



Project Number: U2716-095-191

August 6, 2019

Sunmodo
14800 NE 65th Street
Vancouver, WA 98682

**REFERENCE: Sunmodo Sunturf Ground Mount B2
Ground Mount PV Array Installation**

To Whom It May Concern:

Per request of Sunmodo, we have been asked to prepare the structural design of a ground-mounted PV solar array system with several foundation options as shown in the attached calculations. The adopted building code in this jurisdiction is the 2009 International Building Code. Vector Structural Engineering requires that we review each site specific install, and we are not liable for installs at site specific locations we have not reviewed. The following design parameters are used in our analysis:

- Minimum Design Loads for Buildings and Other Structures (ASCE 7-05)
- Design wind speed for occupancy category I structures: 90 mph
- Importance Factor: 0.77
- Wind exposure: C
- Ground snow load: 30 psf
- The ground screws and helical piers must be tested to 1.5 times uplift and 2.0 times lateral reactions found in the table below. A minimum of one ground screw or helical pier must be tested.

Load (ASD)	Value (lbs)	Factor of Safety	Test Value (lbs)
UPLIFT	2480	1.5	3960
LATERAL	1620	2	3370

Foundation concrete shall have a minimum compressive strength of 2500 psi at 28 days. Cement for all concrete shall be Type I or II with a minimum of 6% entrained air with a water/cement ratio of 0.50. Maximum aggregate size shall be 3/4". No special inspection of concrete strength is required.

Footings are designed based on an allowable soil bearing pressure of 1500 psf an allowable skin friction of 250 psf, an allowable lateral bearing pressure of 150 pcf, and a coefficient of friction of 0.3. Vector Structural Engineering strongly recommends independent soils testing be performed by a licensed geotechnical engineer to verify the assumed soil parameters.

All ground mounts are to be installed per manufacturer's recommendations. The use of solar panel support span tables provided by the manufacturer is allowed only where the site conditions and solar panel configuration match the description of the span tables. Electrical engineering is beyond our scope. All work performed must be in accordance with accepted industry-wide methods and applicable safety standards. Vector Structural Engineering assumes no responsibility for improper installation of the solar panels.

Very truly yours,

VECTOR STRUCTURAL ENGINEERING, LLC

Clifton Palmer, P.E.
License: 51669 - Expires: 10/31/2021
Project Engineer

Enclosures

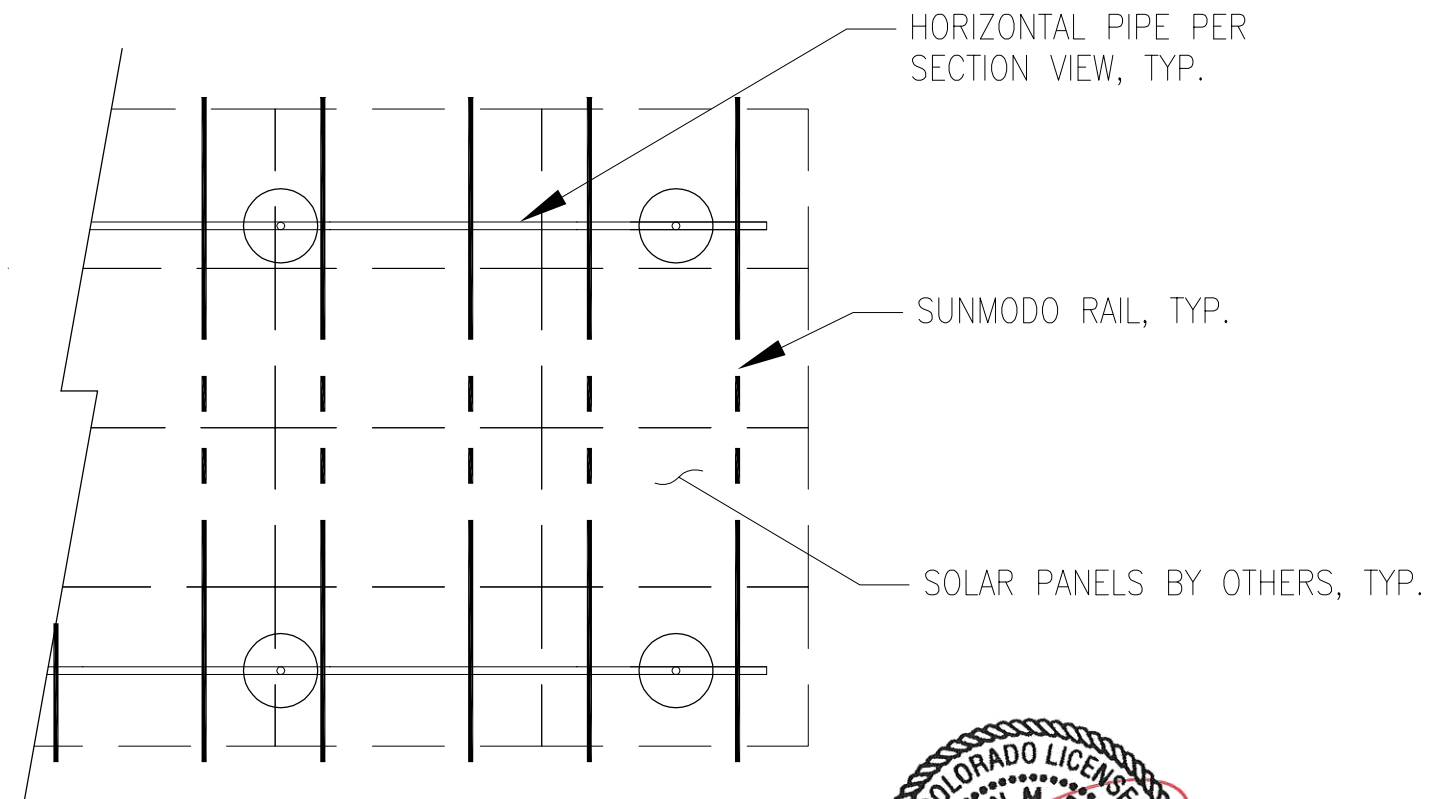
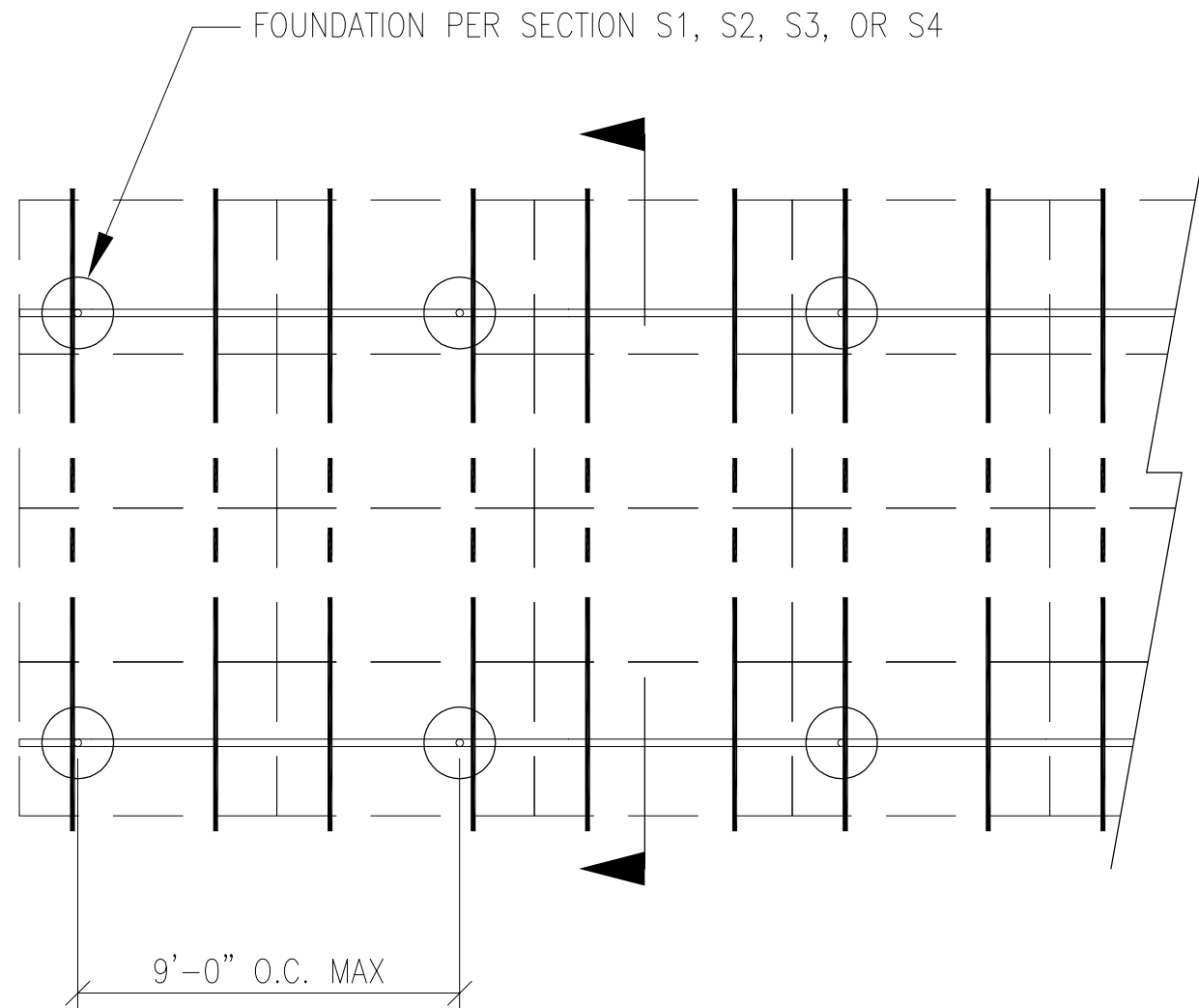
CMP/stb





JOB NO. U2716-095-191
 PROJECT SUNMODO SUNTURF GROUND MOUNTS B2
 SUBJECT ALL OPTIONS

651 W GALENA PARK BLVD. #101 (801) 990-1775
 DRAPER, UTAH 84020 (801) 990-1776 FAX



PV ARRAY PLAN

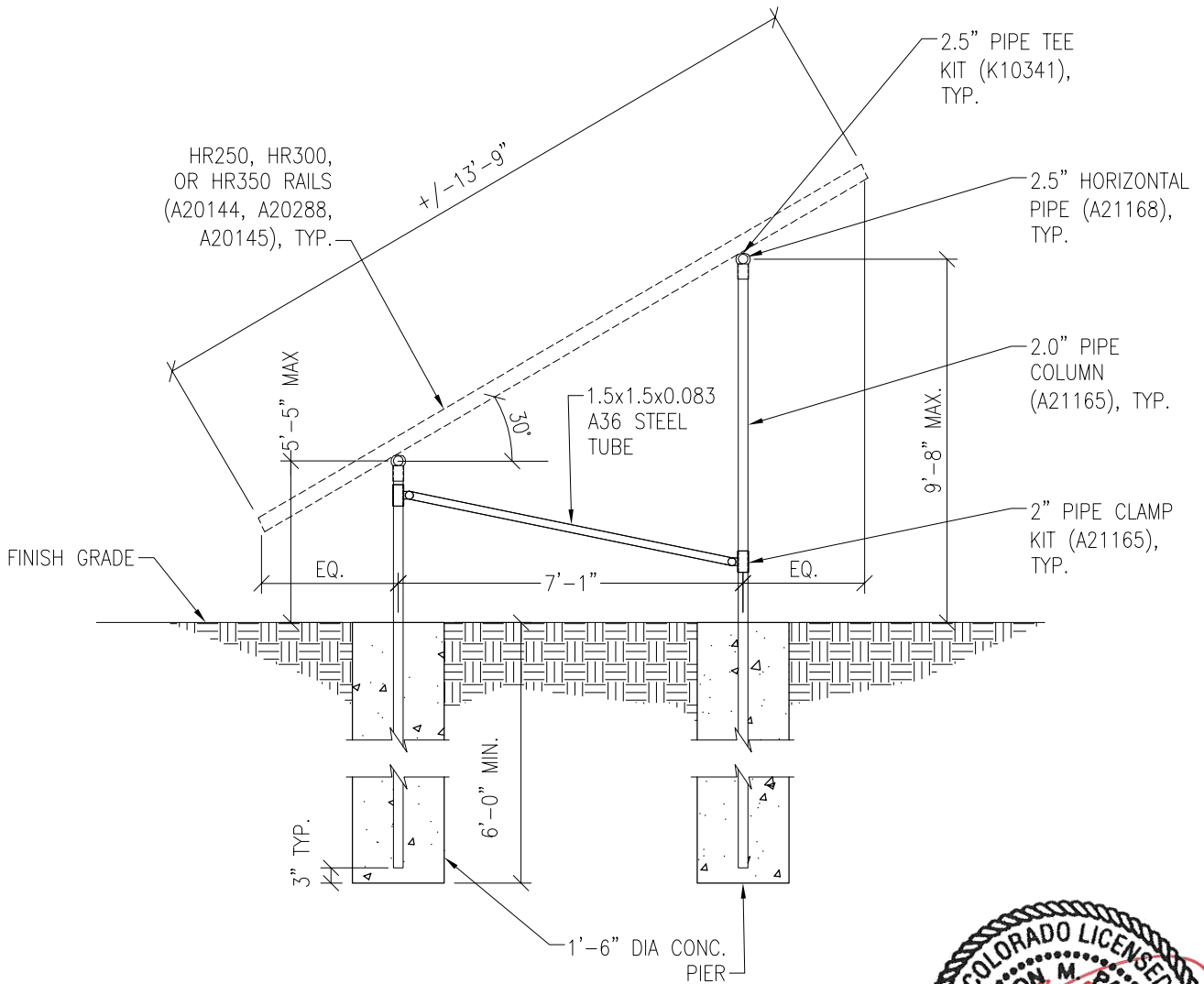
N.T.S.

P1

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PROJECT SUNMODO SUNTURF GROUND MOUNTS B2

SUBJECT DRILLED PIER OPTION



Vector Engineers requires that we review each site specific install and is not liable for installs at site specific locations we have not reviewed.

PV ARRAY SECTION

N.T.S.

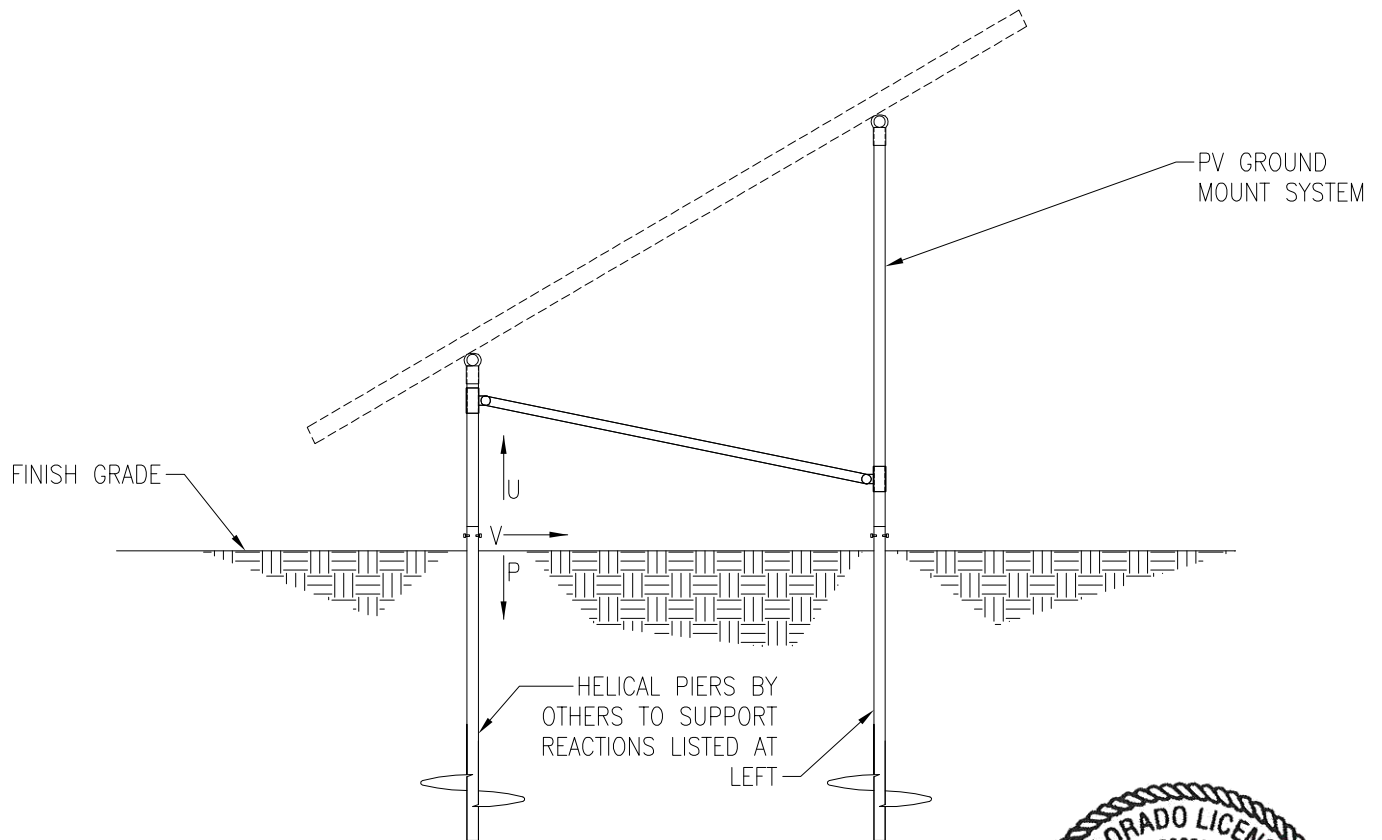
S1

PROJECT SUNMODO SUNTURF GROUND MOUNTS B2

SUBJECT HELICAL PIER OPTION

NOTES:

1. For ground mount components see Section S1.
2. A minimum of (1) helical pier must be load-tested as follows:
 - 2.1. Safety factor for uplift = 1.5,
 - 2.2. Safety factor for lateral loads = 2.0
 - 2.3. Upward deflection limit = 1/2"
 - 2.4. Lateral deflection limit = 1"
 - 2.5. The load tests must be performed by an approved contractor.



