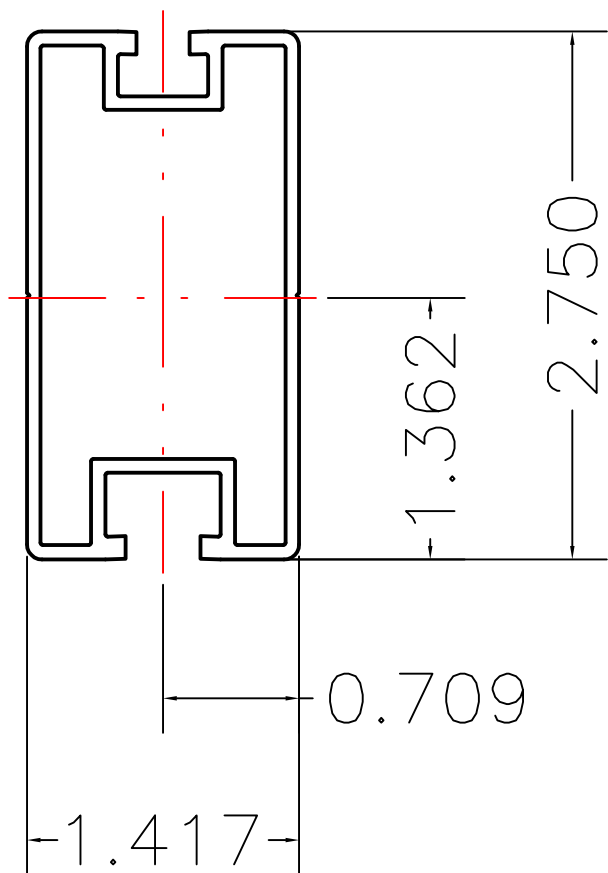
3

2

1

NOTES: UNLESS OTHERWISE SPECIFIED

- 1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
- 2. MATERIAL: 6005-T5.  
FINISH: CLEAR ANODIZED 10um THICK.
- 3. THE UNSPECIFIED RADII ARE .02" MAX.
- 4. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.



### Section properties:

Weight: 0.862 lbs/ft  
 Area: 0.736 in<sup>2</sup>  
 Perimeter: 19.824 in  
 Bounding Box: X: -0.709,0.709  
                   Y: -1.362,1.388  
 Centroid:(0.000,0.000)  
 Moments of Inertia(in<sup>4</sup>): I<sub>x</sub>=0.727,I<sub>y</sub>=0.214  
 Section modulus in bending(in<sup>3</sup>): W<sub>x</sub>=0.524,W<sub>y</sub>=0.302  
 Radii of Gyration: X: 0.994, Y: 0.539

MATERIAL SEE NOTES	
Third Angle Projection:	
GENERAL SPECIFICATIONS All Dimensions in inches [millimeters]	
Tolerances	
X.XXX ±0.01 [0.25mm]	Break all sharp edges .010-.020 unless otherwise specified.
X.XX ±0.02 [0.50mm]	
X.X ±0.039 [1.0mm]	
Unless otherwise spec'd	
DRAWN BY KYY	DATE 01/18/2018
CHECKED BY	
APPROVALS	

<b>SunModo Corp.</b>	
14800 NE 65TH STREET, VANCOUVER WA 98682	
TITLE RAIL, HR300 (SUNRAY), EXTRUSION	
B	DRAWING NUMBER A20288
SCALE: NONE	SHEET 1 of 1

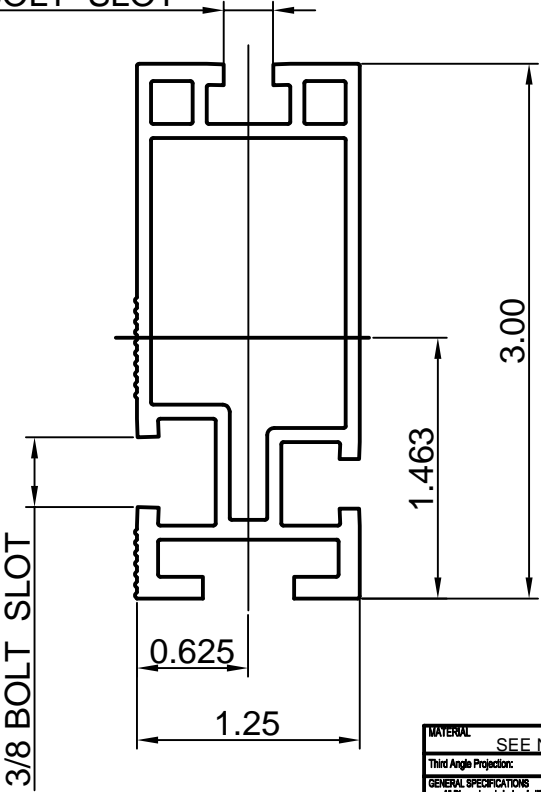
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REV	DESCRIPTON	BY	DATE
A	0.44 WAS 0.41, 0.44 WAS 0.33	LWF	11/30/2015

NOTES: UNLESS OTHERWISE SPECIFIED

- 1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
- 2. MATERIAL: ALUMINUM 6005-T5.  
FINISH: CLEAR ANODIZED 15 μm THICK.
- 3. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.