MAXIMUM REACTIONS:	
U	= 2,480 LBS
P	= 3,100 LBS
V	= 1,620 LBS



PV ARRAY SECTION

N.T.S.

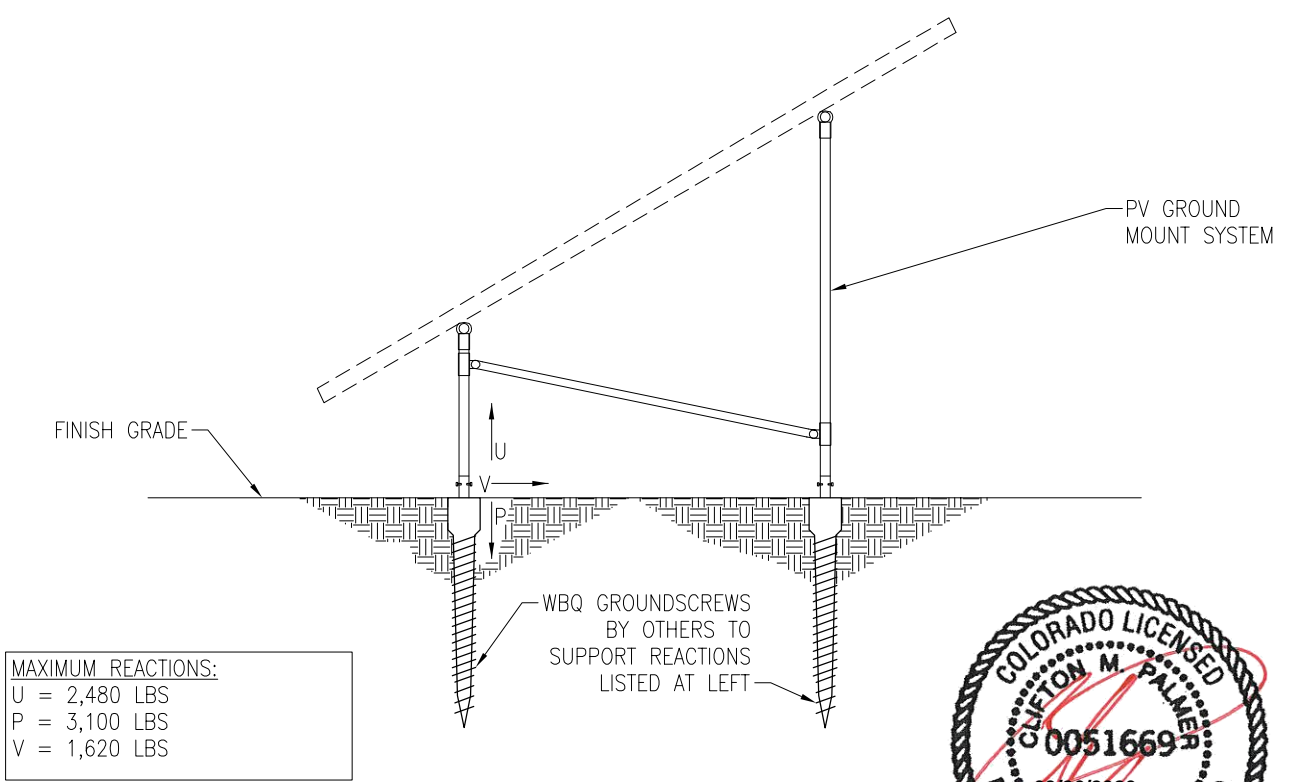
S2

PROJECT SUNMODO SUNTURF GROUND MOUNTS B2

SUBJECT GROUND SCREW OPTION

NOTES:

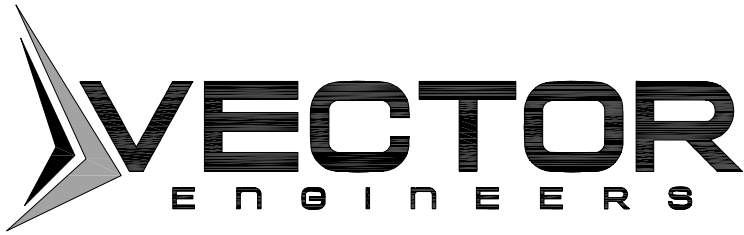
1. For ground mount components see Section S1.
2. A minimum of (1) ground screw must be load-tested as follows:
 - 2.1. Safety factor for uplift = 1.5,
 - 2.2. Safety factor for lateral loads = 2.0
 - 2.3. Upward deflection limit = 1/2"
 - 2.4. Lateral deflection limit = 1"
 - 2.5. The load tests must be performed by an approved contractor.



PV ARRAY SECTION

N.T.S.

S3



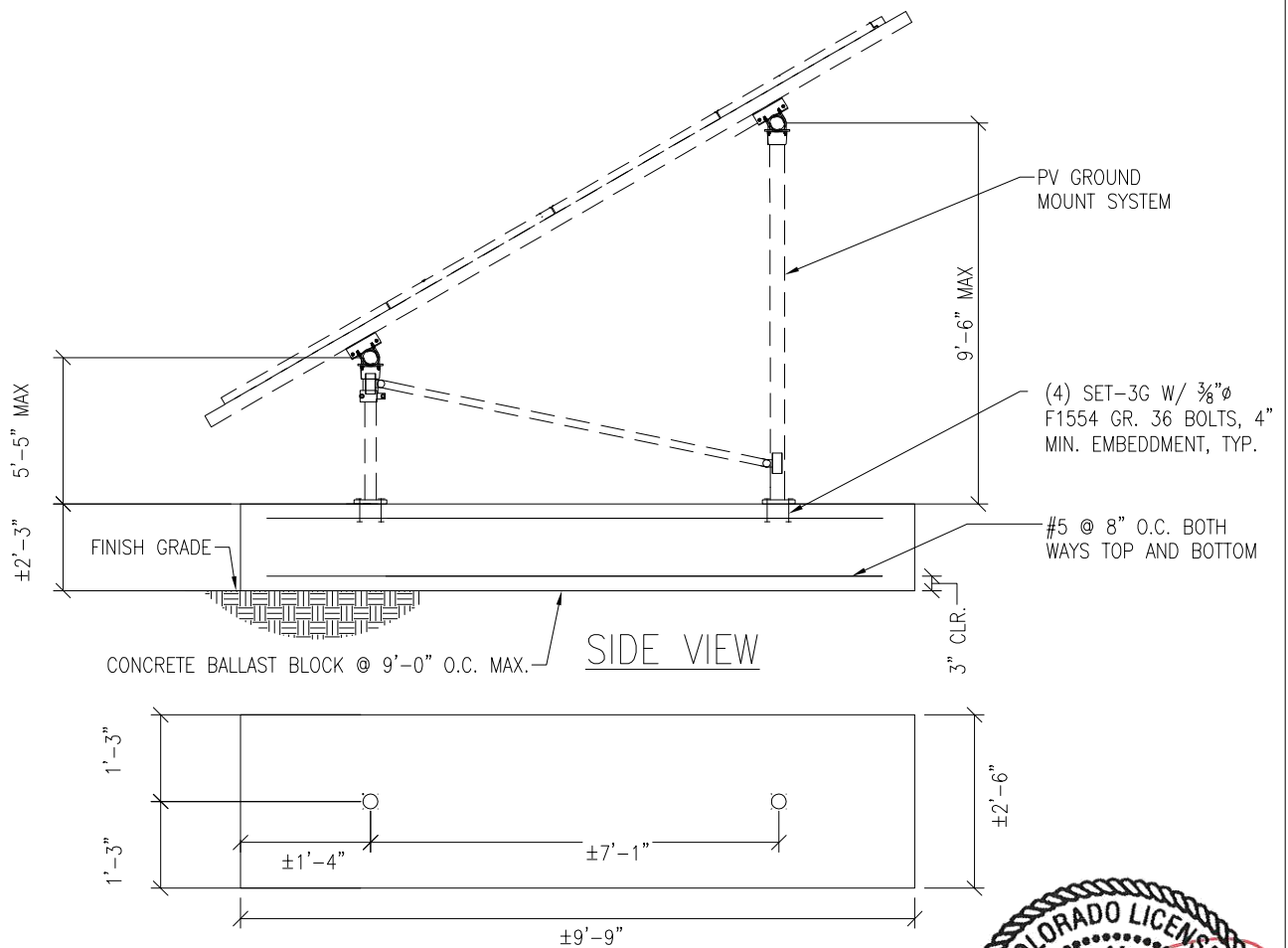
JOB NO. U2716-095-191

PROJECT SUNMOD0 SUNTURF GROUND MOUNTS B2

SUBJECT BALLASTED BLOCK OPTION

NOTES:

1. For ground mount components see Section S1.



Vector Engineers requires that we review each site specific install and is not liable for installs at site specific locations we have not reviewed.

PV ARRAY SECTION

N.T.S.

S4

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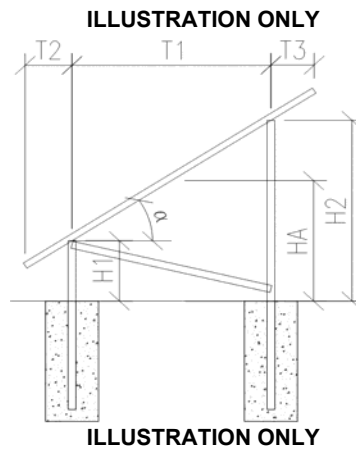
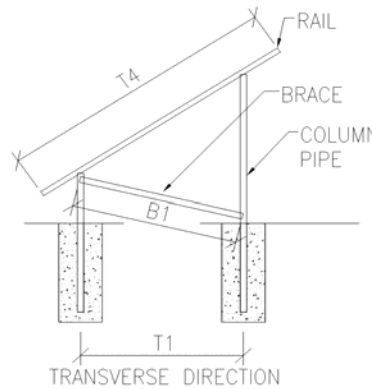
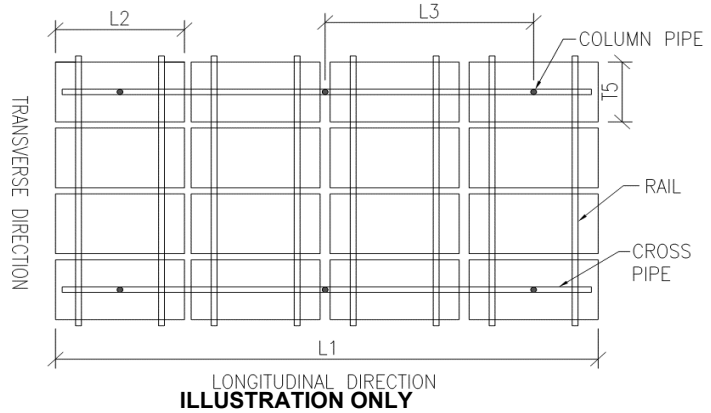
WWW.VECTORSE.COM

PROJECT: B2 – Sunmodo Sunturf GM

SUBJECT: Dead Load

Design Weight:

Individual Panel Weight [lb]:	50.7
Panel Transverse Length (T5) [in]:	41.2
Panel Transverse Length (T5) [ft]:	3.4
Panel Longitudinal Length (L2) [in]:	81.4
Panel Longitudinal Length (L2) [ft]:	6.8
Individual Panel Area [ft ²]:	23.3
Individual Panel Weight [psf]:	2.2
# of Panels in Transverse Direction:	4
Approximate Transverse Length (T4) [ft]:	13.7
# of Panels in Longitudinal Direction:	12
Approximate Longitudinal Length (L1) [ft]:	81.4
Transverse Column Spacing (T1) [ft]:	7.1
Longitudinal Column Spacing (L3) [ft]:	9.0
# of Columns in Longitudinal Direction:	10
# of Columns in Transverse Direction:	2
Total Number of Columns:	20
Panel Slope from Horizontal (a) [°]:	30.0
Short Column Height (H1) [ft]:	5.4
Approximate Tall Column Height (H2) [ft]:	9.5
Transverse Brace between Columns :	Yes
Approximate Brace Length (B1) [ft]:	9.0
Weight of Columns [plf]:	3.7
Weight of Cross Pipe [plf]:	3.7
Weight of Brace [plf]:	3.7
Tributary Transverse Length per Column [ft]:	6.9
Tributary Longitudinal Length per Column [ft]:	9.0
Tributary Area per Column [ft ²]:	61.8
Rail Weight [plf]:	1.0
Transverse Rail Weight per Column [lb]:	27.5
Longitudinal Rail Weight per Column [lb]:	32.9
Tall Column Weight [lb]:	34.7
Panel Weight per Column [lb]:	134.5
Rail Weight per Column [lb]:	27.5
Cross Pipe Weight per Column [lb]:	32.9
Brace Weight per Column [lb]:	16.4
Total Weight per Column (1.0 D) [lb]:	246.0



Assumptions:

- T2 = T3



JOB NO.: U2716-095-191

DESIGNED: STB

DATE: 07/26/19

PROJECT: B2 – Sunmodo Sunturf GM

SUBJECT: Snow Load

SNOW LOAD (S):

ASCE 7 Standard:	05	
Panel Slope from Horizontal [°]:	30.0	
Snow Ground Load, p_g [psf]:	30.0	(Section 7.2)
Terrain Category:	C	(Table 7-2)
Exposure of Roof:	Fully Exposed	(Table 7-2)
Exposure Factor, C_e :	0.9	(Table 7-2)
Thermal Factor, C_t :	1.2	(Table 7-3)
Occupancy Category:	I	(Table 1-1)
Importance Factor, I_s :	0.8	(Table 7-4)
Flat Roof Snow Load, p_f [psf]:	18	(Equation 7-1)
Minimum Roof Snow Load, p_m [psf]:	0	(Section 7.3.4)
Unobstructed Slippery Surface?	Yes	(Section 7.4)
Slope Factor Figure:	Figure 7-2c	(Section 7.4)
Roof Slope Factor, C_s :	0.727	(Figure 7-2)
Sloped Roof Snow Load, p_s [psf]:	13	(Equation 7-2)
Design Snow Load, S [psf]:	13	
Tributary Transverse Length [ft]:	5.9	
Tributary Longitudinal Length [ft]:	9	
Tributary Area per Column [ft ²]:	53.5	
Snow Load per Column (1.0 S) [lb]:	706.2	



PROJECT: B2 – Sunmodo Sunturf GM

SUBJECT: Wind Pressure

Design Wind Load:

ASCE 7 Standard:	05	
Basic Wind Speed, V [mph]:	90	
Occupancy Category:	I	
Importance, I _w :	0.87	
Exposure Category	C	(Section 6.5.6.3)
Velocity Pressure Exposure Coefficient, K _z :	0.85	(Table 6-3)
Topographic Factor, K _{zt} :	1.0	(Section 6.5.7.2)
Wind Directionality Factor, K _d :	0.85	(Table 6-4)
Internal Pressure Coefficient, GC _{pi} :	0.00	(Figure 6-5)
Velocity Pressure, q _h [psf]:	13.0	(Equation 6-15)
Gust Effect Factor, G:	0.85	(Section 6.5.8)
Panel Slope [degrees]:	30.0	

Net Pressure Coefficients (C_N) per: (Figure 6-18A)

Clear Wind Flow	C _{NW}	C _{NL}
Case 1 (γ = 0°, Load Case A)	-1.80	-1.80
Case 2 (γ = 0°, Load Case B)	-2.50	-0.50
Case 3 (γ = 180°, Load Case A)	2.10	2.10
Case 4 (γ = 180°, Load Case B)	2.60	1.00

Design Wind Pressures (p) [psf] per: (Equation 6-25)

Clear Wind Flow	q _h GC _{NW}	q _h GC _{NL}
Case 1 (γ = 0°, Load Case A)	-19.9	-19.9
Case 2 (γ = 0°, Load Case B)	-27.7	-5.5
Case 3 (γ = 180°, Load Case A)	23.3	23.3
Case 4 (γ = 180°, Load Case B)	28.8	11.1

Wind Pressure on Each Side of Panels [psf]

Clear Wind Flow	Short Col. Pressure	Long Col. Pressure
Case 1 (γ = 0°, Load Case A)	-19.9	-19.9
Case 2 (γ = 0°, Load Case B)	-5.5	-27.7
Case 3 (γ = 180°, Load Case A)	23.3	23.3
Case 4 (γ = 180°, Load Case B)	28.8	11.1



JOB NO.: U2716-095-191

PROJECT: B2 – Sunmodo Sunturf GM

SUBJECT: Open Building Wind Loads

Design Wind Load Per ASCE 7-05

$$p = q_h G C_n$$

Velocity Pressure Exposure Coefficient, K_{zt} :	0.85	(Table 6-3)
Topographic Factor, K_{ht} :	1.0	(Section 6.5.7.2)
Wind Directionality Factor, K_d :	0.85	(Table 6-4)
Ultimate Wind Speed, V [mph]:	90	

Velocity Pressure, q_h [psf]:	13.0	(Equation 6-15)
Gust Effect Factor, G :	0.85	(Section 6.5.8)