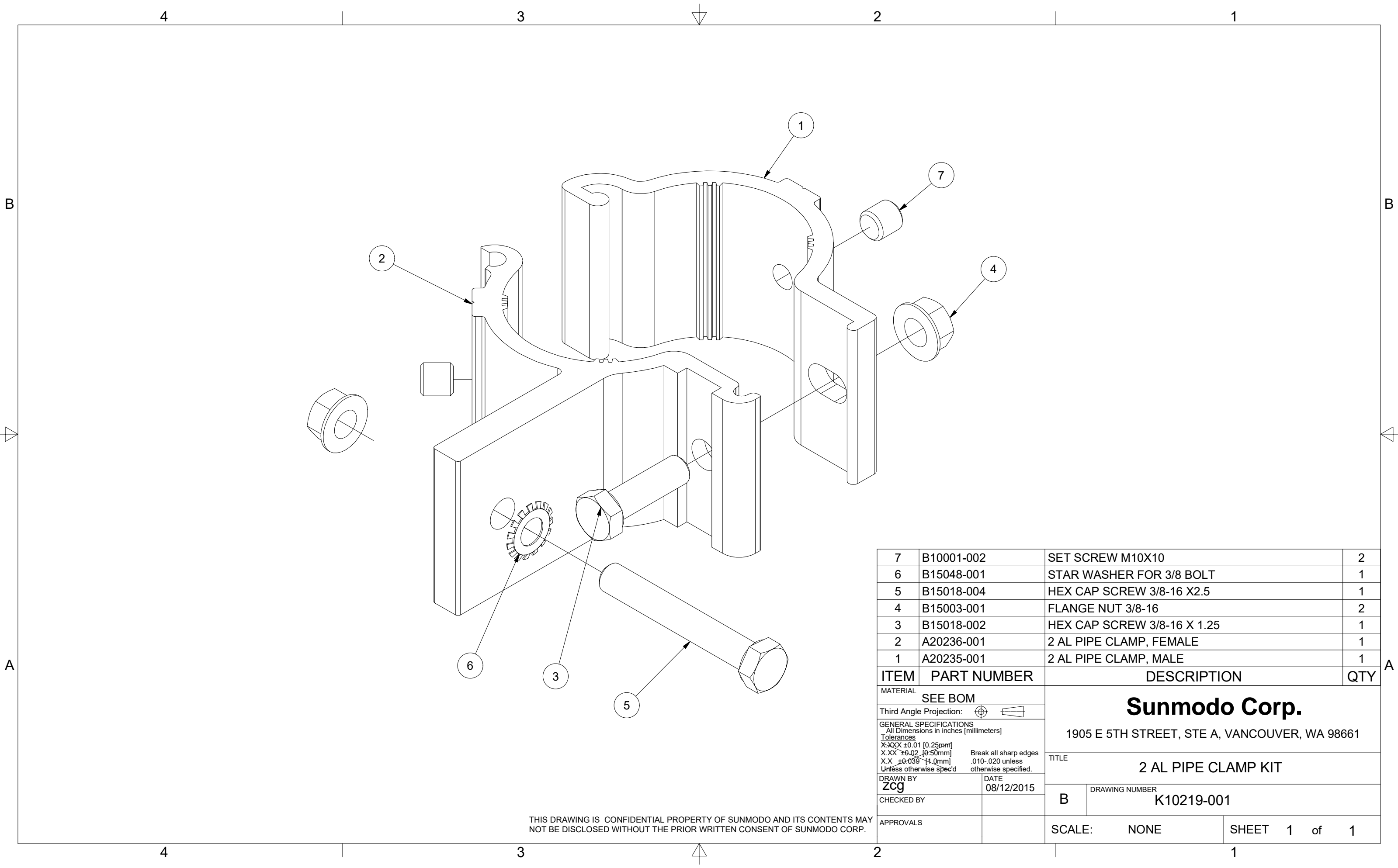
2X 1/4 BOLT SLOT



**Section properties:**

Weight: 1.151 lbs/ft  
 Area: 0.980 in<sup>2</sup>  
 Perimeter: 22.104 in  
 Bounding Box: X: -0.625,0.625  
                   Y: -1.463,1.537  
 Centroid:(0.000,0.000)  
 Moments of Inertia(in<sup>4</sup>): Ix=1.047,Iy=0.207  
 Section modulus in bending(in<sup>3</sup>): Wx=0.681,Wy=0.331  
 Radii of Gyration: X: 1.034, Y: 0.460

MATERIAL		SEE NOTES	
Third Angle Projection			
GENERAL SPECIFICATIONS			
All Dimensions in Inches [millimeters]			
Tolerances			
XXX ±0.01 (0.25mm)			
XX ±0.02 (0.50mm)			
X ±0.03 (1.0mm)			
Unless otherwise specified.			
Break all sharp edges 0.10-0.25 unless otherwise specified.			
DRAWN BY		DATE	
ZCJ		02/21/2015	
CHECKED BY		DATE	
APPROVALS		DATE	
Sunmodo Corp.		1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661	
TITLE		HELIO HEAVY RAIL	
DRAWING NUMBER		A20145	
SCALE:		NONE	
SHEET		1 of 1	



7	B10001-002	SET SCREW M10X10	2
6	B15048-001	STAR WASHER FOR 3/8 BOLT	1
5	B15018-004	HEX CAP SCREW 3/8-16 X2.5	1
4	B15003-001	FLANGE NUT 3/8-16	2
3	B15018-002	HEX CAP SCREW 3/8-16 X 1.25	1
2	A20236-001	2 AL PIPE CLAMP, FEMALE	1
1	A20235-001	2 AL PIPE CLAMP, MALE	1

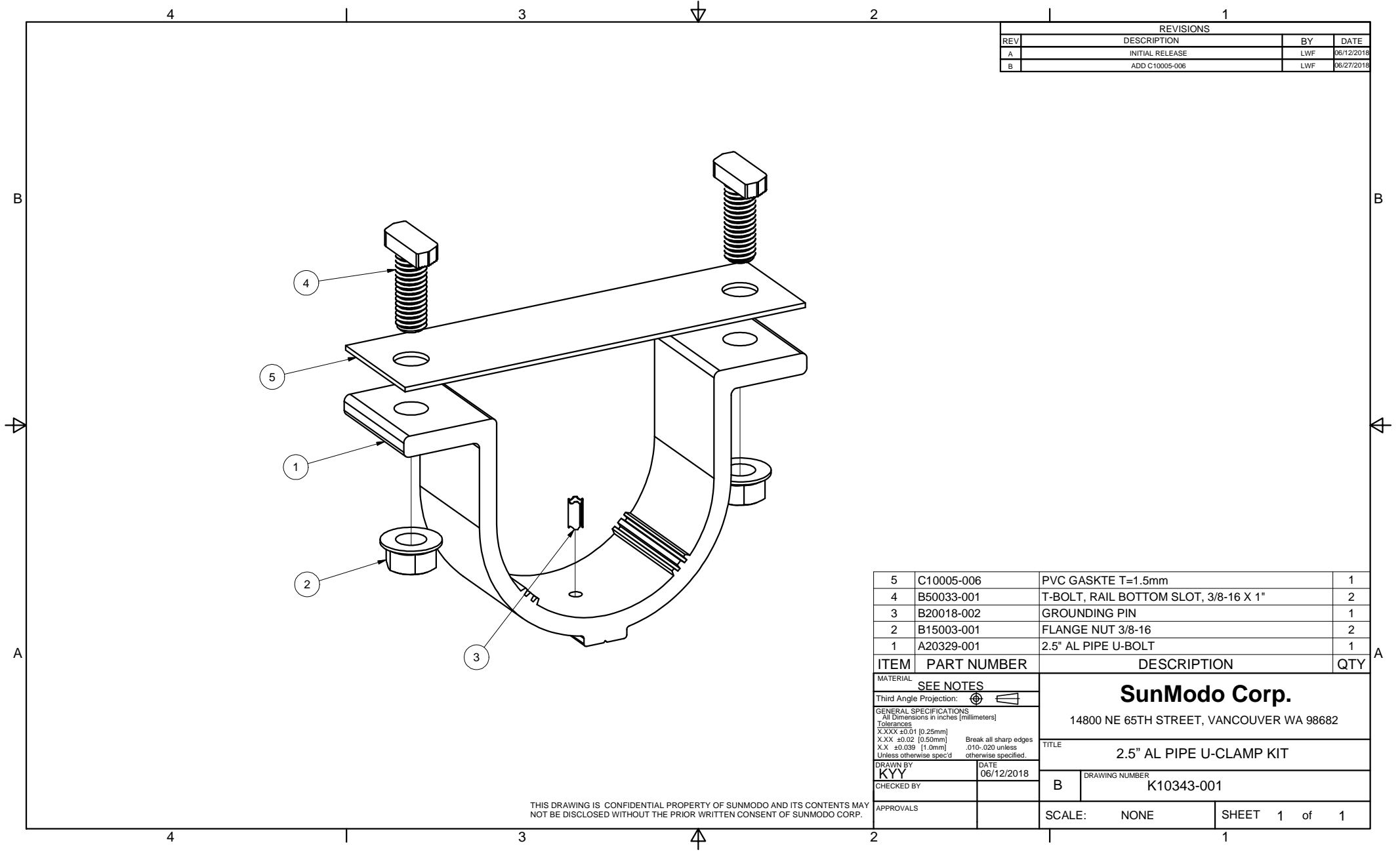
ITEM	PART NUMBER	DESCRIPTION	QTY
MATERIAL		SEE BOM	
Third Angle Projection:			
GENERAL SPECIFICATIONS All Dimensions in inches (millimeters)			
Tolerances			
X.XXX ±0.01 [0.25mm]		Break all sharp edges	
X.XX ±0.02 [0.50mm]		.010-.020 unless	
X.X ±0.039 [1.0mm]		otherwise specified.	
DRAWN BY		DATE	
zcg		08/12/2015	
CHECKED BY		B	
APPROVALS		DRAWING NUMBER	
		K10219-001	
SCALE:		NONE	SHEET 1 of 1