$\gamma = 90^\circ$ or 270°

Force Coefficient, C_N :

Horizontal Distance from Winward Edge	Roof angle		
	Load Case	Obstructed Wind Flow	
		CN	
$\leq h$	30	A	-0.8
		B	0.8
$> h, \leq 2h$	30	A	-0.6
		B	0.5
$> 2h$	30	A	-0.3
		B	0.3

Design Wind Pressure, p [psf]:

	Roof angle		
	Load Case	Obstructed Wind Flow	
$\leq h$	30	A	-8.9
		B	8.9
$> h, \leq 2h$	30	A	-6.6
		B	5.5
$> 2h$	30	A	-3.3
		B	3.3



JOB NO.: U2716-095-191

DESIGNED: STB

Foundation Option 1: Drilled Concrete Pier



JOB NO.: U2716-095-191
DATE: 07/29/19

DESIGNED: STB

PROJECT: B2 – Sunmodo Sunturf GM

Drilled Pier Design

Design Loads:

Max. Shear, V [k]:	1.6	Max. Down, P _d [k]:	3.1
Max. Moment, M [k-ft]:	0.0	Max. Uplift, P _u [k]:	2.5

Pier Properties:

Pier Diameter, b [ft]:	1.5	Volume of Concrete [ft ³]:	11
Min. Pier Diameter, b _{min} (opt'l) [ft]:		Volume of Concrete [yd ³]:	0.4
Top of Pier Elevation [ft]:	0.00	Weight of Concrete [k]:	1.6
Pier Depth, d [ft]:	6.0		
Min. Pier Depth, d _{min} (opt'l) [ft]:			
Max. Pier Depth, d _{max} (opt'l) [ft]:			

Soil Properties:

Allow. Bearing Pressure [psf]:	1,500	<u>Optional Parameters for Uplift:</u>	
1/3 increase for short term loads?	No	Skin Friction [psf]:	250 (IBC 1810.3.3.1.4)
Lateral Bearing, S [pcf]:	150	Top Length to Ignore [ft]:	0.0
Max. Lateral Bearing (opt'l) [psf]:		1/3 increase for short term loads?	No
Top Depth to Ignore [ft]:	0.0	Combine w/ Bearing:	No
1/3 increase for short term loads?	No		
1/2" deflection at t/o pier allowed:	Yes		

Check Bearing:

Bearing Capacity [k]: 7.1 **Bearing capacity OK.**

Check Uplift:

Uplift Capacity [k]: 8.5 **Uplift capacity OK.**

Check Lateral Bearing:

Applied Lateral Force, P [lb]:	1,620	
Point of Application, h [ft]:	0.0	
S _{1_max} [psf]:		
S ₁ [psf]:	600	
A = 2.34*P/(S ₁ b):	4.21	
Required Pier Depth, d _{reqd} [ft]:	4.2	Lateral bearing capacity OK.

Foundation Option 2: Helical Pier

The ground screws and helical piers must be tested to 1.5 times uplift and 2.0 times lateral reactions found in the table below.

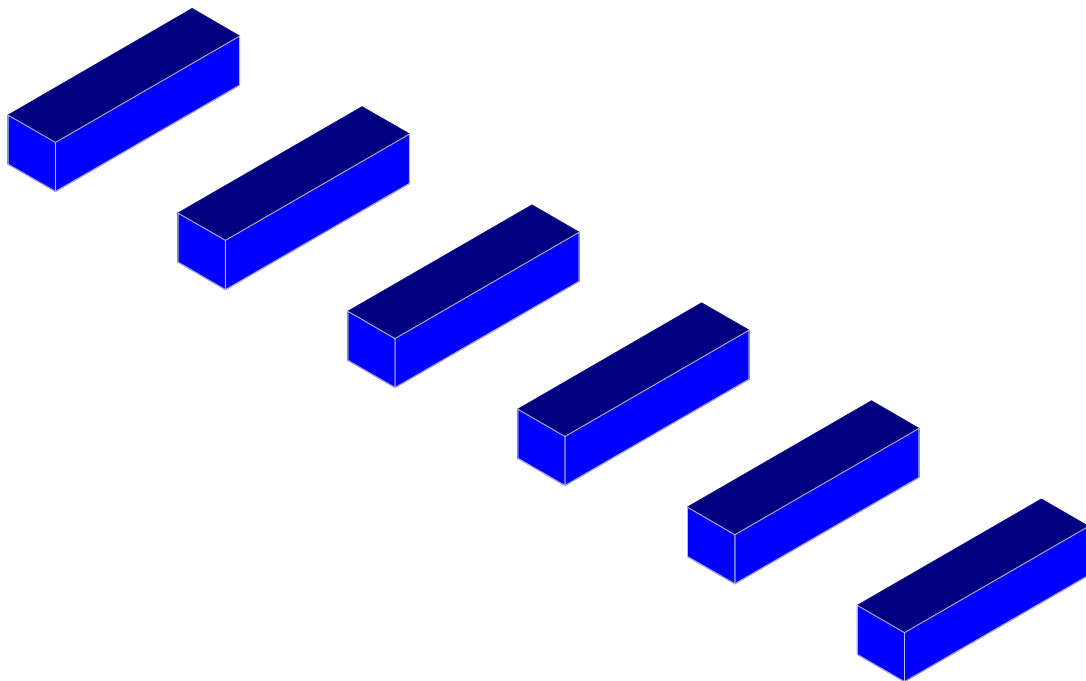
Load (ASD)	Value (lbs)	Factor of Safety	Test Value (lbs)
UPLIFT	2480	1.5	3960
LATERAL	1620	2	3370

Foundation Option 3: Ground Screw

The ground screws and helical piers must be tested to 1.5 times uplift and 2.0 times lateral reactions found in the table below.

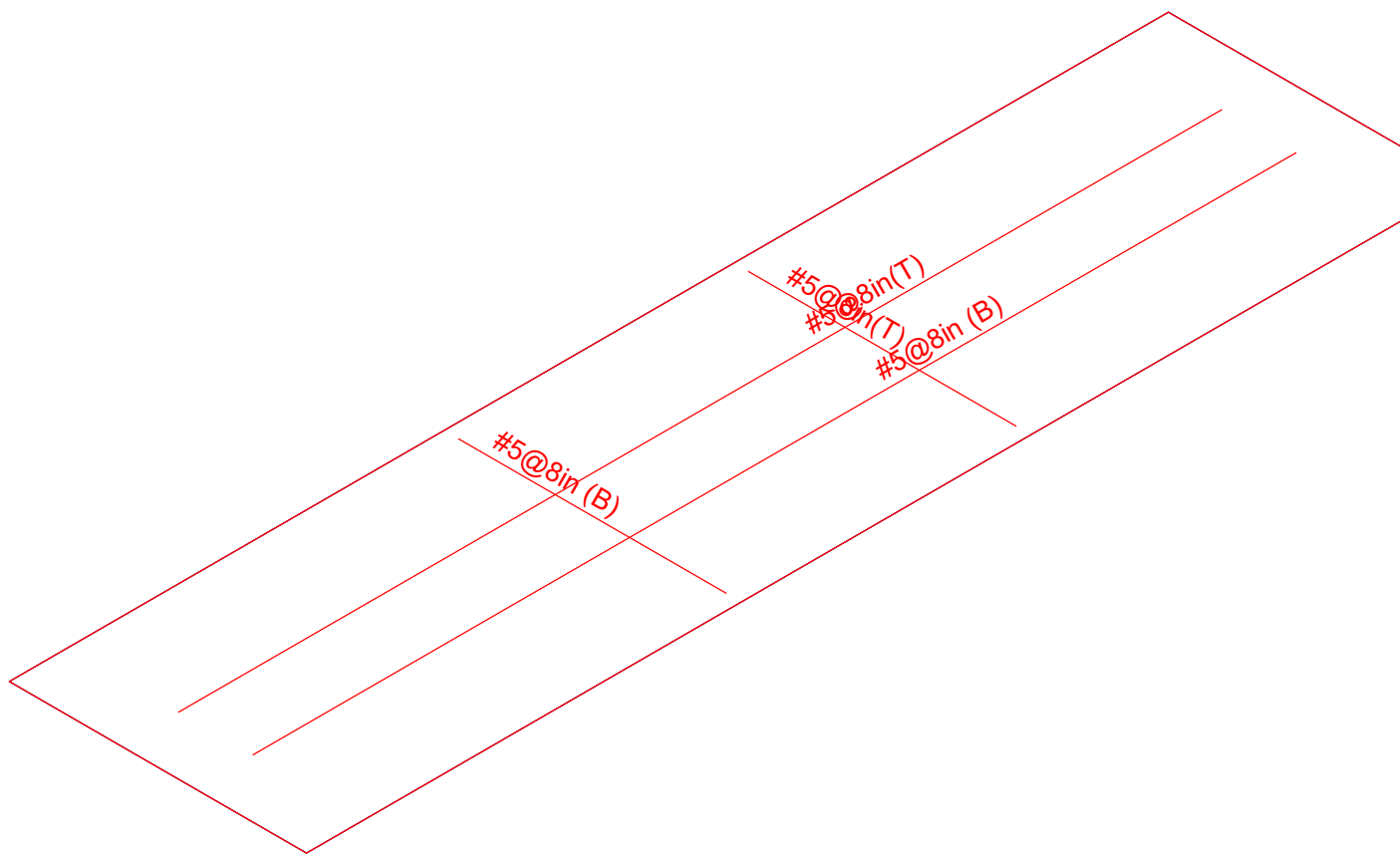
Load (ASD)	Value (lbs)	Factor of Safety	Test Value (lbs)
UPLIFT	2480	1.5	3960
LATERAL	1620	2	3370

Foundation Option 4: Ballasted Block



Results for LC 1, 1.0 D

Vector Structural Engineeri..	Ground Mount	SK - 1
STB		July 26, 2019 at 4:25 PM
U2716.095.191		USA B2 GM v4.r3d



Results for LC 1, 1.0 D

Vector Structural Engineeri..	Ground Mount	SK - 15
STB		May 7, 2019 at 1:06 PM
U2716.095.191		USA B2 GM - 81 in panels.r3d



(Global) Model Settings

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	100
Mesh Size (in)	12
Max Iterations	10
Merge Tolerance (in)	.12
Solver	Sparse Accelerated
Coefficient of Friction	.3
No. of Shear Regions	4
Shear Region Spacing Increment (in)	4
Min 1 Bar Dia Spacing for Beams?	No
Optimize footings for OTM / Sliding?	Yes
Parame Beta Factor	.65
Pile Safety Factor	3
Concrete Stress Block	Rectangular
Concrete Rebar Set	ASTM A615
Concrete Code	ACI 318-08
HR Steel Pile Code	AISC 14th(360-10): ASD
Wood Pile Code	AWC NDS-15: ASD

Concrete Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (\...	Density[lb/ft^3]	fc[psi]	Lambda	Flex Stee...	Shear St...
1	Conc3000NW	3156	1372	.15	.6	145	3000	1	60000	60000
2	Conc3500NW	3409	1482	.15	.6	145	3500	1	60000	60000
3	Conc4000NW	3644	1584	.15	.6	145	4000	1	60000	60000
4	Conc3000LW	2085	907	.15	.6	109.999	3000	.75	60000	60000
5	Conc3500LW	2252	979	.15	.6	109.999	3500	.75	60000	60000
6	Conc4000LW	2408	1047	.15	.6	109.999	4000	.75	60000	60000
7	Conc2500NW	3156	1372	.15	.6	145	2500	1	60000	60000

General Design Parameters

	Label	Max Bending Chk	Max Shear Chk	Top Cover[in]	Bottom Cover[in]
1	Typical	1	1	3	3

Slab Rebar Parameters

	Label	Top Bar	Bottom Bar	Max Top Bar Sp...	Min Top Bar Sp...	Max Bot Bar Sp...	Min Bot Bar Sp...	Spacing Incr...	Rebar Options
1	Typical	#5	#5	8	8	8	8	1	Optimize

Soil Definitions

	Label	Subgrade Modulus[lb/ft^3]	Allowable Bearing[psf]	Depth Properties	Default?
1	Default	1e+5	1500	None	Yes

Point Loads and Moments (Cat 1 : DL)

	Label	Direction	Magnitude[lb,lb-ft]
1	R3D_N1	X	-16.057
2	R3D_N1	Y	225.456
3	R3D_N2	X	-6.511
4	R3D_N2	Y	205.758
5	R3D_N132	Y	249.973
6	R3D_N133	X	-1.806
7	R3D_N133	Y	273.894
8	R3D_N109	Y	249.808
9	R3D_N110A	Y	275.351

Point Loads and Moments (Cat 1 : DL) (Continued)

	Label	Direction	Magnitude[lb,lb-ft]
10	R3D_N121	Y	257.609
11	R3D_N122	Y	262.078
12	R3D_N133B	Y	257.742
13	R3D_N134B	Y	263.011
14	R3D_N137C_1	X	15.56
15	R3D_N137C_1	Y	226.992
16	R3D_N138B_1	X	6.875
17	R3D_N138B_1	Y	208.156

Point Loads and Moments (Cat 6 : RLL)

	Label	Direction	Magnitude[lb,lb-ft]
1	R3D_N1	X	-46.465
2	R3D_N1	Y	549.744
3	R3D_N2	X	-19.805
4	R3D_N2	Y	551.975
5	R3D_N132	Y	641.548
6	R3D_N133	X	-5.891
7	R3D_N133	Y	759.01
8	R3D_N109	Y	640.562
9	R3D_N110A	X	1.868
10	R3D_N110A	Y	763.567
11	R3D_N121	Y	667.063
12	R3D_N122	Y	721.759
13	R3D_N133B	X	1.231
14	R3D_N133B	Y	667.431
15	R3D_N134B	X	3.288
16	R3D_N134B	Y	724.843
17	R3D_N137C_1	X	44.845
18	R3D_N137C_1	Y	550.247
19	R3D_N138B_1	X	21.355
20	R3D_N138B_1	Y	554.933

Point Loads and Moments (Cat 16 : OL1)

	Label	Direction	Magnitude[lb,lb-ft]
1	R3D_N1	X	80.454
2	R3D_N1	Y	-1811.462
3	R3D_N1	Z	946.417
4	R3D_N2	X	33.617
5	R3D_N2	Y	121.896
6	R3D_N2	Z	-44.035
7	R3D_N132	Y	-2377.195
8	R3D_N132	Z	1369.974
9	R3D_N133	X	5.38
10	R3D_N133	Y	242.588
11	R3D_N133	Z	-53.461
12	R3D_N109	X	1.787
13	R3D_N109	Y	-2392.606
14	R3D_N109	Z	1379.995
15	R3D_N110A	X	-5.594
16	R3D_N110A	Y	248.319
17	R3D_N110A	Z	-53.704
18	R3D_N121	X	-1.511
19	R3D_N121	Y	-2314.666
20	R3D_N121	Z	1271.676
21	R3D_N122	Y	186.557
22	R3D_N122	Z	-53.667



Company : Vector Structural Engineering
 Designer : STB
 Job Number : U2716.095.191
 Model Name : Ground Mount

July 29, 2019
 2:39 PM
 Checked By: JSP

Slab Sliding Safety Factors (Continued)

	LC	Slab	Angle[deg]	Va-xx[lb]	Vr-xx[lb]	Va-zz[lb]	Vr-zz[lb]	SR-xx	SR-zz
115	20	S1	0	21.978	1288.179	403.613	1288.179	9.999+	3.192
116	20	S2	0	3.089	1309.28	443.344	1309.28	9.999+	2.953
117	20	S3	0	4.32	1414.899	210.032	1414.899	9.999+	6.737
118	20	S4	0	1.657	1420.01	196.96	1420.01	9.999+	7.21
119	20	S5	0	2.914	1421.556	218.858	1421.556	9.999+	6.495
120	20	S6	0	10.081	1422.739	151.475	1422.739	9.999+	9.393

Envelope Slab Soil Pressures

	Label	UC	LC	Soil Pressure[psf]	Allowable Bearing[psf]	Point
1	S1	.425	5	637.306	1500	N1
2	S2	.49	5	734.585	1500	N18
3	S3	.481	5	721.1	1500	N22
4	S4	.48	5	720.182	1500	N26
5	S5	.488	5	731.988	1500	N30
6	S6	.427	5	640.619	1500	N34



Company:		Date:	5/14/2018
Engineer:		Page:	1/6
Project:			
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-08
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: F1554 Grade 36
Diameter (inch): 0.375
Effective Embedment depth, h_{ef} (inch): 4.000
Code report: ICC-ES ESR-4057
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 5.25
 c_{ac} (inch): 7.12
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 24.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Ignore concrete breakout in tension: No
Ignore concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 150/110°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.75 x 4.75 x 0.31

Recommended Anchor

Anchor Name: SET-3G - SET-3G w/ 3/8"Ø F1554 Gr. 36
Code Report: ICC-ES ESR-4057





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Load and Geometry

Load factor source: ACI 318 Section 9.2

Load combination: not set

Seismic design: No

Anchors subjected to sustained tension: No

Apply entire shear load at front row: No

Anchors only resisting wind and/or seismic loads: No

Strength level loads:

N_{ua} [lb]: 4140

V_{uax} [lb]: 540

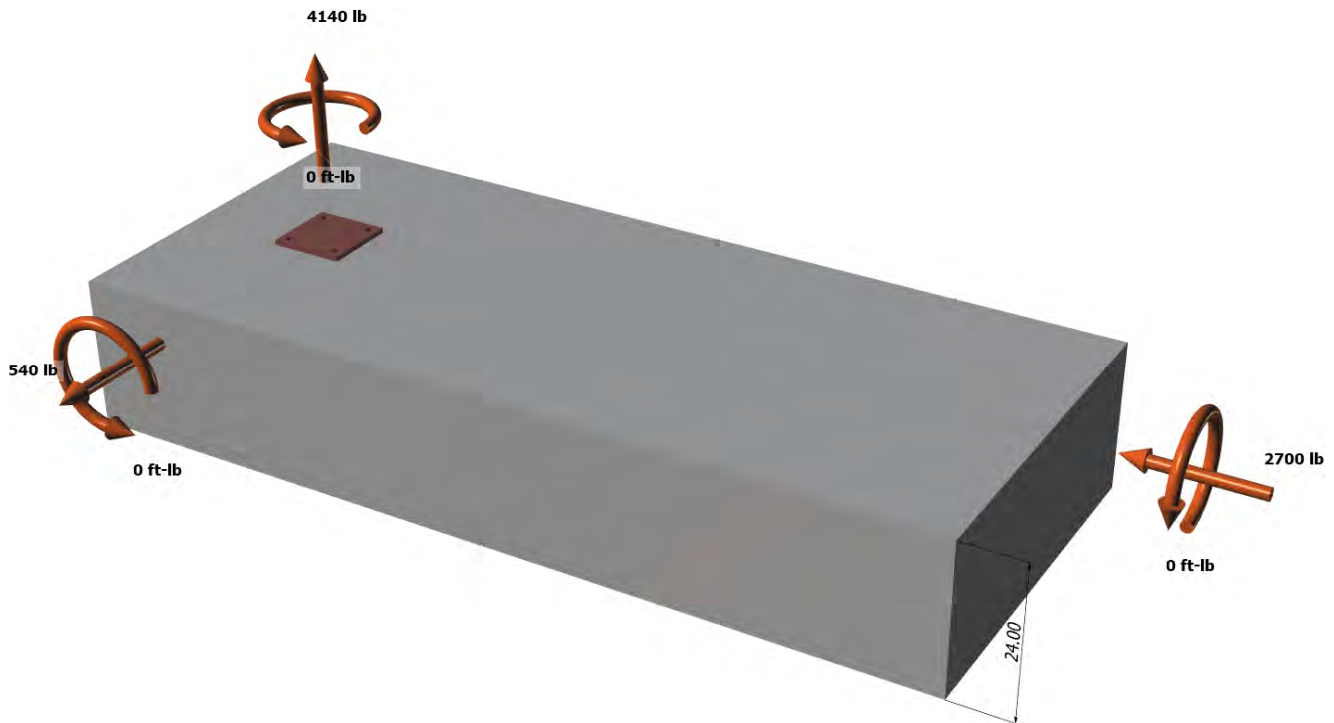
V_{uay} [lb]: -2700

M_{ux} [ft-lb]: 0

M_{uy} [ft-lb]: 0

M_{uz} [ft-lb]: 0

<Figure 1>





Company:		Date:	5/14/2018
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<Figure 2>





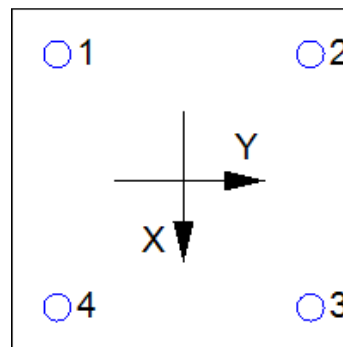
Company:		Date:	5/14/2018
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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1035.0	135.0	-675.0	688.4
2	1035.0	135.0	-675.0	688.4
3	1035.0	135.0	-675.0	688.4
4	1035.0	135.0	-675.0	688.4
Sum	4140.0	540.0	-2700.0	2753.5

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 4140
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
4525	0.75	3394

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	4.000	6800

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$c_{a,min}$ (in)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
240.25	144.00	7.25	1.000	1.000	1.00	1.000	6800	0.65	7374

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr,short-term} K_{sat} (f_c / 2,500)^n$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	f_c (psi)	n	$\tau_{k,cr}$ (psi)
1346	1.00	1.00	2500	0.24	1346

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1346	0.38	4.000	6343

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_{ag} (lb)
161.83	85.03	1.000	1.038	1.000	1.000	6343	0.55	6892

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
2715	1.0	0.65	1765

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
3.00	0.375	1.00	2500	13.75	16564

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
647.11	850.78	1.000	0.805	1.000	1.000	16564	0.70	7103

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
3.00	0.375	1.00	2500	10.75	11450

$$\phi V_{cbgy} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgy} (lb)
387.00	520.03	1.000	0.891	1.000	1.000	11450	0.70	5313

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
3.00	0.375	1.00	2500	7.25	6342

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
261.00	236.53	1.000	1.000	1.000	1.000	6342	0.70	9797

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
3.00	0.375	1.00	2500	10.25	10661

$$\phi V_{cbgy} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbgy} (lb)
401.67	472.78	1.000	1.000	1.000	1.000	10661	0.70	12680

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min\{k_{cp} N_{ag}; k_{cp} N_{cbg}\} = \phi \min\{k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b\} \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	161.83	85.03	1.000	1.038	1.000	1.000	6343	12531

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
240.25	144.00	1.000	1.000	1.000	1.000	6800	11345	0.70

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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ϕV_{cpq} (lb)
15883

11. Results

Interaction of Tensile and Shear Forces (Sec. RD.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	1035	3394	0.30	Pass	
Concrete breakout	4140	7374	0.56	Pass	
Adhesive	4140	6892	0.60	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	688	1765	0.39	Pass	
T Concrete breakout x+	540	7103	0.08	Pass	
T Concrete breakout y-	2700	5313	0.51	Pass	
Concrete breakout y-	270	9797	0.03	Pass	
Concrete breakout x-	1350	12680	0.11	Pass	
Concrete breakout, combined	-	-	0.51	Pass (Governs)	
Pryout	2753	15883	0.17	Pass	
Interaction check	$(N_{ua}/\phi N_{ua})^{5/3}$	$(V_{ua}/\phi V_{ua})^{5/3}$	Combined Ratio	Permissible	Status
Sec. RD.7	0.43	0.33	75.7%	1.0	Pass

SET-3G w/ 3/8"Ø F1554 Gr. 36 with hef = 4.000 inch meets the selected design criteria.

12. Warnings

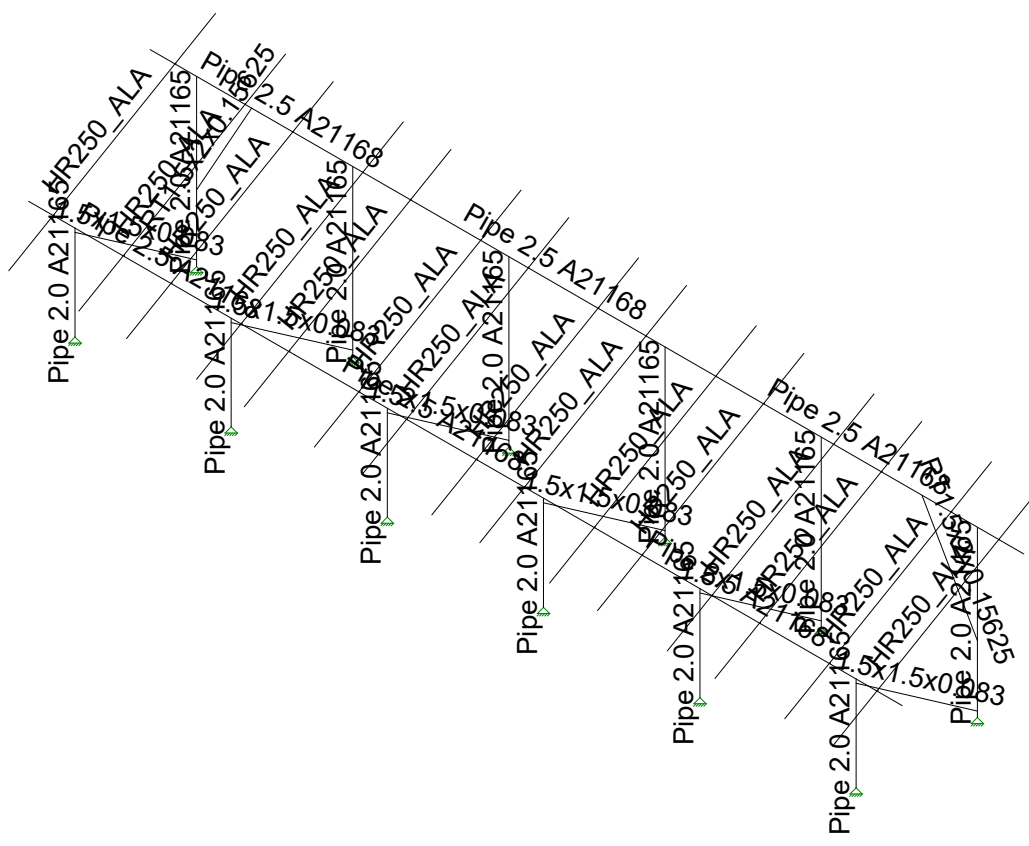
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



JOB NO.: U2716-095-191

DESIGNED: STB

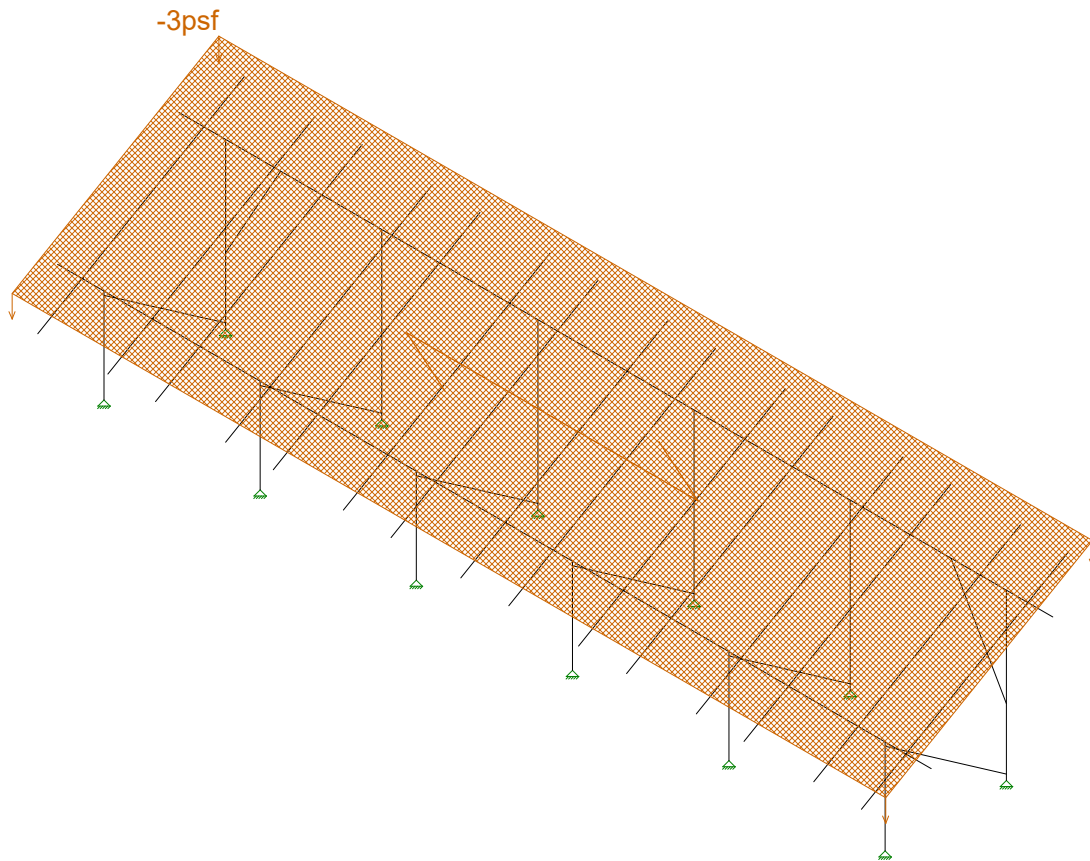
Framing Analysis



Vector Structural Engineeri...
STB
U2716.095.191

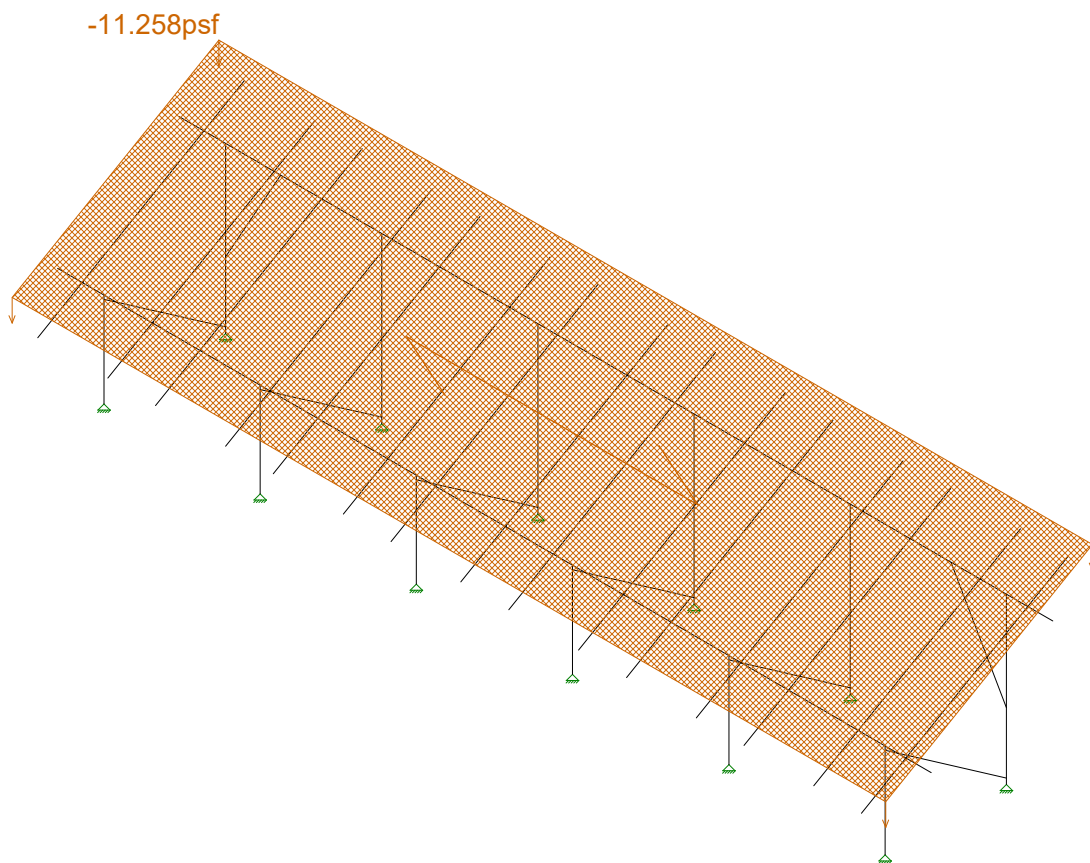
Ground Mount

SK - 4
July 29, 2019 at 2:46 PM
USA B2 GM v4.r3d



Loads: BLC 2, Solar Panel Weight

Vector Structural Engineeri..	Ground Mount	SK - 6
STB		July 29, 2019 at 2:46 PM
U2716.095.191		USA B2 GM v4.r3d



Loads: BLC 3, Roof Live/Snow

Vector Structural Engineeri..