**Sunmodo Corp.**  
1905 E 5TH STREET, STE A, VANCOUVER, WA 98661

TITLE  
**2 AL PIPE CLAMP KIT**

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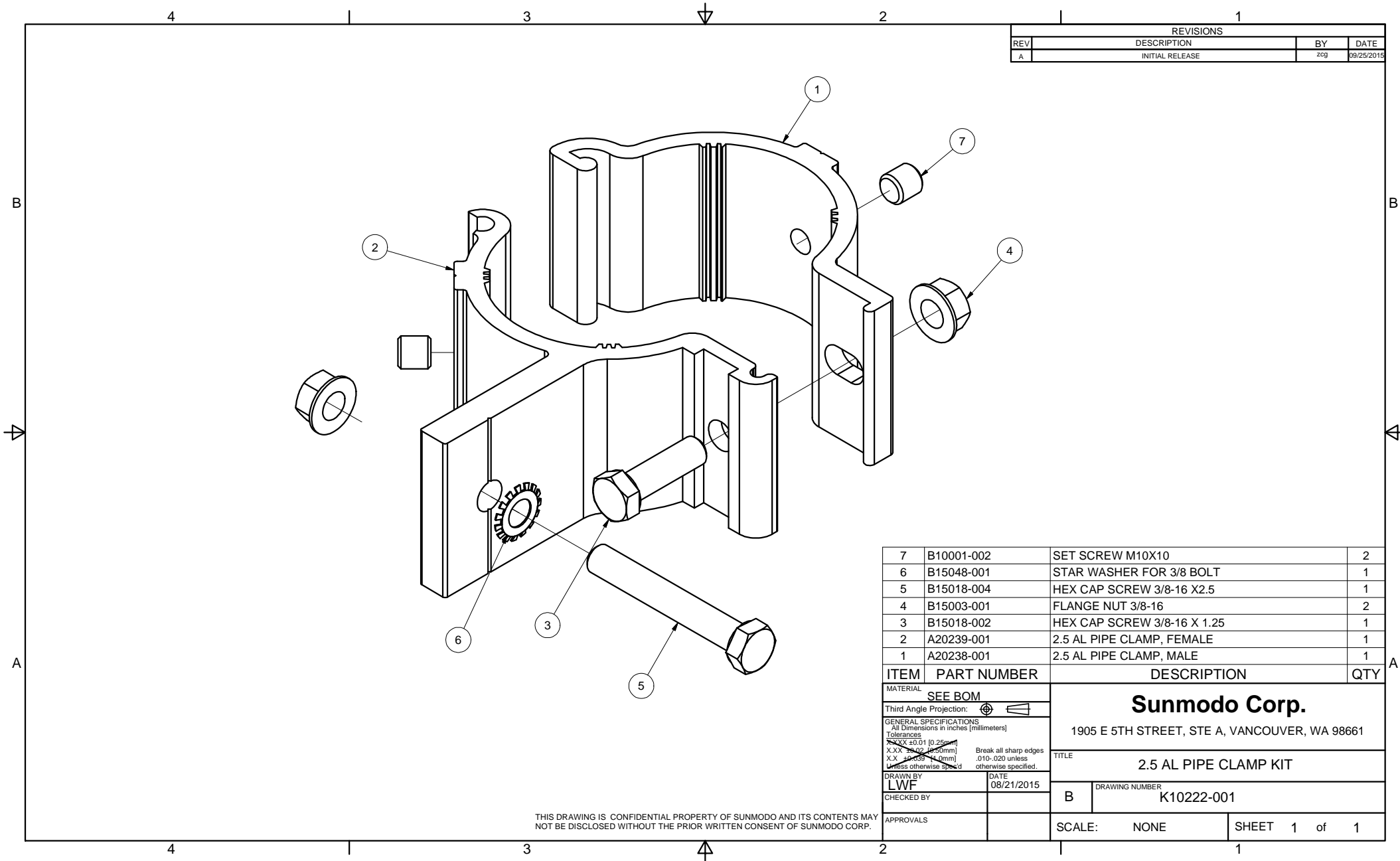


REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	06/12/2018
B	ADD C10005-006	LWF	06/27/2018

ITEM	PART NUMBER	DESCRIPTION	QTY
5	C10005-006	PVC GASKTE T=1.5mm	1
4	B50033-001	T-BOLT, RAIL BOTTOM SLOT, 3/8-16 X 1"	2
3	B20018-002	GROUNDING PIN	1
2	B15003-001	FLANGE NUT 3/8-16	2
1	A20329-001	2.5" AL PIPE U-BOLT	1

MATERIAL		<b>SEE NOTES</b>	
Third Angle Projection:			
GENERAL SPECIFICATIONS		<p><b>SunModo Corp.</b> 14800 NE 65TH STREET, VANCOUVER WA 98682</p>	
<p>All Dimensions in inches [millimeters]</p> <p>Tolerances</p> <p>X.XXX ±0.01 [0.25mm]</p> <p>X.XX ±0.02 [0.50mm]</p> <p>X.X ±0.039 [1.0mm]</p> <p>Unless otherwise spec'd</p>			
TITLE		2.5" AL PIPE U-CLAMP KIT	
DRAWN BY	DATE	DRAWING NUMBER	
KYY	06/12/2018	B K10343-001	
CHECKED BY			
APPROVALS		SCALE: NONE	SHEET 1 of 1

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REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	zcg	09/25/2015