STB

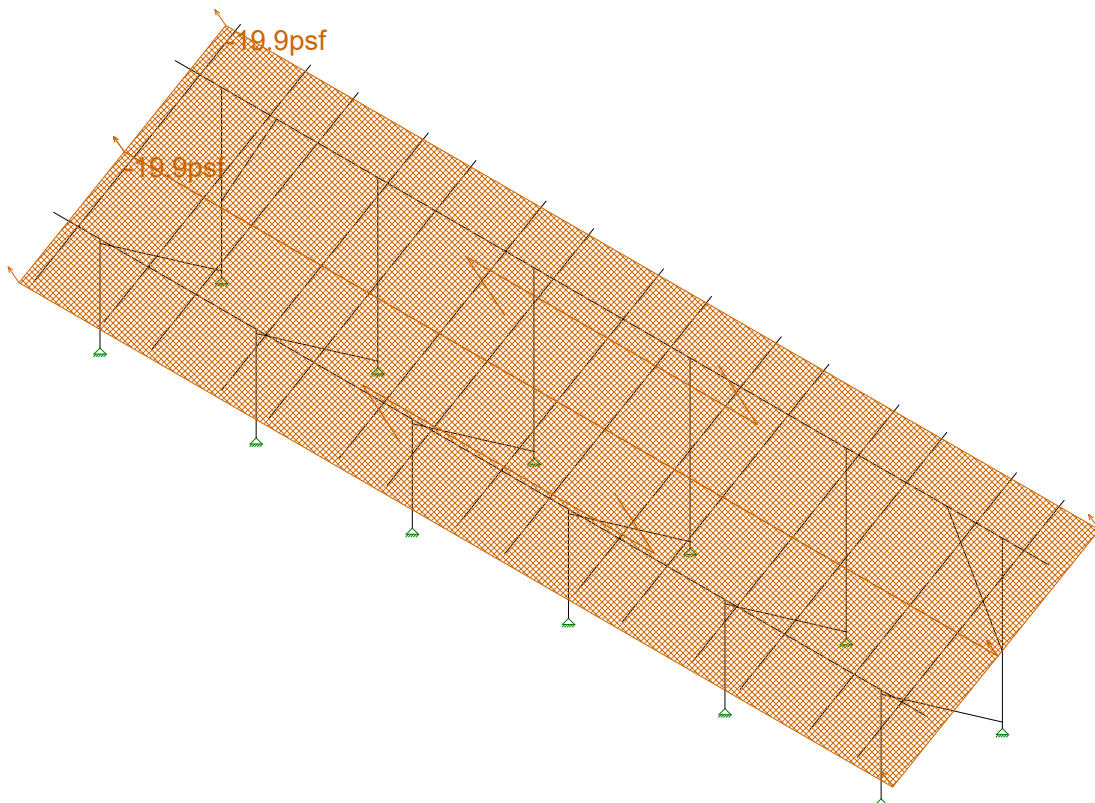
U2716.095.191

Ground Mount

SK - 7

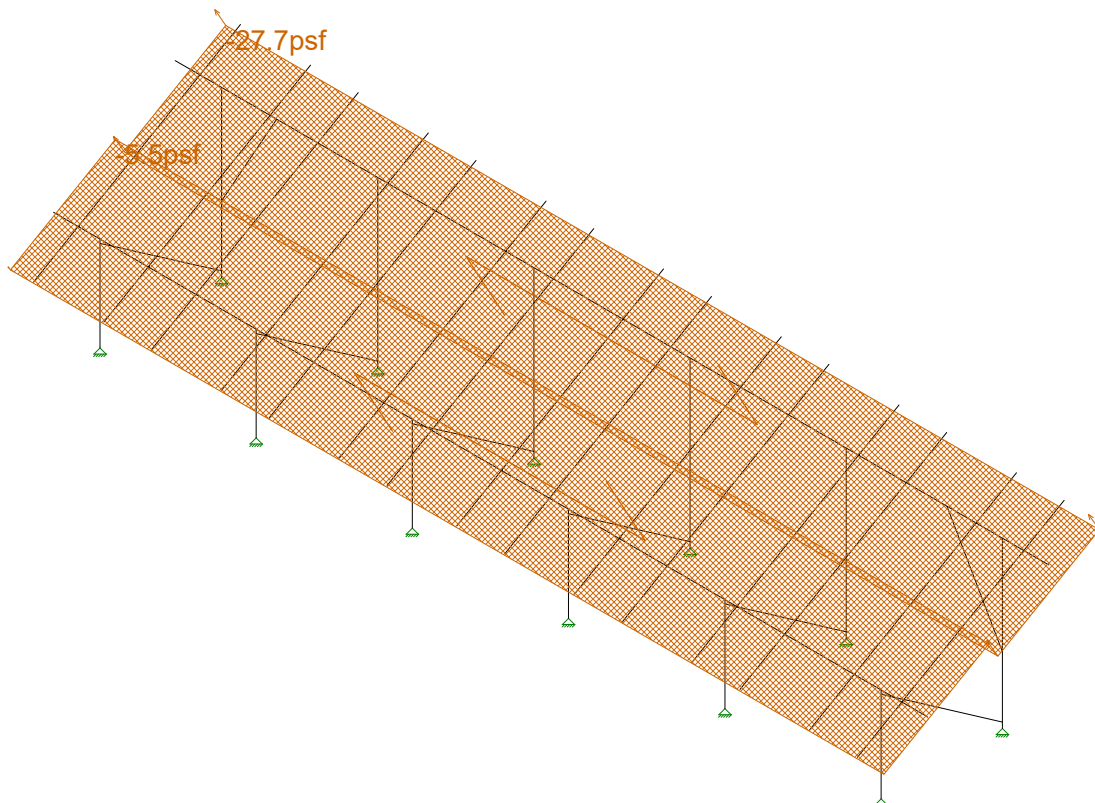
July 29, 2019 at 2:46 PM

USA B2 GM v4.r3d



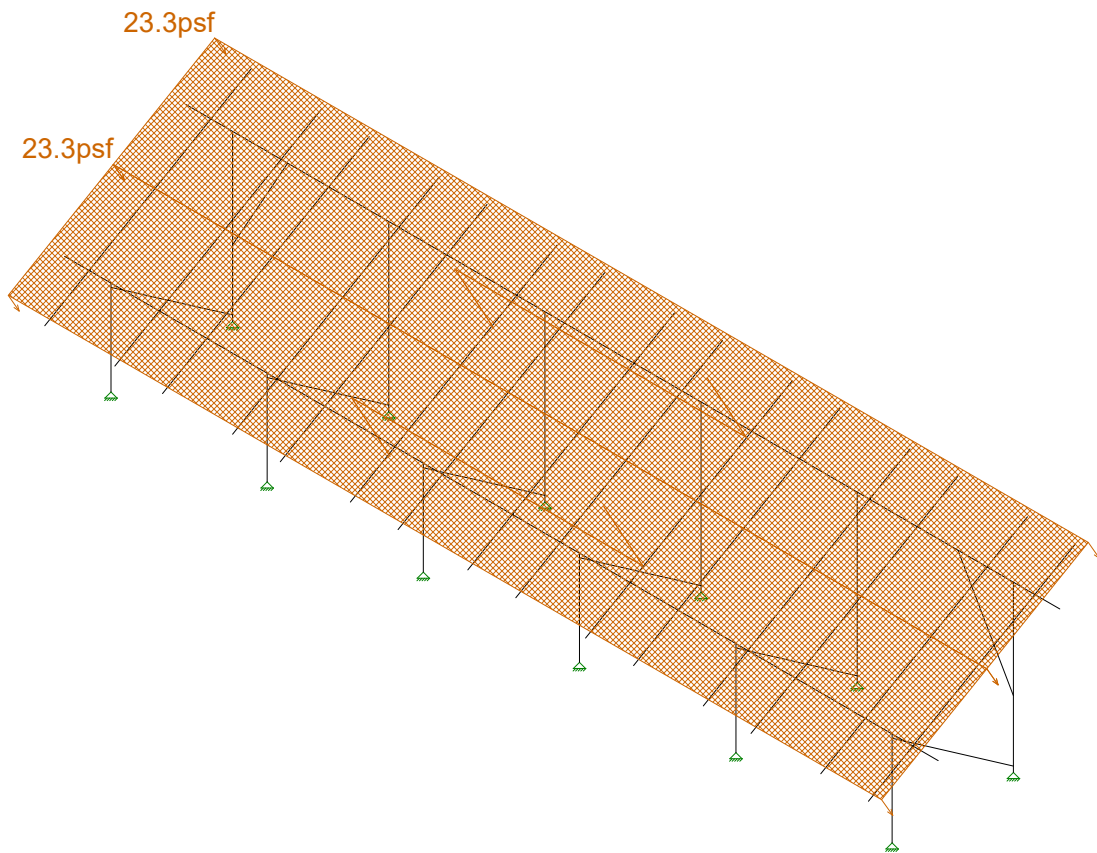
Loads: BLC 4, Wind A 0 deg

Vector Structural Engineeri..	Ground Mount	SK - 8
STB		July 29, 2019 at 2:46 PM
U2716.095.191		USA B2 GM v4.r3d



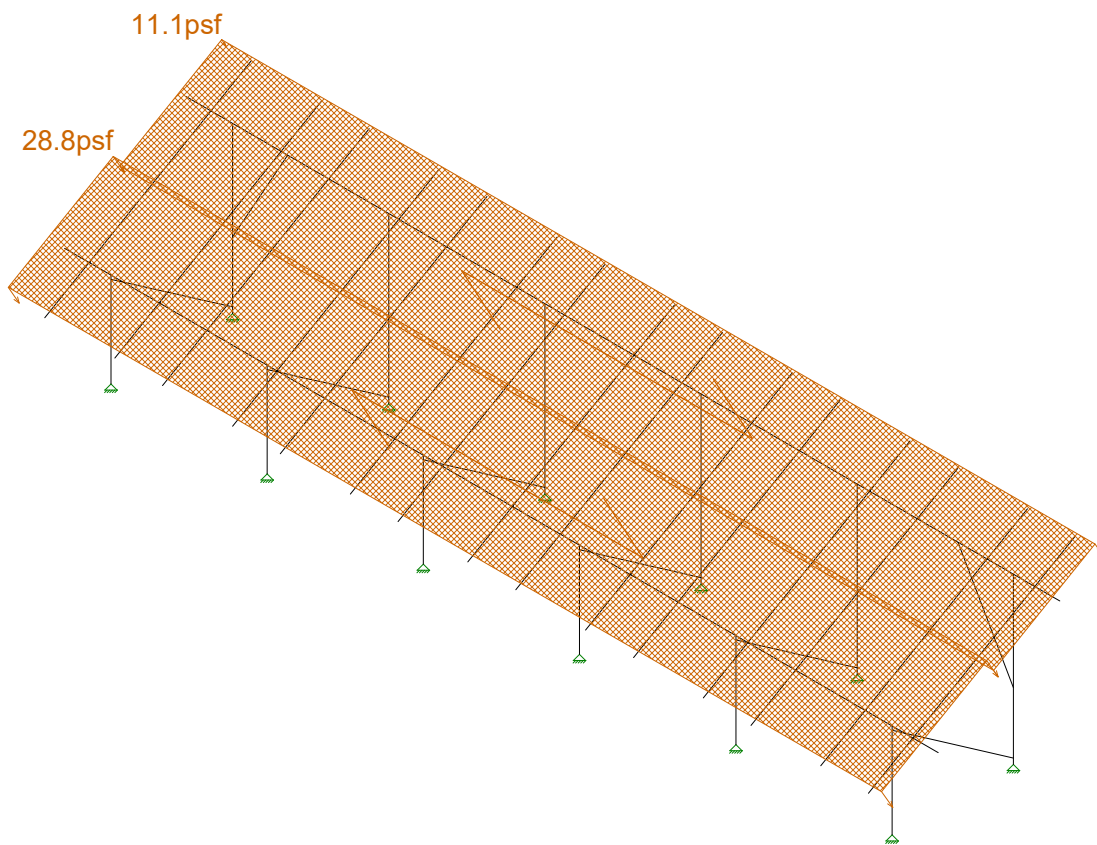
Loads: BLC 5, Wind B 0 deg

Vector Structural Engineeri..	Ground Mount	SK - 9
STB		July 29, 2019 at 2:46 PM
U2716.095.191		USA B2 GM v4.r3d



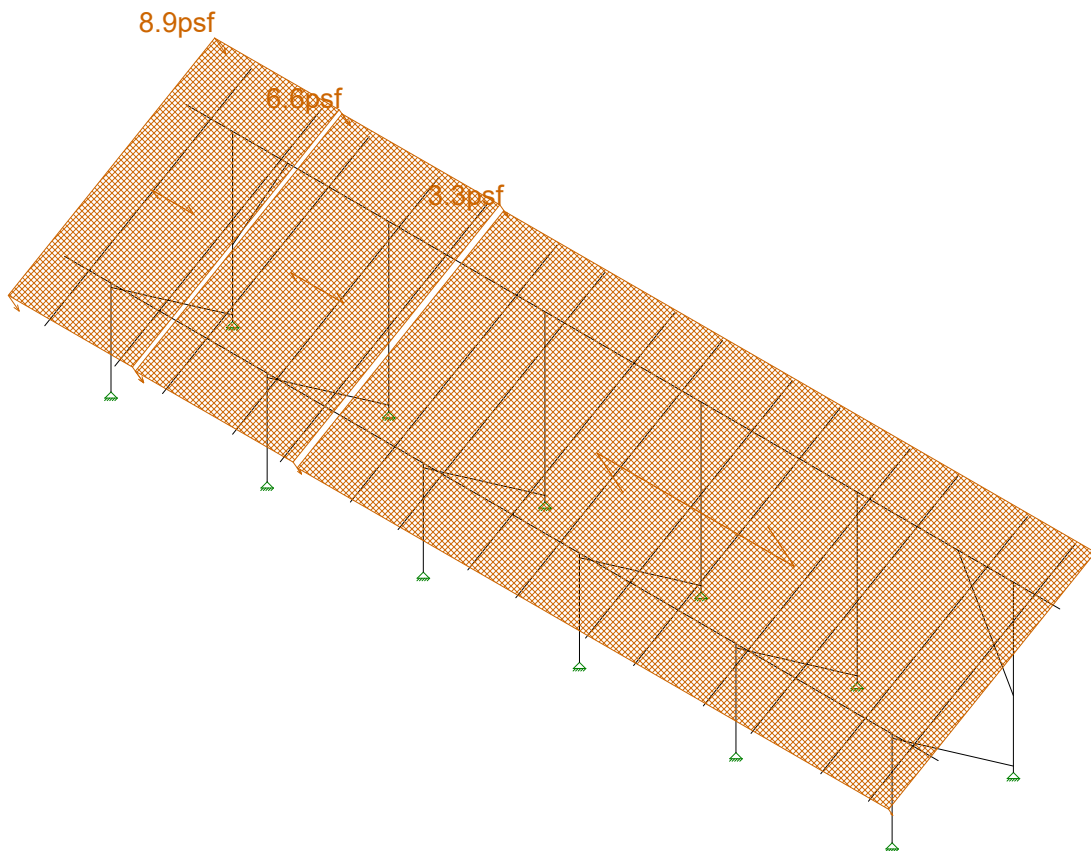
Loads: BLC 6, Wind A 180 deg

Vector Structural Engineeri..	Ground Mount	SK - 10
STB		July 29, 2019 at 2:47 PM
U2716.095.191		USA B2 GM v4.r3d



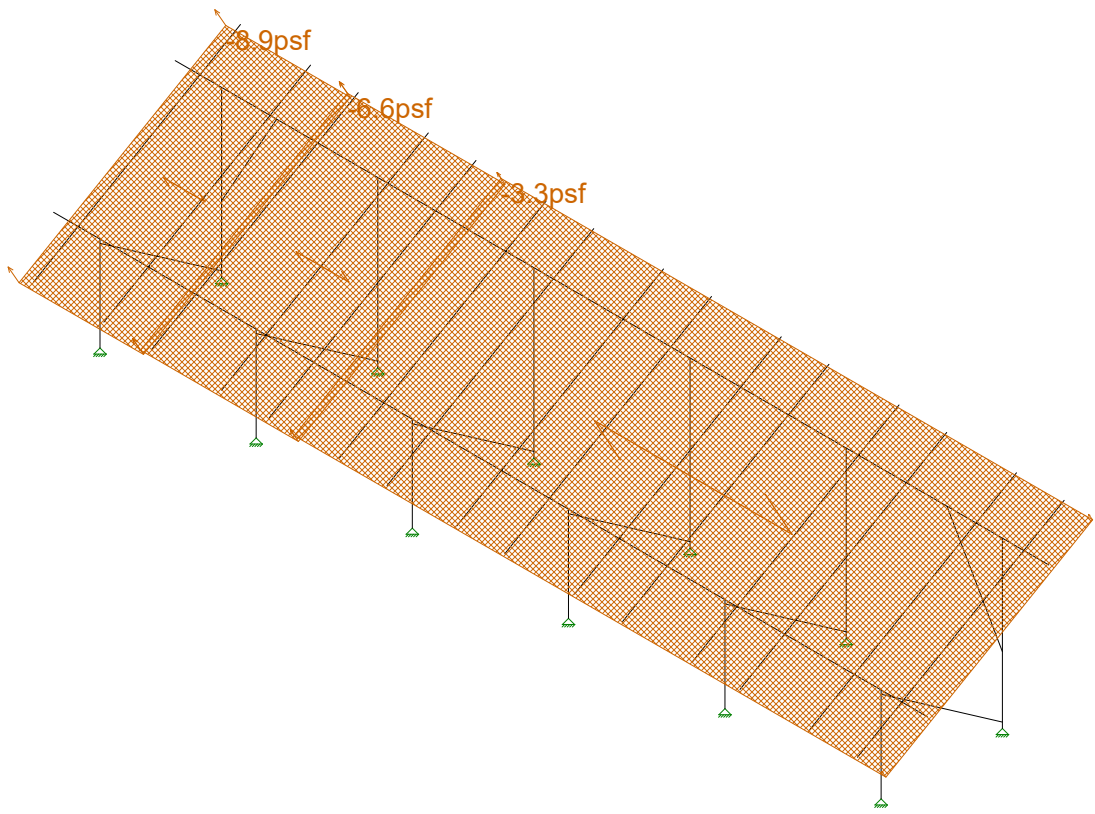
Loads: BLC 7, Wind B 180 deg

Vector Structural Engineeri..	Ground Mount	SK - 12
STB		July 29, 2019 at 2:47 PM
U2716.095.191		USA B2 GM v4.r3d



Loads: BLC 8, Wind A 90

Vector Structural Engineeri..	Ground Mount	SK - 13
STB		July 29, 2019 at 2:47 PM
U2716.095.191		USA B2 GM v4.r3d

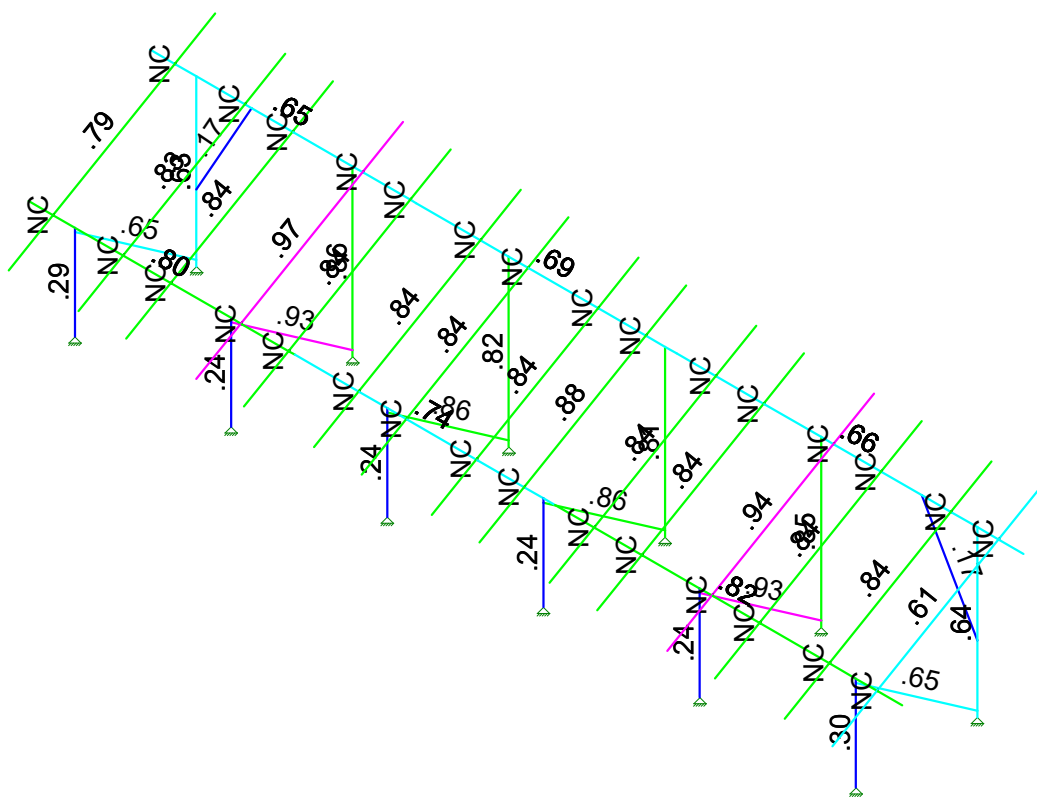


Loads: BLC 9, Wind B 90

Vector Structural Engineeri...	Ground Mount	SK - 14
STB		July 29, 2019 at 2:47 PM
U2716.095.191		USA B2 GM v4.r3d



Code Check (Enr)	Color
No Calc	Black
> 1.0	Red
40-1.0	Orange
75-90	Yellow
50-75	Green
0-.50	Blue



Member Code Checks Displayed (Enveloped)
Envelope Only Solution

Vector Structural Engineeri...	Ground Mount	SK - 1
STB		July 29, 2019 at 2:45 PM
U2716.095.191		USA B2 GM v4.r3d

(Global) Model Settings, Continued

Seismic Code	ASCE 7-10
Seismic Base Elevation (in)	15600
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
T X (sec)	Not Entered
T Z (sec)	Not Entered
R X	3
R Z	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	I or II
Drift Cat	Other
Om Z	1
Om X	1
Cd Z	4
Cd X	4
Rho Z	1
Rho X	1

Hot Rolled Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (1E5 F)	Density[lb/ft^3]	Yield[psi]	Ry	Fu[psi]	Rt
1	A992	29000	11154	.3	.65	490	50000	1.1	65000	1.1
2	A36 Gr.36	29000	11154	.3	.65	490	36000	1.5	58000	1.2
3	A572 Gr.50	29000	11154	.3	.65	490	50000	1.1	65000	1.1
4	A500 Gr.B R...	29000	11154	.3	.65	527	42000	1.4	58000	1.3
5	A500 Gr.B Re...	29000	11154	.3	.65	527	46000	1.4	58000	1.3
6	A53 Gr.B	29000	11154	.3	.65	490	35000	1.6	60000	1.2
7	A1085	29000	11154	.3	.65	490	50000	1.4	65000	1.3

Aluminum Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (...Density[... Table 3.3	kt	Ftu[psi]	Fty[psi]	Fcy[psi]	Fsu[psi]	Ct
1	3003-H14	10100	3787.5	.33	1.3 172.8 Table 3.3-3	1	19000	16000	13000	12000	141
2	6061-T6	10100	3787.5	.33	1.3 172.8 Table 3.3-4	1	38000	35000	35000	24000	141
3	6063-T5	10100	3787.5	.33	1.3 172.8 Table 3.3-4	1	22000	16000	16000	13000	141
4	6063-T6	10100	3787.5	.33	1.3 172.8 Table 3.3-4	1	30000	25000	25000	19000	141
5	5052-H34	10200	3787.5	.33	1.3 172.8 Table 3.3-3	1	34000	26000	24000	20000	141
6	6061-T6 W	10100	3787.5	.33	1.3 172.8 Table 3.3-3	1	24000	15000	15000	15000	141
7	6005-T5	10100	3787.5	.33	1.3 172.8 Table 3.3-3	1	38000	35000	35000	24000	141

Hot Rolled Steel Section Sets

	Label	Shape	Type	Design List	Material	Design R...	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	Post	Pipe 2.0 A21165	Column	Pipe	A572 Gr.50	Typical	.776	.499	.499	.998
2	Cross Beam	Pipe 2.5 A21168	Beam	Wide Flange	A572 Gr.50	Typical	.947	.907	.907	1.814
3	Diagonal Brace	1.5x1.5x0.083	HBrace	SquareTube	A36 Gr.36	Typical	.47	.158	.158	.236



Company : Vector Structural Engineering
 Designer : STB
 Job Number : U2716.095.191
 Model Name : Ground Mount

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Aluminum Section Sets

	Label	Shape	Type	Design List	Material	Design R...	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	AL Posts	2.375ODX0.188	Column	Pipe	6005-T5	Typical	1.29	.778	.778	1.54
2	AL Brace	RT1.5x2x0.15625	VBrace	Rectangular Tubes	6005-T5	Typical	.996	.327	.524	.602
3	AL Rails	HR250 ALA	Beam	Rectangular Tubes	6005-T5	Typical	.723	.095	.486	.261
4	AL Cross Beam	Cross Rail	Beam	Rectangular Tubes	6005-T5	Typical	1.909	1.97	4.366	4.017

Member Area Loads (BLC 2 : Solar Panel Weight)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N199	N196	Y	A-B	-3

Member Area Loads (BLC 3 : Roof Live/Snow)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N199	N196	PY	A-B	-13

Member Area Loads (BLC 4 : Wind A 0 deg)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	-19.9
2	N198	N201	N199	N196	Perp	A-B	-19.9

Member Area Loads (BLC 5 : Wind B 0 deg)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	-27.7
2	N198	N201	N199	N196	Perp	A-B	-5.5

Member Area Loads (BLC 6 : Wind A 180 deg)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	23.3
2	N198	N201	N199	N196	Perp	A-B	23.3

Member Area Loads (BLC 7 : Wind B 180 deg)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N200	N201	N198	Perp	A-B	11.1
2	N198	N201	N199	N196	Perp	A-B	28.8

Member Area Loads (BLC 8 : Wind A 90)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N203	N202	N196	Perp	A-B	8.9
2	N203	N209	N208	N202	Perp	A-B	6.6
3	N209	N200	N199	N208	Perp	A-B	3.3

Member Area Loads (BLC 9 : Wind B 90)

	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N197	N203	N202	N196	Perp	A-B	-8.9
2	N203	N209	N208	N202	Perp	A-B	-6.6
3	N209	N200	N199	N208	Perp	A-B	-3.3

Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed	Area(M...Surface...
1	Self Weight	DL		-1.05					
2	Solar Panel Weight	DL							1



Company : Vector Structural Engineering
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Envelope Joint Reactions (Continued)

Joint	X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [lb-ft]	LC	MY [lb-ft]	LC	MZ [lb-ft]	LC
14	min	-2.099	19	-2428.307	16	-1272....	3	0	1	0	1	0
15	N122	max	2.305	14	1419.091	10	54.185	3	0	1	0	1
16		min	-2.066	19	-69.579	17	-63.959	5	0	1	0	1
17	N133B	max	5.28	16	2972.003	5	1482.26	5	0	1	0	1
18		min	-1.705	13	-2423.124	16	-1265....	3	0	1	0	1
19	N134B	max	.252	15	1419.31	10	53.93	3	0	1	0	1
20		min	-7.911	12	-61.343	17	-63.717	5	0	1	0	1
21	N137C	max	97.359	16	2370.732	5	1122.0...	5	0	1	0	1
22		min	-105.771	11	-1907.154	16	-957.175	3	0	1	0	1
23	N138B	max	27.371	15	1075.058	10	44.588	3	0	1	0	1
24		min	-58.775	12	-34.804	17	-52.683	5	0	1	0	1
25	Totals:	max	.137	11	19275.533	11	8064.4...	5				
26		min	-.056	20	-10155.318	15	-6887....	15				

Envelope AISC 13th(360-05): ASD Steel Code Checks

Member	Shape	Code Check	Loc[in]	LC	Shear	...	Loc[in]	Dir	LC	Pnc/om	...	Pnt/om	[lb]	Mnyy/om	...	Mnzz/om	...	Cb	Eqn
1	M5	Pipe 2.0 A2...	.292	63.195	12	.172	63.195	5	14323.286	23232.186	1397.505	1397.505	1...	H1-1b					
2	M6	Pipe 2.0 A2...	.630	3.561	5	.171	0	5	5778.24	23232.186	1397.505	1397.505	1...	H1-1a					
3	M13	Pipe 2.5 A2...	.803	139....	12	.159	141....	6	11641.036	28358.413	2081.747	2081.747	1...	H1-1b					
4	M14	Pipe 2.5 A2...	.647	139....	11	.166	141....	5	11641.036	28358.413	2081.747	2081.747	1...	H1-1b					
5	M15	1.5x1.5x0.083	.647	53.339	5	.081	0	y	11	2263.956	10141.308	449.583	449.583	1...	H1-1a				
6	M80	Pipe 2.0 A2...	.241	62.516	17	.235	63.195	5	14323.286	23232.186	1397.505	1397.505	1...	H1-1b					
7	M81	Pipe 2.0 A2...	.854	3.561	5	.247	0	5	5778.24	23232.186	1397.505	1397.505	2...	H1-1a					
8	M82	1.5x1.5x0.083	.925	53.339	5	.017	102....	y	4	2263.956	10141.308	449.583	449.583	1...	H1-1a				
9	M50	Pipe 2.0 A2...	.244	62.516	17	.237	63.195	5	14323.286	23232.186	1397.505	1397.505	1...	H1-1b					
10	M51	Pipe 2.0 A2...	.861	3.561	5	.249	0	5	5778.24	23232.186	1397.505	1397.505	1...	H1-1a					
11	M52	1.5x1.5x0.083	.932	53.339	5	.014	102....	y	5	2263.956	10141.308	449.583	449.583	1...	H1-1a				
12	M56A	Pipe 2.0 A2...	.240	62.516	17	.217	63.195	5	14323.286	23232.186	1397.505	1397.505	1...	H1-1b					
13	M57A	Pipe 2.0 A2...	.816	3.561	5	.229	0	5	5778.24	23232.186	1397.505	1397.505	1...	H1-1a					
14	M58A	1.5x1.5x0.083	.860	53.339	5	.011	0	y	11	2263.956	10141.308	449.583	449.583	1...	H1-1a				
15	M68	Pipe 2.0 A2...	.239	62.516	17	.215	63.195	5	14323.286	23232.186	1397.505	1397.505	1...	H1-1b					
16	M69	Pipe 2.0 A2...	.814	3.561	5	.228	0	5	5778.24	23232.186	1397.505	1397.505	1...	H1-1a					
17	M70	1.5x1.5x0.083	.856	53.339	5	.008	102....	y	13	2263.956	10141.308	449.583	449.583	1...	H1-1a				
18	M68A	Pipe 2.5 A2...	.743	172.5	12	.144	170....	6	11641.036	28358.413	2081.747	2081.747	3	H1-1b					
19	M69A	Pipe 2.5 A2...	.819	100	12	.174	100	6	11641.036	28358.413	2081.747	2081.747	2	H1-1b					
20	M70A	Pipe 2.5 A2...	.688	172.5	11	.151	170....	11	11641.036	28358.413	2081.747	2081.747	3	H1-1b					
21	M71	Pipe 2.5 A2...	.664	100	11	.158	100	5	11641.036	28358.413	2081.747	2081.747	2	H1-1b					
22	M66A	Pipe 2.0 A2...	.305	63.195	12	.175	63.195	5	14323.286	23232.186	1397.505	1397.505	1...	H1-1b					
23	M67A	Pipe 2.0 A2...	.638	3.561	5	.173	0	5	5778.24	23232.186	1397.505	1397.505	1...	H1-1a					
24	M68C	1.5x1.5x0.083	.654	53.339	5	.093	0	y	11	2263.956	10141.308	449.583	449.583	1...	H1-1a				

Envelope AA ADM1-05: ASD - Building Aluminum Code Checks

Member	Shape	Code	...	Loc[in]	LC	Shear	...	Loc[in]	Dir	LC	Fc [psi]	Ft [psi]	Fb y-y	...	Fb z-z	...	Fs y-y	...	Fs z-z	...	Cb	Cmy	Eqn
1	M19	RT1.5x2...	.168	49.431	11	.008	77.793	y	5	2773.1...	19487...	21212...	21154...	12246...	12246...	1...	.6	.6	4.1.1-1				
2	M16	HR250795	36.1	12	.075	37.819	y	12	3058.2...	19487...	19551...	17939...	12246...	12246...	1	.6	.6	4.1.1-3				
3	M35	HR250828	85.953	11	.088	37.819	y	12	3058.2...	19487...	19487...	19487...	12246...	12246...	1	.6	.6	4.1.2-1				
4	M38	HR250838	36.1	12	.081	37.819	y	12	3058.2...	19487...	19551...	17939...	12246...	12246...	1	.6	.6	4.1.1-3				
5	M41	HR250974	89.391	16	.073	37.819	y	12	3058.2...	19487...	19551...	17939...	12246...	12246...	1	.6	.6	4.1.1-1				
6	M44	HR250838	36.1	12	.076	37.819	y	12	3058.2...	19487...	19551...	17939...	12246...	12246...	1	.6	.6	4.1.1-3				
7	M47	HR250838	36.1	12	.076	37.819	y	12	3058.2...	19487...	19551...	17939...	12246...	12246...	1	.6	.6	4.1.1-3				
8	M50A	HR250839	36.1	12	.077	37.819	y	12	3058.2...	19487...	19551...	17939...	12246...	12246...	1	.6	.6	4.1.1-3				
9	M53	HR250838	36.1	12	.075	37.819	y	12	3058.2...	19487...	19551...	17939...	12246...	12246...	1	.6	.6	4.1.1-3				
10	M56	HR250882	89.391	16	.077	37.819	y	12	3058.2...	19487...	19551...	17939...	12246...	12246...	1	.6	.6	4.1.1-1				
11	M59	HR250839	36.1	12	.077	37.819	y	12	3058.2...	19487...	19551...	17939...	12246...	12246...	1	.6	.6	4.1.1-3				



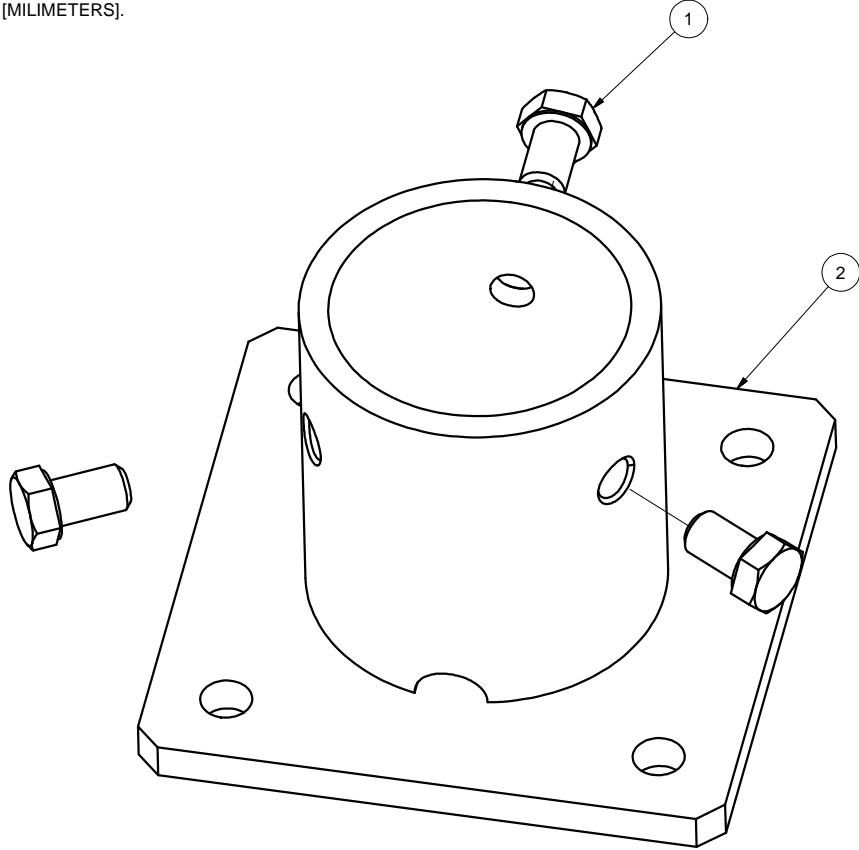
Company : Vector Structural Engineering
 Designer : STB
 Job Number : U2716.095.191
 Model Name : Ground Mount

July 29, 2019
 2:48 PM
 Checked By: _____

Envelope AA ADM1-05: ASD - Building Aluminum Code Checks (Continued)

Member	Shape	Code	Loc[in]	LC	Shear	Loc[in]	Dir	LC	Fc [psi]	Ft [psi]	Fb y-y	Fb z-z	Fs y-y	Fs z-z	Cb	Cmy	Cmz	Eqn	
12	M62	HR250_...	.838	36.1	12	.077	37.819	y	12	3058.2	19487	19551	17939	12246	12246	1	.6	.6	4.1.1-3
13	M65	HR250_...	.943	89.391	16	.080	37.819	y	12	3058.2	19487	19551	17939	12246	12246	1	.6	.6	4.1.1-1
14	M68B	HR250_...	.836	36.1	12	.080	37.819	y	12	3058.2	19487	19551	17939	12246	12246	1	.6	.6	4.1.1-3
15	M71A	HR250_...	.840	85.953	11	.090	37.819	y	12	3058.2	19487	19487	19487	12246	12246	1	.6	.6	4.1.2-1
16	M69C	RT1.5x2...	.166	49.509	11	.011	77.916	y	5	2764.3	19487	21212	21150	12246	12246	1	.6	.6	4.1.1-1
17	M71B	HR250_...	.611	36.1	12	.061	37.819	y	12	3058.2	19487	19551	17939	12246	12246	1	.6	.6	4.1.1-3

NOTES: UNLESS OTHERWISE SPECIFIED
 1. DIMENSIONS SHOWN ARE INCHES [MILLIMETERS].