ITEM	PART NUMBER	DESCRIPTION	QTY
7	B10001-002	SET SCREW M10X10	2
6	B15048-001	STAR WASHER FOR 3/8 BOLT	1
5	B15018-004	HEX CAP SCREW 3/8-16 X2.5	1
4	B15003-001	FLANGE NUT 3/8-16	2
3	B15018-002	HEX CAP SCREW 3/8-16 X 1.25	1
2	A20239-001	2.5 AL PIPE CLAMP, FEMALE	1
1	A20238-001	2.5 AL PIPE CLAMP, MALE	1

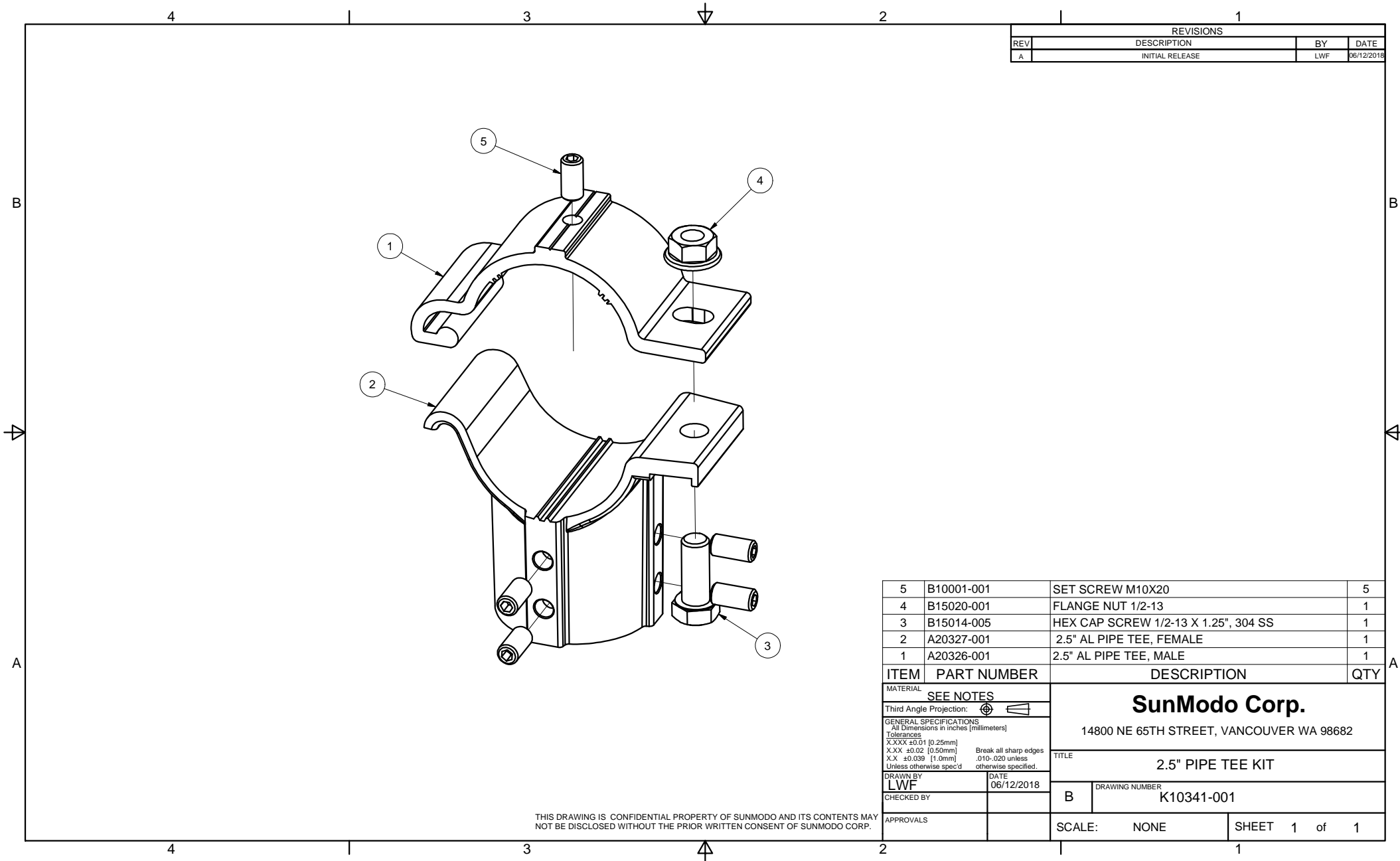
MATERIAL		<b>SEE BOM</b>	
Third Angle Projection:			
<b>GENERAL SPECIFICATIONS</b> All Dimensions in inches [millimeters] <b>Tolerances</b> X.XX ±0.01 (0.25mm) X.X ±0.02 (0.5mm) X.X ±0.03 (0.75mm) Unless otherwise specified.			
DRAWN BY		DATE	
LWF		08/21/2015	
CHECKED BY		B	
APPROVALS		SCALE: NONE	
		SHEET 1 of 1	