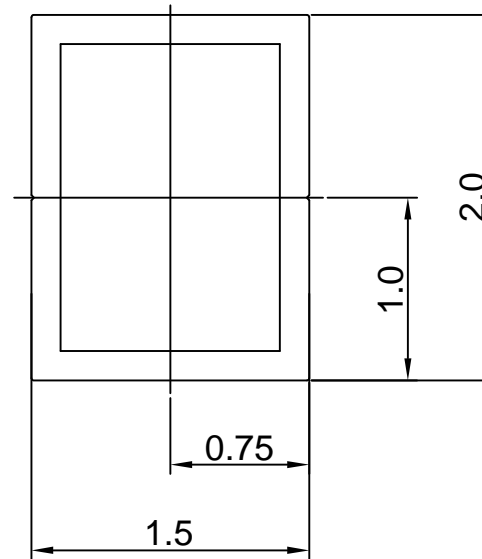
REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	10/20/2016

2	A21120-001	2" PIPE BASE	1
1	B15018-011	HEX CAP SCREW 3/8-16 X 5/8	3
ITEM	PART NUMBER	DESCRIPTION	QTY
MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		Sunmodo Corp. 1905 E 5TH STREET, STE A, VANCOUVER, WA 98661	
Tolerances		TITLE	
X.XXX ±0.01 [0.25mm]		2" PIPE BASE KIT	
X.XX ±0.02 [0.50mm]		DRAWING NUMBER	
X.X ±0.039 [1.0mm]		B K10268-001	
Unless otherwise spec'd		SCALE: NONE	
DRAWN BY		SHEET 1 of 1	
LWF		DATE	
CHECKED BY		10/20/2016	
APPROVALS			

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NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERAIL: ALUMINUM 6005-T5.
FINISH: CLEAR ANODIZED 10 μ m THICK.
3. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.



Section properties:

Weight: 1.156 lbs/ft

Area: 0.992 in²

Perimeter: 12.601 in

Bounding Box: X: -1.000,1.000

Y: -0.750, 0.750

Centroid:(0.000,0.000)

Moments of Inertia(in⁴): I_x=0.506,I_y=0.322

Section modulus in bending(in³): W_x=0.675,W_y=0.322

Radii of Gyration: X: 0.714, Y: 0.570

MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS			
All Dimensions in inches [millimeters]			
Tolerances			
X.XXX ± 0.01 [0.25mm]		Break all sharp edges	
X.XX ± 0.02 [0.50mm]		.010-.020 unless	
X.X ± 0.039 [1.0mm]		otherwise specified.	
Unless otherwise specd			
DRAWN BY	DATE	TITLE	
zcg	03/12/2014	1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661	
CHECKED BY		B	DRAWING NUMBER
			A20164
APPROVALS		SCALE:	SHEET 1 of 1
		NONE	

Sunmodo Corp.

1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661

TITLE
1.5X2 AL TUBE BRACE EXTRUSION

DRAWING NUMBER
A20164

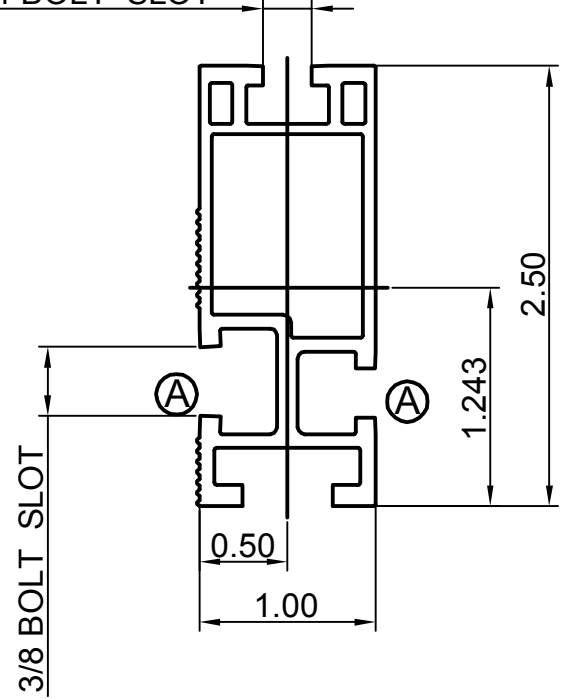
SCALE: NONE SHEET 1 of 1

REVISIONS			
REV	DESCRIPTION	BY	DATE
A	ADDED BOTTOM CHANNEL & CHANGED ONE 3/8 CHANNEL TO 1/4	zcg	02/21/2013

NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERAIL: ALUMINUM 6005-T5.
FINISH: CLEAR ANODIZED 15 μ m THICK.
3. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.

2X 1/4 BOLT SLOT



Section properties:

Weight: 0.850 lbs/ft
 Area: 0.723 in²
 Perimeter: 17.325 in
 Bounding Box: X: -0.500,0.500
 Y: -1.243,1.257

Centroid:(0.000,0.000)
 Moments of Inertia(in⁴): Ix=0.486,Iy=0.095
 Section modulus in bending(in³): Wx=0.387,Wy=0.190
 Radii of Gyration: X: 0.820, Y: 0.363

MATERIAL		SEE NOTES
Third Angle Projection:		
GENERAL SPECIFICATIONS		
All Dimensions in inches [millimeters]		
Tolerances		
X.XXX ± 0.01 [0.25mm]	Break all sharp edges	
X.XX ± 0.02 [0.50mm]	.010-.020 unless	
X.X ± 0.039 [1.0mm]	otherwise specified.	
DRAWN BY	DATE	
zcg	02/21/2013	
CHECKED BY		
APPROVALS		

Sunmodo Corp.	
1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661	
TITLE HELIO STANDARD RAIL	
DRAWING NUMBER	A20144
SCALE:	NONE
SHEET	1 of 1

4

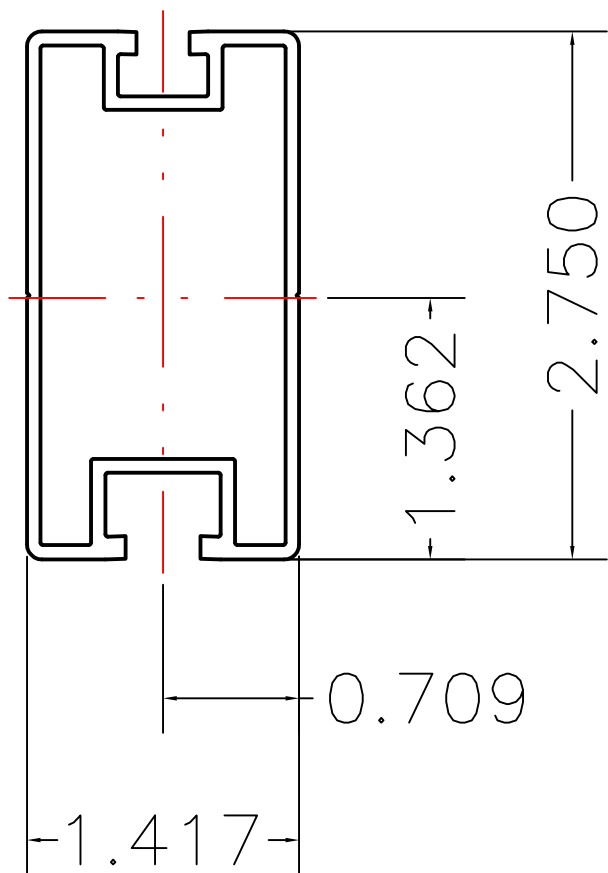
3

2

1

NOTES: UNLESS OTHERWISE SPECIFIED

- 1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
- 2. MATERIAL: 6005-T5.
FINISH: CLEAR ANODIZED 10um THICK.
- 3. THE UNSPECIFIED RADII ARE .02" MAX.
- 4. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.



Section properties:

Weight: 0.862 lbs/ft
 Area: 0.736 in²
 Perimeter: 19.824 in
 Bounding Box: X: -0.709,0.709
 Y: -1.362,1.388
 Centroid:(0.000,0.000)
 Moments of Inertia(in⁴): I_x=0.727,I_y=0.214
 Section modulus in bending(in³): W_x=0.524,W_y=0.302
 Radii of Gyration: X: 0.994, Y: 0.539

MATERIAL SEE NOTES	
Third Angle Projection:	
GENERAL SPECIFICATIONS All Dimensions in inches [millimeters]	
Tolerances	
X.XXX ±0.01 [0.25mm]	Break all sharp edges .010-.020 unless otherwise specified.
X.XX ±0.02 [0.50mm]	
X.X ±0.039 [1.0mm]	
Unless otherwise spec'd	
DRAWN BY KYY	DATE 01/18/2018
CHECKED BY	
APPROVALS	

SunModo Corp.	
14800 NE 65TH STREET, VANCOUVER WA 98682	
TITLE RAIL, HR300 (SUNRAY), EXTRUSION	
B	DRAWING NUMBER A20288
SCALE: NONE	SHEET 1 of 1

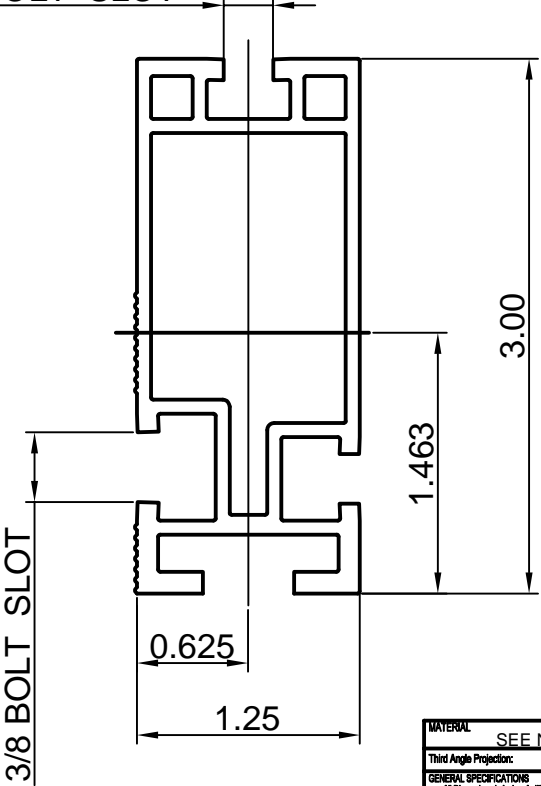
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REV	DESCRIPTON	BY	DATE
A	0.44 WAS 0.41, 0.44 WAS 0.33	LWF	11/30/2015

NOTES: UNLESS OTHERWISE SPECIFIED

1. DIMENSIONS SHOWN ARE INCHES [MILIMETERS].
2. MATERAIL: ALUMINUM 6005-T5.
FINISH: CLEAR ANODIZED 15 μ m THICK.
3. THE UNSPECIFIED DIMENSIONS ARE SPECIFIED BY 2D CAD FILE.

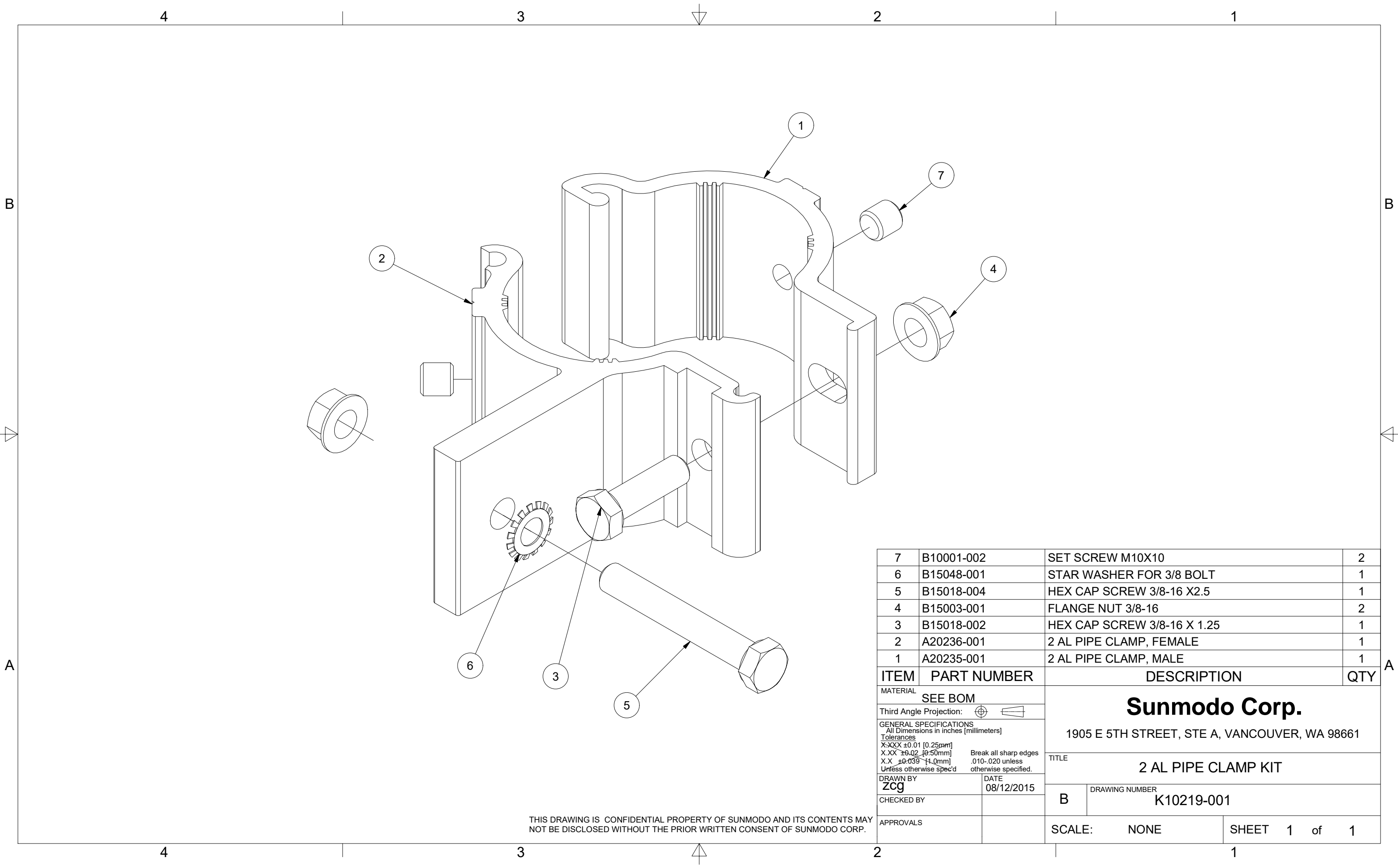
2X 1/4 BOLT SLOT



Section properties:

Weight: 1.151 lbs/ft
 Area: 0.980 in²
 Perimeter: 22.104 in
 Bounding Box: X: -0.625,0.625
 Y: -1.463,1.537
 Centroid:(0.000,0.000)
 Moments of Inertia(in⁴): Ix=1.047,Iy=0.207
 Section modulus in bending(in³): Wx=0.681,Wy=0.331
 Radii of Gyration: X: 1.034, Y: 0.460

MATERIAL		SEE NOTES	
Third Angle Projection			
GENERAL SPECIFICATIONS			
All Dimensions in Inches (millimeters)			
Tolerances			
XXX ±0.01 (0.25mm)			
XX ±0.02 (0.50mm)			
X ±0.03 (1.0mm)			
Unless otherwise specified.			
DRAWN BY		DATE	
ZCJ		02/21/2013	
CHECKED BY		DATE	
APPROVALS		DATE	
Sunmodo Corp.		1905 E 5TH STREET, SUITE A, VANCOUVER, WA 98661	
TITLE		HELIO HEAVY RAIL	
DRAWING NUMBER		A20145	
SCALE:		NONE	
SHEET		1 of 1	



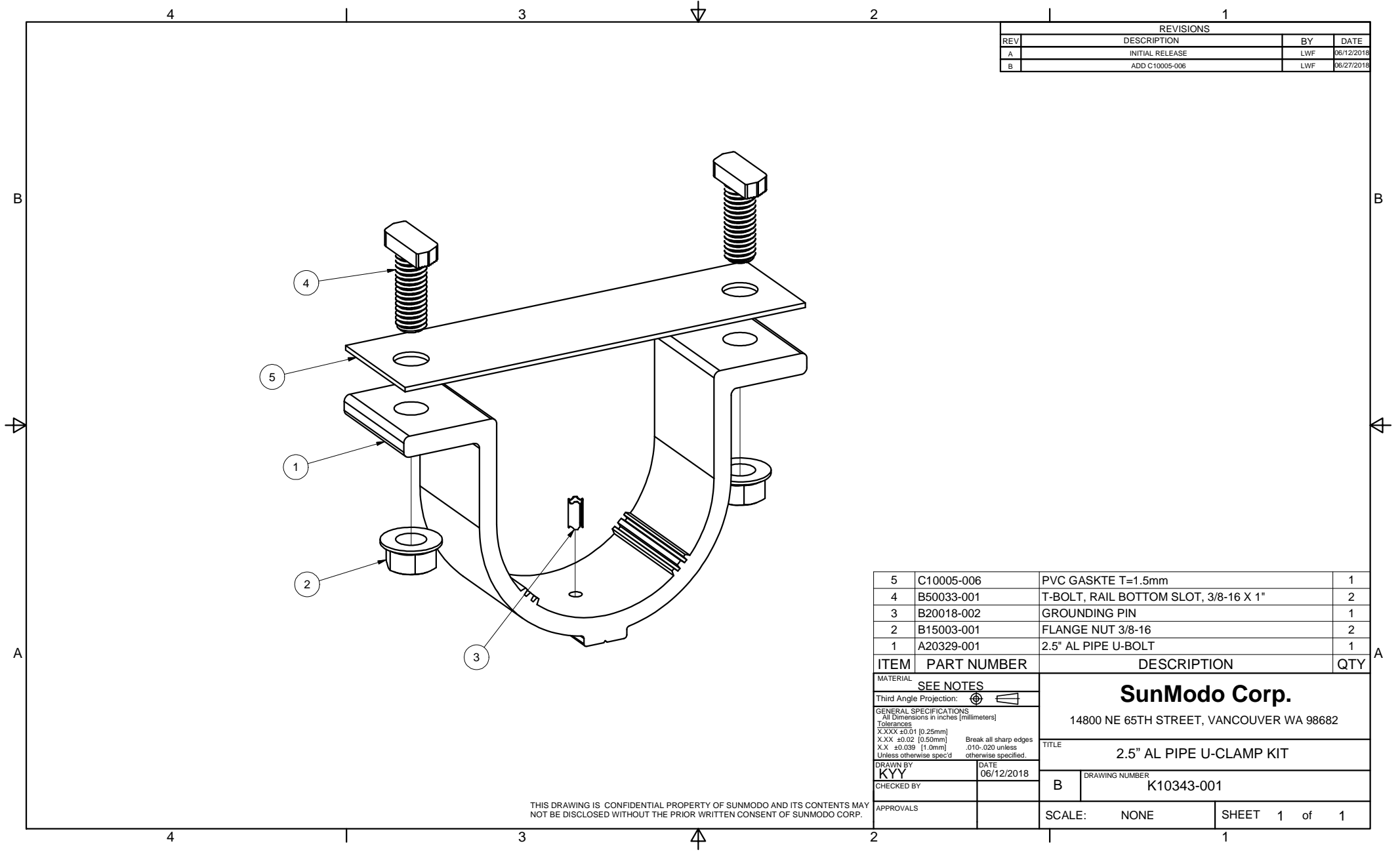
7	B10001-002	SET SCREW M10X10	2
6	B15048-001	STAR WASHER FOR 3/8 BOLT	1
5	B15018-004	HEX CAP SCREW 3/8-16 X2.5	1
4	B15003-001	FLANGE NUT 3/8-16	2
3	B15018-002	HEX CAP SCREW 3/8-16 X 1.25	1
2	A20236-001	2 AL PIPE CLAMP, FEMALE	1
1	A20235-001	2 AL PIPE CLAMP, MALE	1

ITEM	PART NUMBER	DESCRIPTION	QTY
MATERIAL		SEE BOM	
Third Angle Projection:			
GENERAL SPECIFICATIONS All Dimensions in inches (millimeters)			
Tolerances			
X.XXX ±0.01 [0.25mm]		Break all sharp edges	
X.XX ±0.02 [0.50mm]		.010-.020 unless	
X.X ±0.039 [1.0mm]		otherwise specified.	
DRAWN BY		DATE	
zcg		08/12/2015	
CHECKED BY		B	
APPROVALS		DRAWING NUMBER	
		K10219-001	
SCALE:		NONE	SHEET 1 of 1

Sunmodo Corp.
1905 E 5TH STREET, STE A, VANCOUVER, WA 98661

TITLE
2 AL PIPE CLAMP KIT

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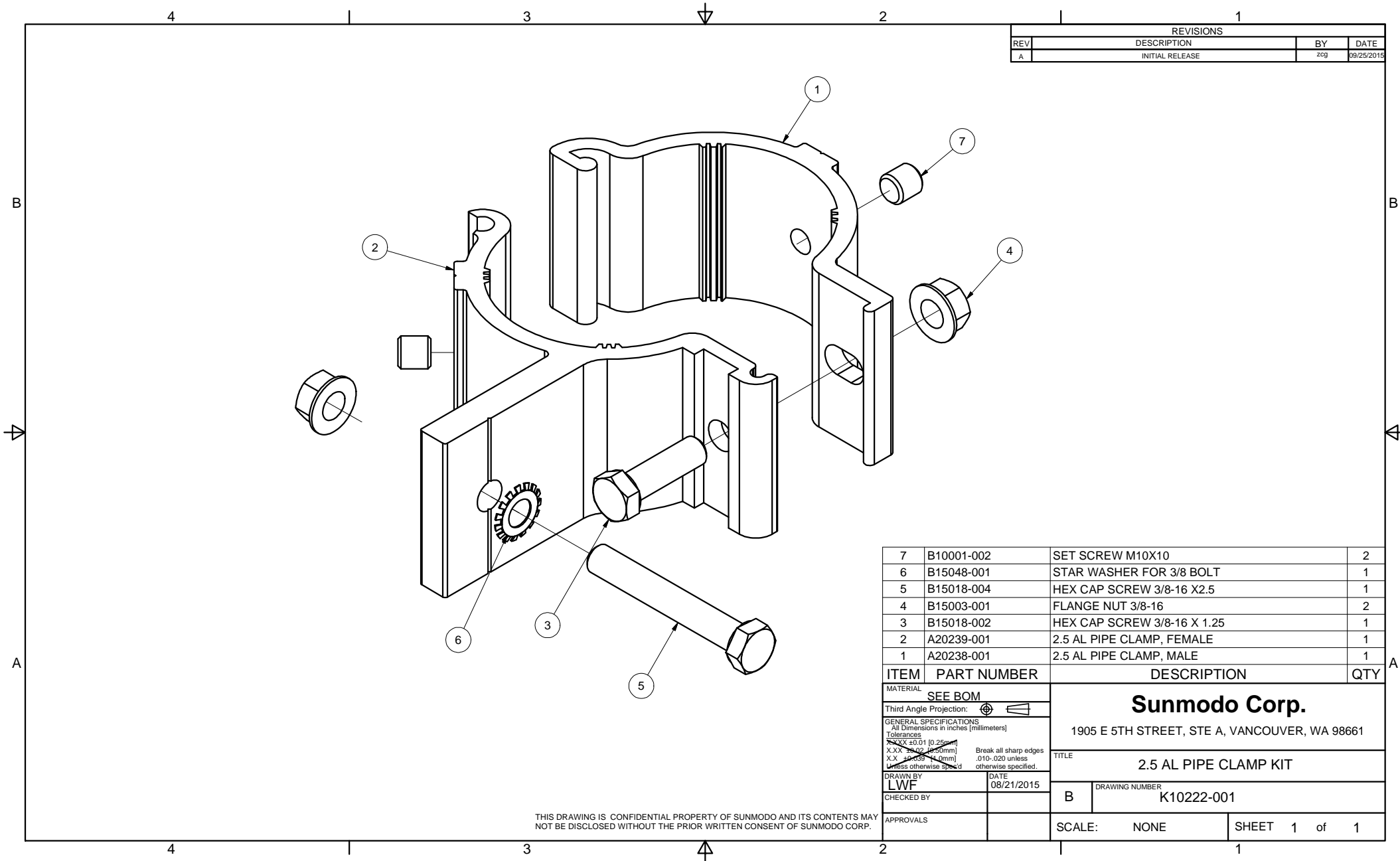


REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	06/12/2018
B	ADD C10005-006	LWF	06/27/2018

ITEM	PART NUMBER	DESCRIPTION	QTY
5	C10005-006	PVC GASKTE T=1.5mm	1
4	B50033-001	T-BOLT, RAIL BOTTOM SLOT, 3/8-16 X 1"	2
3	B20018-002	GROUNDING PIN	1
2	B15003-001	FLANGE NUT 3/8-16	2
1	A20329-001	2.5" AL PIPE U-BOLT	1

MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		<p>SunModo Corp. 14800 NE 65TH STREET, VANCOUVER WA 98682</p>	
<p>All Dimensions in inches [millimeters]</p> <p>Tolerances</p> <p>X.XXX ±0.01 [0.25mm]</p> <p>X.XX ±0.02 [0.50mm]</p> <p>X.X ±0.039 [1.0mm]</p> <p>Unless otherwise spec'd</p>			
TITLE		2.5" AL PIPE U-CLAMP KIT	
DRAWN BY	DATE	DRAWING NUMBER	
KYY	06/12/2018	B K10343-001	
CHECKED BY			
APPROVALS		SCALE: NONE	SHEET 1 of 1

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REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	zcg	09/25/2015

ITEM	PART NUMBER	DESCRIPTION	QTY
7	B10001-002	SET SCREW M10X10	2
6	B15048-001	STAR WASHER FOR 3/8 BOLT	1
5	B15018-004	HEX CAP SCREW 3/8-16 X2.5	1
4	B15003-001	FLANGE NUT 3/8-16	2
3	B15018-002	HEX CAP SCREW 3/8-16 X 1.25	1
2	A20239-001	2.5 AL PIPE CLAMP, FEMALE	1
1	A20238-001	2.5 AL PIPE CLAMP, MALE	1

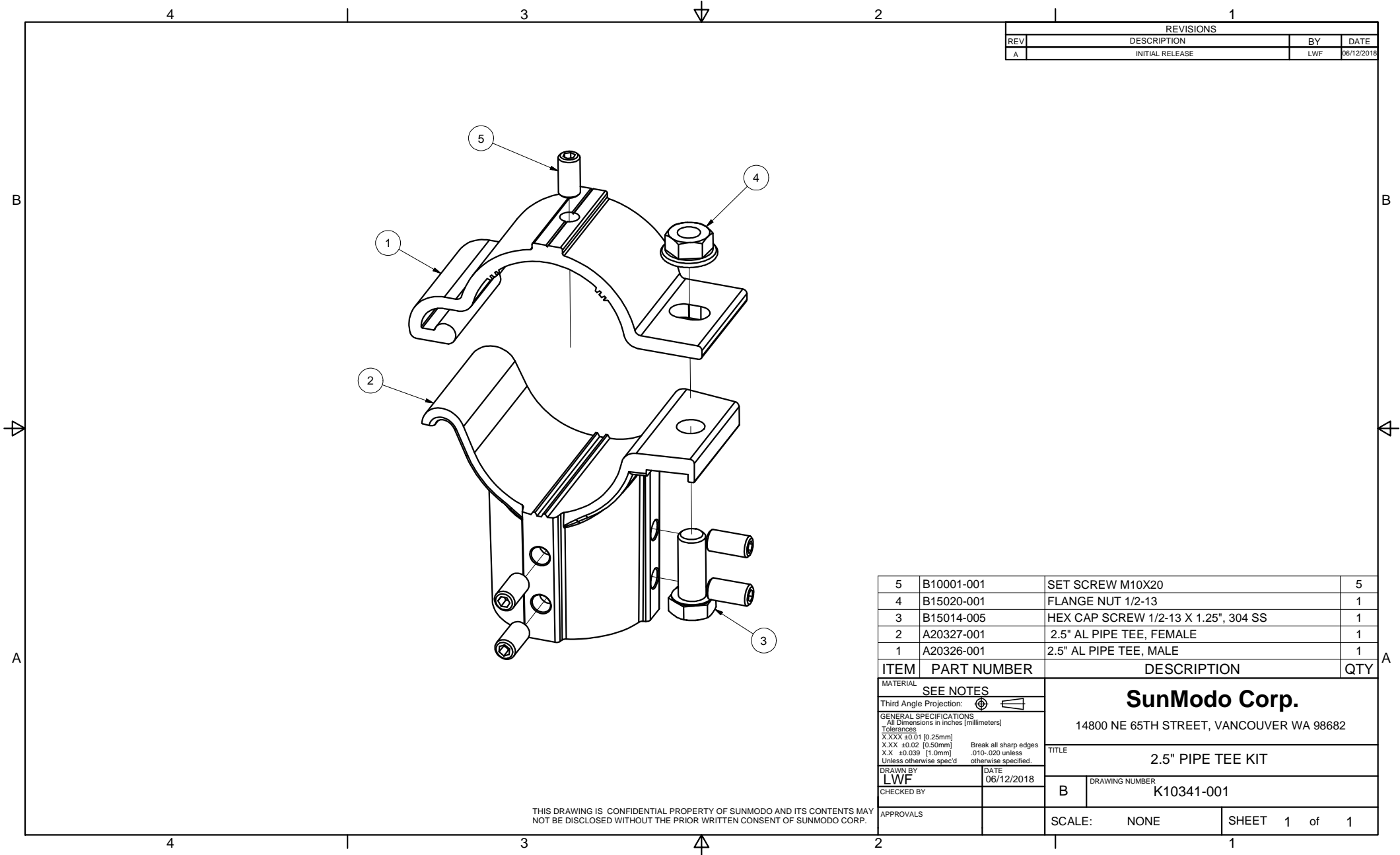
MATERIAL		SEE BOM	
Third Angle Projection:			
GENERAL SPECIFICATIONS All Dimensions in inches [millimeters] Tolerances X.XX ±0.01 [0.25mm] X.X ±0.02 [0.5mm] X.X ±0.03 [0.75mm] Unless otherwise specified.			
DRAWN BY		DATE	
LWF		08/21/2015	
CHECKED BY		B	
APPROVALS		SCALE: NONE	
		SHEET 1 of 1	

Sunmodo Corp.
 1905 E 5TH STREET, STE A, VANCOUVER, WA 98661

TITLE: **2.5 AL PIPE CLAMP KIT**

DRAWING NUMBER: **K10222-001**

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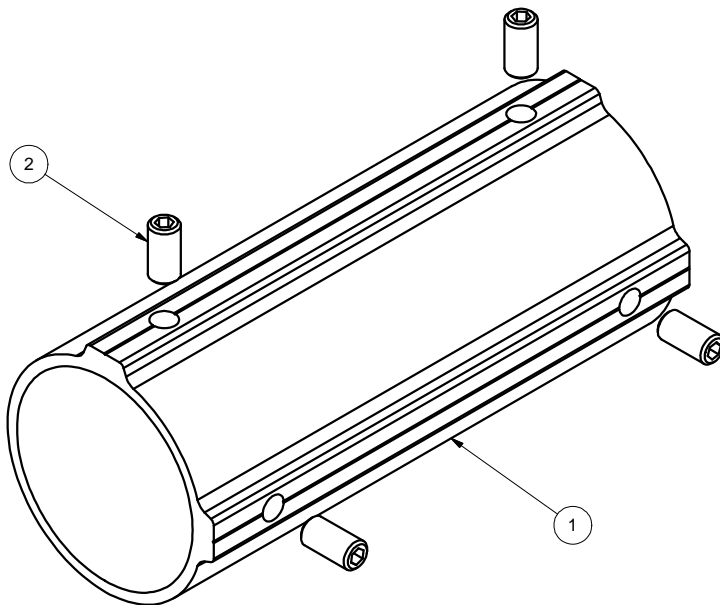
REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	06/12/2018

ITEM	PART NUMBER	DESCRIPTION	QTY
5	B10001-001	SET SCREW M10X20	5
4	B15020-001	FLANGE NUT 1/2-13	1
3	B15014-005	HEX CAP SCREW 1/2-13 X 1.25", 304 SS	1
2	A20327-001	2.5" AL PIPE TEE, FEMALE	1
1	A20326-001	2.5" AL PIPE TEE, MALE	1

MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		SunModo Corp. 14800 NE 65TH STREET, VANCOUVER WA 98682	
Tolerances		TITLE	
X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] X.X ±0.039 [1.0mm] Unless otherwise spec'd		2.5" PIPE TEE KIT	
DRAWN BY		DATE	DRAWING NUMBER
LWF		06/12/2018	B K10341-001
CHECKED BY			
APPROVALS		SCALE: NONE	SHEET 1 of 1

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REVISIONS			
REV	DESCRIPTION	BY	DATE
A	INITIAL RELEASE	LWF	06/12/2018



2	B10001-001	SET SCREW M10X20	4
1	A20328-001	2.5" PIPE SPLICE	1
ITEM	PART NUMBER	DESCRIPTION	QTY
MATERIAL		SEE NOTES	
Third Angle Projection:			
GENERAL SPECIFICATIONS		SunModo Corp. 14800 NE 65TH STREET, VANCOUVER WA 98682	
All Dimensions in inches [millimeters] Tolerances X.XXX ±0.01 [0.25mm] X.XX ±0.02 [0.50mm] X.X ±0.039 [1.0mm] Unless otherwise spec'd		Break all sharp edges .010-.020 unless otherwise specified.	
DRAWN BY		TITLE	
LWF		2.5" PIPE SPLICE KIT	
CHECKED BY		DRAWING NUMBER	
		B K10342-001	
APPROVALS		SCALE: NONE SHEET 1 of 1	

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