**Sunmodo Corp.**  
 1905 E 5TH STREET, STE A, VANCOUVER, WA 98661

TITLE: **2.5 AL PIPE CLAMP KIT**

DRAWING NUMBER: **K10222-001**

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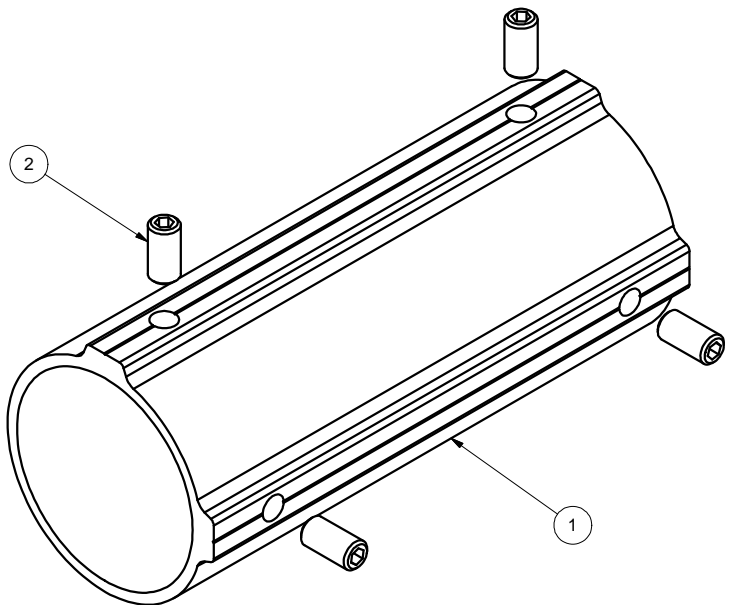
REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	06/12/2018

ITEM	PART NUMBER	DESCRIPTION	QTY
5	B10001-001	SET SCREW M10X20	5
4	B15020-001	FLANGE NUT 1/2-13	1
3	B15014-005	HEX CAP SCREW 1/2-13 X 1.25", 304 SS	1
2	A20327-001	2.5" AL PIPE TEE, FEMALE	1
1	A20326-001	2.5" AL PIPE TEE, MALE	1

MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		<b>SunModo Corp.</b> 14800 NE 65TH STREET, VANCOUVER WA 98682	
Tolerances		TITLE	
X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] X.X ±0.039 [1.0mm] Unless otherwise spec'd		2.5" PIPE TEE KIT	
DRAWN BY		DATE	
LWF		06/12/2018	
CHECKED BY		DRAWING NUMBER	
		B K10341-001	
APPROVALS		SCALE: NONE SHEET 1 of 1	

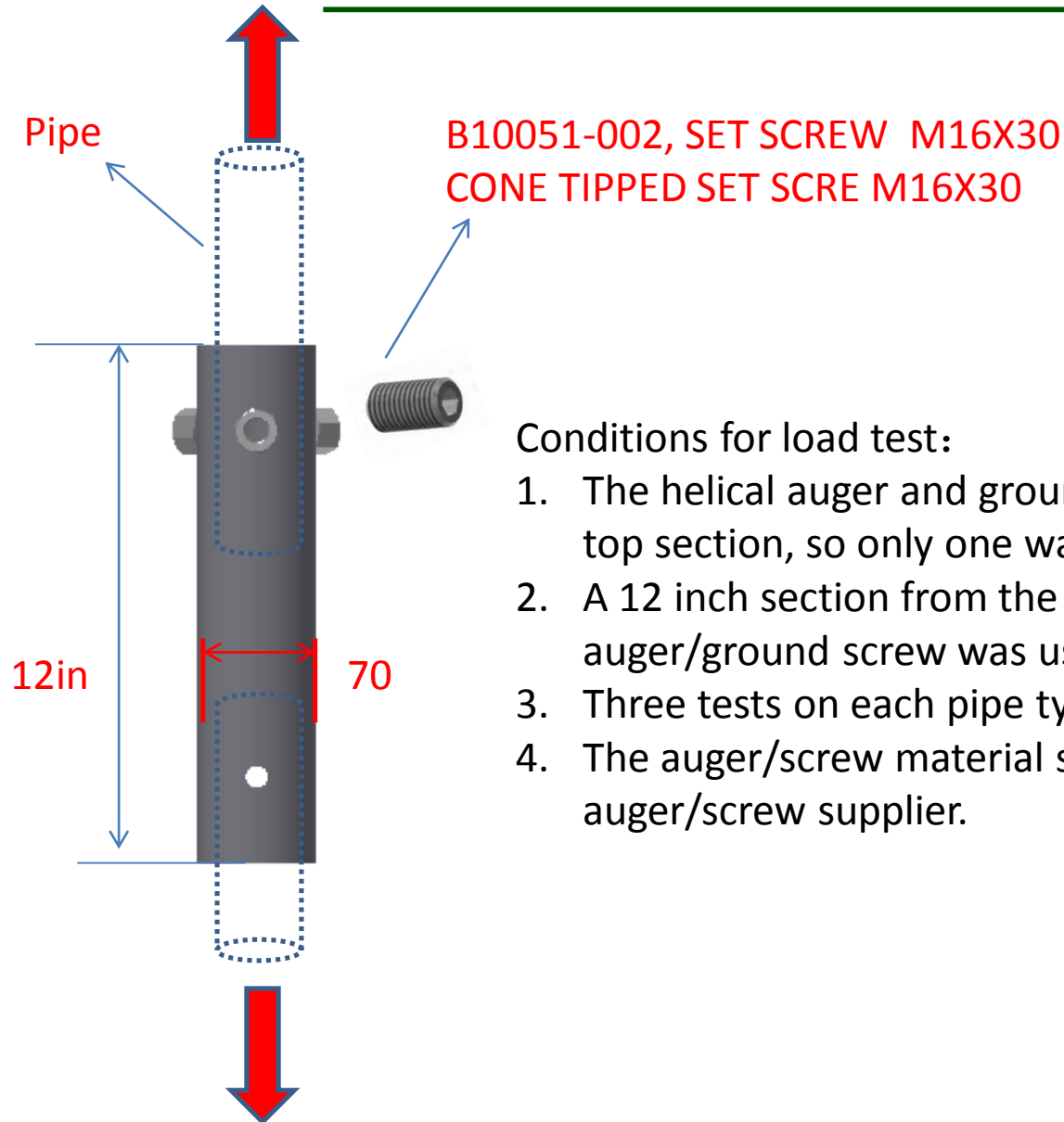
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REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	06/12/2018



2	B10001-001	SET SCREW M10X20	4
1	A20328-001	2.5" PIPE SPLICE	1
ITEM	PART NUMBER	DESCRIPTION	QTY
MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		<b>SunModo Corp.</b> 14800 NE 65TH STREET, VANCOUVER WA 98682	
All Dimensions in inches [millimeters] Tolerances X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] X.X ±0.039 [1.0mm] Unless otherwise spec'd		Break all sharp edges .010-.020 unless otherwise specified.	
DRAWN BY		TITLE	
LWF		2.5" PIPE SPLICE KIT	
CHECKED BY		DRAWING NUMBER	
		B K10342-001	
APPROVALS		SCALE: NONE SHEET 1 of 1	

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### Conditions for load test:

1. The helical auger and ground screw use the same top section, so only one was used for testing
2. A 12 inch section from the top of the auger/ground screw was used
3. Three tests on each pipe type was used
4. The auger/screw material supplied by the auger/screw supplier.